CITY OF MURFREESBORO PLANNING COMMISSION AGENDA

City Hall, 111 W. Vine Street, Council Chambers

MAY 5, 2021 6:00 PM Kathy Jones Chair

- 1. Call to order
- 2. Determination of a quorum.
- 3. Public Hearings:
 - **a.** Zoning application [2021-407] for approximately 17.25 acres located along the north side of Ashers Fork Drive to be rezoned from CF to RS-6, O'Brien Loyd, LLC applicant. (Project Planner: Marina Rush)
 - **b.** Zoning application [2021-403] for approximately 78 acres located along Medical Center Parkway, Robert Rose Drive, Wilkinson Pike and Willowoak Trail to be rezoned from MU, GDO-1 and GDO-2 to PUD, CH, GDO-1 and GDO-2 (Clari Park), Hines Acquisitions LLC applicant. (Project Planner: Margaret Ann Green)
 - **c.** Street renaming [2021-902] to rename an approximately two-mile long segment of Mercury Boulevard (west of South Rutherford Boulevard) to Dr Martin Luther King Jr Boulevard, City of Murfreesboro Planning Department applicant. (Project Planner: Matthew Blomeley)
 - **d.** Street renaming [2021-903] to rename an approximately 600'-long segment of Mercury Boulevard (east of South Rutherford Boulevard) to John Bragg Highway, City of Murfreesboro Planning Department applicant. (Project Planner: Matthew Blomeley)

MURFREESBORO PLANNING COMMISSION AGENDA PAGE 2 May 5, 2021

- **e.** Proposed amendments to the Zoning Ordinance [2020-807] regarding townhouses, the RS-A zone, and other miscellaneous topics and pertaining to the following sections:
 - Section 2: Interpretation and Definitions;
 - Section 19: Residential Districts;
 - Section 26: Off-Street Parking, Queuing, and Loading;
 - Chart 1: Uses Permitted by Zoning District (including Chart 1 Endnotes);
 - Chart 2: Minimum Lot Requirements, Minimum Yard Requirements, and Land Use Intensity Ratios (including Chart 2 Endnotes); and
 - Chart 4: Required Off-Street Parking and Queuing Spaces by Use.

City of Murfreesboro Planning Department applicant. (Project Planner: Matthew Blomeley)

- 4. Staff Reports and Other Business:
- 5. Adjourn.

MURFREESBORO PLANNING COMMISSION STAFF COMMENTS, PAGE 1 MAY 5, 2021

PROJECT PLANNER: MARINA RUSH

3.a. Zoning application [2021-407] for approximately 17.25 acres located along the north side of Ashers Fork Drive to be rezoned from CF to RS-6, O'Brien Loyd, LLC applicant.

The subject property consists of 17.25 acres identified as Tax Map 114 and a portion of Parcel 14.00. The subject property is along the north side of Ashers Fork Drive and east of Cason Lane. The property is contiguous to the north of the Waites Creek Crossing subdivision. The applicant requests to rezone this property from Commercial Fringe (CF) to Single Family Residential 6,000 square foot minimum lot size (RS-6).

The property is currently vacant. In the northern portion of the property, Spence Creek traverses the parcel east to southwest. The applicant stated in his application that he wishes to develop the property with 43 single family lots, at a density of 2.49 dwelling units per acre.

Adjacent Zoning and Land Uses

The adjacent zoning to the west is Planned Commercial District (PCD) and is developed with Creekside at Three Rivers Assisted Living nursing home. To the south is zoned RS-A1 and has an approved plat for the Waites Creek Crossing single-family subdivision. To the east is unincorporated land in Rutherford County zoned RM and developed with single family residences, and to the north is Spence Creek the remainder of this Tax Parcel and will remain zoned CF.

Future Land Use Map

The future land use map of the Murfreesboro 2035 Comprehensive Plan Future Land Use Map indicates that Neighborhood Commercial is the most appropriate land use for the project area. Neighborhood Commercial supports automobile oriented but designed as neighborhood scale commercial to cater to adjacent residential neighborhoods. Typically, a smaller footprint and uses such as convenience stores, professional services and small retail uses.

The proposed RS-6 zoning district is not consistent with the Future Land Use Map designation of Neighborhood Commercial because it is a residential use. Staff feels this is an appropriate change to the FLUM due to the bisection of the property from Spence Creek. The Planning Commission should discuss if this rezoning is an appropriate deviation from the FLUM. For reference, an excerpt from the future land use map can be found on the following page.



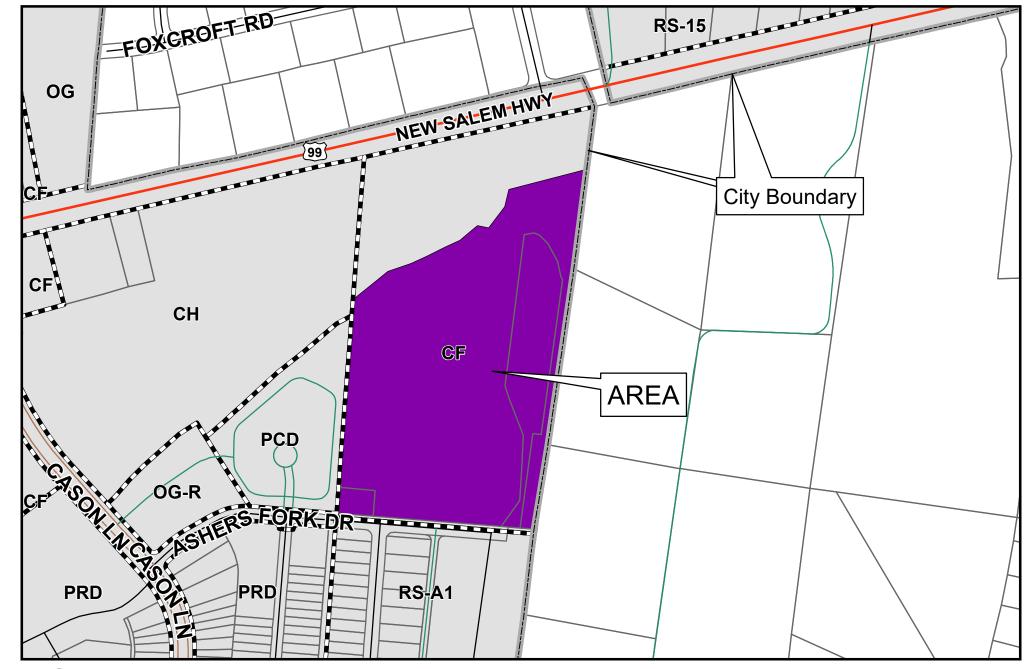
Recommendation:

Staff supports the rezoning request from CF to RS-6, including the deviation from the future land use map, for the following reasons:

- 1) Proposed RS-6 zone will be developed with single family detached residential land use will be compatible with the surrounding residential land uses.
- 2) The northern portion of the subject property fronting along New Salem Highway will remain commercially zoned (CF).
- 3) Spence Creek divides the property from east to west resulting in challenges for developing commercial along the southern half due to crossing the creek.

Action needed

The applicant will be available at the Planning Commission meeting to discuss the proposed rezoning request. The Planning Commission should discuss the matter after which it will need to discuss this matter and then formulate a recommendation for the City Council.



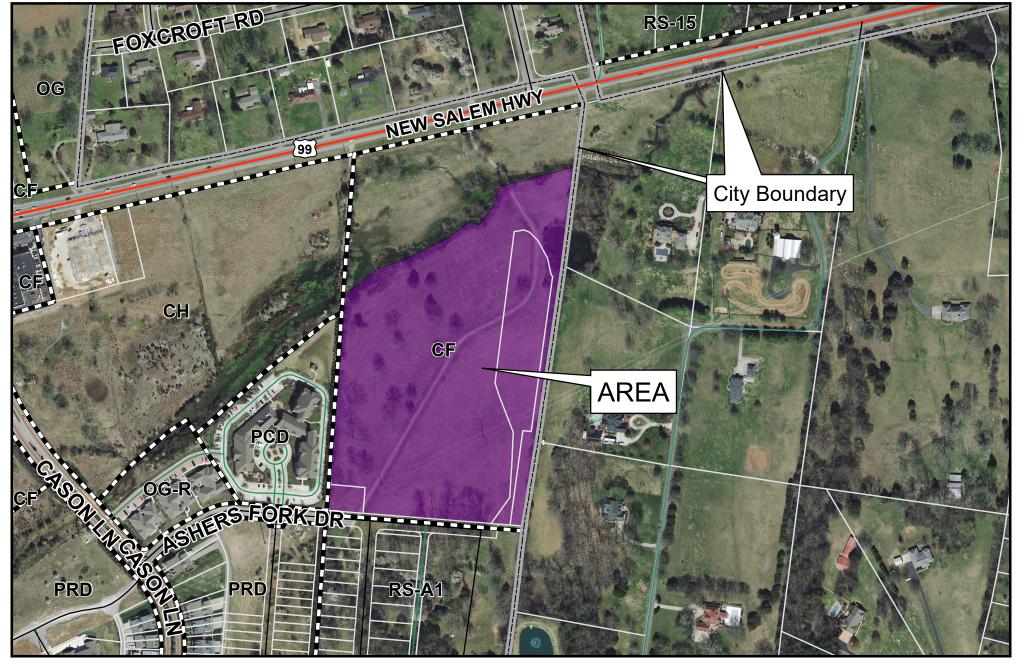


Rezoning Request for Property Along Ashers Fork Drive CF to RS-6





Planning Department City of Murfreesboro 11 W Vine St Murfreesboro, TN 37130 www.murfreesborotn.gov





Rezoning Request for Property Along Ashers Fork Drive CF to RS-6





Planning Department City of Murfreesboro 11 W Vine St Murfreesboro, TN 37130 www.murfreesborotn.gov



City of Murfreesboro

Planning and Engineering Department 111 W. Vine Street, P.O. Box 1139 Murfreesboro, TN 37133-1139 (615) 893-6441 Fax (615) 849-2606 www.murfreesborotn.gov

Creating a better quality of life

- Water January	X	www.ptm.	Revised 7/20/2018
Amount paid:	I.	Receipt #:	
Date received:	MPC YR.:	MPC #:	
*******For Office Use Only**	*********	*********	****
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Procedure for applicant:			\$950.00
•	g Applications – Pla	nned Unit Development,	\$700.00
Zoning & Rezoning development	g Applications – otl	ner than rezoning to plann	



3.17.2021

Mr. Greg McKnight Planning Director City of Murfreesboro 111 W. Vine Street Murfreesboro, TN 37130

Re: Rezoning Request

Described as a portion of Tax Map 114, Parcels 14 in Murfreesboro, Tennessee

Dear Mr. McKnight:

On behalf of our client, O'Brien Lloyd, LLC., we hereby request to rezone a portion of tax map 114, Parcel 14 consisting of 17.25 acres, currently zoned CF to the new zoning of RS-6. The developer plans to construct 43 single family lots. Thank you for your assistance with this request.

Sincerely,

Clyde Rountree, RLA

HUDDLESTON-STEELE ENG., INC.

This instrument was prepared by:
Murfree & Goodman, PLLC
805 S. Church Street, Suite 21
Murfreesboro, TN 37130
Upon information provided by the parties

SCRIVENER'S AFFIDAVIT

(Correction of Quitclaim Deed of record in Book 318, Page 431, in the Register's Office of Rutherford County, Tennessee; and

The undersigned swears and avers as follows:

I, Thomas J. Haynes, Attorney, prepared a Quitclaim Deed from William A. Waite and wife Carolyn A. Waite, to Thomas J. Haynes, Trustee, dated March 3, 1983, of record in Book 318, Page 431, Register's Office for Rutherford County, Tennessee, and do hereby certify that the following correction should be made to the above referenced instrument:

THAT WHEREAS, by inadvertence and mistake, the hereinabove said deed containing the following scrivener's error in that the GRANTORS NAMES on the Quitclaim Deed was mistyped.

NOW, THEREFORE, for and in consideration of the desire to correct the scrivener's error hereinabove recited, the GRANTORS NAMES on the Quitclaim Deed should read:

WILLIAM A. WAITE and wife, CAROLINE A. WAITE

WITNESS MY HAND on this 26 day of August, 2016.

AFFIANT

By: Thomas I

STATE OF TENNESSEE

COUNTY OF RUTHERFORD

Personally appeared before me, the undersigned, a Notary Public in and for said county and state, Thomas J. Haynes, with whom I am personally acquainted (or who proved to me her identity on the basis of satisfactory evidence), and who, upon oath, acknowledge that she executed the within instrument for purposes therein contained.

WITNESS MY HAND and official seal at my office on this the day of

August, 2016.

Notary Public

My commission expires: 5-20-17

STATE OF TENNESSEE NOTARY PUBLIC

Record Book 1498 Page 3410

Send Tax Bills To:

William A. Waite 2217 Tomahawk Trace Murfreesboro, Tennessee 37130 This Instrument Prepared By Thomas J. Haynes, Attorney Murfreesboro, Tennesse

QUITCLAIM DEED

FOR AND IN CONSIDERATION of love and affection (and no monetary consideration) and for the express purpose of making a re-conveyance to create the estate of tenancy by the entirety between William A. Waite and wife Carolyn A. Waite. We, William A. Waite and wife Carolyn A. Waite by these presents do hereby quitclaim and convey unto THOMAS J. HAYNES, Trustee, his successors and assigns, the following described tract or parcel of land in the 12th Civil District of Rutherford County, Tennessee, and being described as follows, to wit:

BEGINNING in the center of the Salem Turnpike at the northeast corner of the Carter tract, the northwest corner of this tract, and running thence with the east line of the Carter tract (the fence line) S $1/2^{\circ}$ W 256.1 poles to a cedar post, an elbow corner in the Jennings tract thence with a fence S 87° E 49 poles to a cedar post between an elm on the south and a walnut tree on the north side thereof; thence N $1/2^{\circ}$ E 78.4 poles to a burnt walnut stump at the northwest corner of the Parsley tract; thence N $87-3/4^{\circ}$ W 11.8 poles to a set stone; thence N 3° E 194 poles to the center of the Salem Turnpike; thence with center of same S 74° W 50.4 poles to the point of beginning, containing 75.76 acres, more or less.

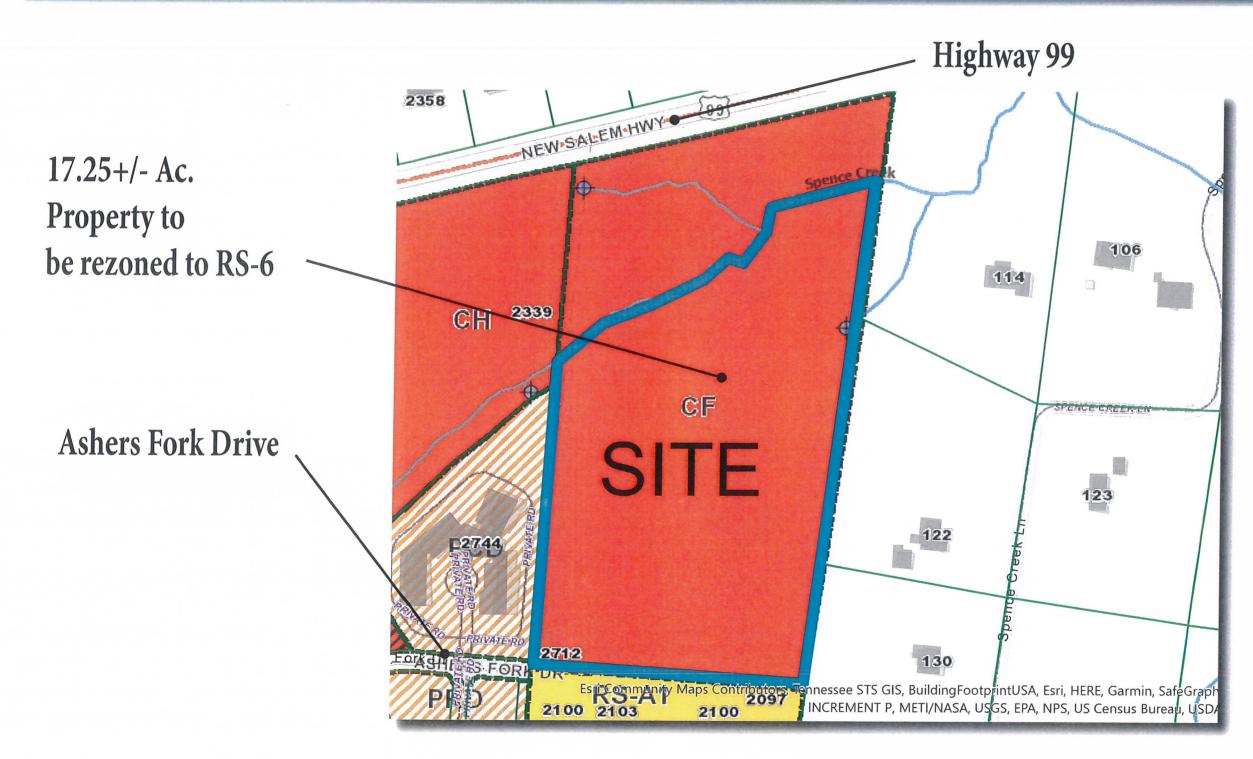
BEING the total of that property conveyed to William A. Waite as follows: fifteen and 76/100 (15.76) acres by Deed of Record in Deed Book 303 page 219; twenty (20) acres by Deed of Record in Deed Book 303 page 221; forty (40) acres by Deed of Record in Deed Book 303 page 227, of the Register's Office of Rutherford County, Tennessee.

Said property is conveyed subject to such limitations, restrictions, and encumbrances as may affect the premises.

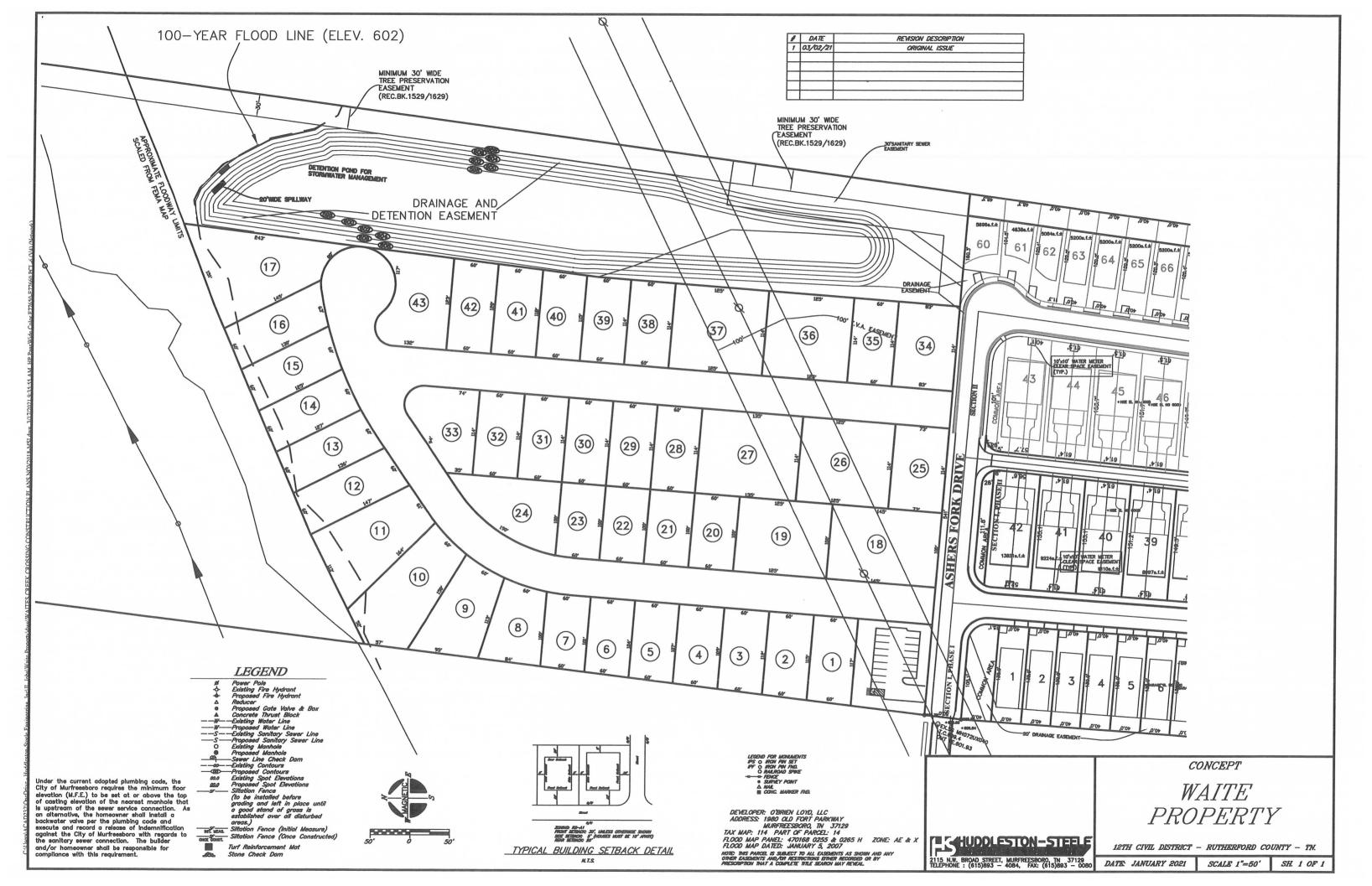
This is improved property, on Salem Road, Rutherford County, Tennessee.

And now, I, the said THOMAS J. HAYNES, Trustee, for the express purpose of carrying out the intent of this conveyance, as above set out, do hereby quitclaim and convey unto WILLIAM A. WAITE and wife, CAROLYN A. WAITE, as tenants by the entirety, their heirs and assigns, the same property hereinabove described and set forth, to which reference is here made, and said property is conveyed subject to the same limitations, restrictions and encumbrances as may affect the premises, as above set forth. 431

Waite Property Rezoning Exhibit







MURFREESBORO PLANNING COMMISSION STAFF COMMENTS, PAGE 1 MAY 5, 2021

PRINICIPAL PLANNER: MARGARET ANN GREEN

3.b. Zoning application [2021-403] for approximately 78 acres located along Medical Center Parkway, Robert Rose Drive, Wilkinson Pike and Willowoak Trail to be rezoned from MU, GDO-1 and GDO-2 to PUD, CH, GDO-1 and GDO-2 (Clari Park), Hines Acquisitions LLC applicant.

Introduction

The subject property is located along the north side of Medical Center Parkway, south of Wilkinson Pike, east of Greshampark Drive and west of Robert Rose Drive (Tax Map 079 Group 094.00). The property consist of 77.8 acres and is zoned MU (Mixed Use District), GDO-1 and GDO-2. The properties to the north are in the unincorporated area of Rutherford County and are mostly developed, single-family lots. The Stones River National Battlefield is to the northeast of the subject property. Properties to the west and south are developed, commercial properties and include the Chamber of Commerce, the convention center and the Avenue Lifestyle center. Henley Station apartments and The Villages of Murfreesboro retirement and assisted living facility are contiguous to the subject area. The Villas at Indian Creek and Manson Pike Crossing are two large townhome developments located in the GDO. Gateway Village located on North Thompson Lane is a mixed-use development that includes residential condominiums. The GDO currently has 2,806 apartments units available or under construction with 1,578 of those units on the east side of I-24, as shown on the following map.



Background

Reapplication when denied

An application to rezone this property from MU, GDO-1 and GDO-2 to PUD, CH, GDO-1 and GDO-2 was made last year (file 2020-409 Clari Park). After review, deferral and then a recommendation of approval from the Planning Commission, the application went before the City Council. After considering the zoning map amendment, the City Council denied the requested rezoning. The Murfreesboro Zoning Ordinance states that:

(G) Reapplication when denied. If an application for an amendment to the zoning ordinance or zoning map is denied by the Council or is withdrawn by the applicant after a first reading of the proposed ordinance by the Council, a reapplication pertaining to the same property and requesting the same amendment may not be filed within eighteen months of the date final action was taken on the previous application or the date it was withdrawn unless such reapplication is initiated by the Department, Commission or authorized by the Council. An applicant may not withdraw the application after the notice of public hearing before the Council has been published in the local newspaper, without the permission of the Mayor and Council. [excerpted from Zoning Ordinance Sec. 6(G)]

It is the Development Services Director and the Planning Director's opinion that the differences between the overall plan in this application and the plan contained in the application rejected by City Council on October 22, 2020, are sufficient to exempt this plan from the 18-month limitation in Zoning Ordinance Sec. 6(G), and asks that the Planning Commission concur. In the alternative, Staff asks that the Planning Commission recommend that City Council authorize the consideration of this plan in accord with the provisions of Section 6(G).

Zoning:

In December of 2013 the City initiated rezoning of the subject property from OG (General Office District) and RS-15 (Single-family, Residential District) to the newly created MU district. A portion of the properties along Wilkinson Pike remained zoned RS-15. In 2017, the City Council approved a request made by the property owner to rezone the remaining segment of RS-15 property to MU and to remove the Wilkinson Pike buffer from the property.

Wilkinson Pike Buffer/ Landscape Berm

The Wilkinson Pike buffer is a 100-foot wide area that extends along the south side of Wilkinson Pike and was placed on this property when the Gateway Design Overlay District was established in 2004. It serves as a transition point between residents across the street and the national park by restricting development and by increasing the minimum building setback to 100-feet or more. In an effort to alleviate concerns with the removal of the Wilkinson Pike Buffer in this area, the property owner volunteered to place restrictive covenants on the property which required a landscaped berm along

Wilkinson Pike to be completed during the next growing season (fall 2017). These restrictive covenants were recorded and presented to the City Council prior to the final reading of the ordinance.

During the public hearing on October 22, 2020, City Council expressed concern that the landscaped berm along Wilkinson Pike had not been installed despite assurances made by the property owner's representatives in 2017. The Planning Commission approved plans for the landscaped berm on September 20, 2017, and again on March 3, 2021. The City Council recently approved an *Amended Agreement for Landscape Buffer and Easement (Wilkinson Pike*) which must be signed by the Mayor and recorded in the Deed Records. The contractor has begun work on the berm.

Mixed Use district

The Mixed Use district permits various types of commercial, office and institutional uses and incorporates some multi-family. A few years after the creation of the MU district in 2013, the Murfreesboro City Council became aware that it was becoming consumed by multi-family uses, the only type of residential use permitted in the MU district. City Council asked staff to draft a Zoning Ordinance amendment that protected the mixed-use vision for this area and requires it to develop with primarily commercial, office and institutional uses. The following was adopted:

In developments consisting of 10 or more acres in the MU zoning district, the use "dwellings, multiple-family" shall constitute no more than 25% of developable land area. In developments consisting of fewer than 10 acres in the MU zoning district, the use "dwellings, multiple-family shall constitute no more than 50% of developable land area.

The Clari Park PUD proposes to utilize approximately 39% of the land for commercial purposes, approximately 30% as townhomes and single-family detached and approximately 25% as apartments with 8,000 square feet first floor commercial space.

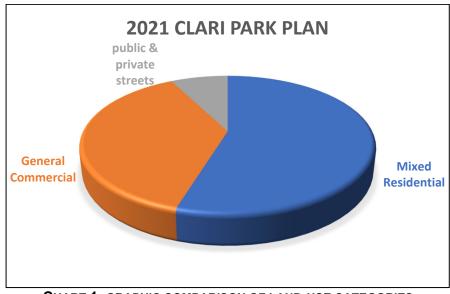


CHART 1- GRAPHIC COMPARISON OF LAND-USE CATEGORIES

Clari Park PUD & CH- 2021 version

Clari Park Planned Unit District- 47.3 acres and CH- 29.3 acres

The Clari Park PUD is a 77.8 acre development, consisting of single-family attached (townhomes), single-family detached, multi-family apartments and commercial outparcels. The project is divided into 7 distinct areas. In total, the plan proposes a 708 residential units which is broken down to a maximum of 488 multi-family dwelling units, 182 townhomes, and 38 single-family detached lots. If the property were to remain zoned MU, then approximately 488 dwelling units would be permitted (25 du/acre on 25% of 77.8 acres). Approval of this zoning plan would also necessitate an amendment to the approved Master Plan for these properties.

To understand the 2021 version of the Clari Park PUD, it is helpful to understand what has changed from the 2020 version. Below are a few charts and graphs to help illustrate the similarities and differences between plans. Essentially, the 2021 version has 182 less dwelling units, leading to a decrease in density, it introduces single-family detached homes as an option, redistributes the amount of acreage devoted to particular uses, and changes the phasing of Area 4 from phase three to phase two. The 2021 version of the plan also eliminates the "stacked flats" or condominium options previously available.

	2020 Clari Park	2021 Clari Park	Difference
Apartment units	600	488	112
Townhome units	290	182	108
Single-Family units	0	38	-38
Total d.u.	890	708	182

TABLE 1- DIFFERENCES BETWEEN NUMBER OF DWELLING UNITS

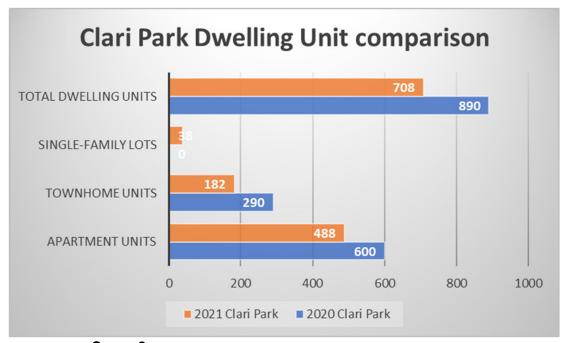


CHART 2- COMPARISON OF CHANGES IN DWELLING UNIT COUNT

2021 Clari Park		Acres	Dwelling Units	Density DU/acre	Phase
Area 1	СН	5.5	0	N/A	
Area 2	Townhomes & Single- family detached (PUD)	17	155	9.1	PHASE 1
Area 3	CH	15.8	0	N/A	
Area 4	СН	8	0	N/A	PHASE 2
Area 5	Multi-family apartments & commercial (PUD)	10.9	280	25.7	
Area 6	Townhomes & Single- family detached (PUD)	6.4	65	10.2	PHASE 3
Area 7	Multi-family apartments OR commercial (PUD)	8.5	208	24.5	

TABLE 2-2021 CLARI PARK DATA

	2020 Clari Park	Acres	Dwelling Units	Density DU/acre	Phase
Area 1	СН	8	0	0	
Area 2	Townhomes (PUD)	15.8	165	10.4	PHASE 1
Area 3	СН	15.8		0	
Area 5	Multi-family apartments & commercial (PUD)	9.3	305	32.8	PHASE 2
Area 4	СН	6.7	0	0	
Area 6	Townhomes or multi- family condominiums (PUD)	8.3	125	15.1	PHASE 3
Area 7	Multi-family apartments & commercial (PUD)	10.1	295	29.2	

TABLE 3-2020 CLARI PARK DATA

The commercial portions of this plan are unknown at this time, therefore the application is to rezone these properties from MU to CH, while remaining within the GDO-1 and GDO-2 overlays. Although they could develop under the existing MU district, the applicants want to demonstrate they do not wish to allow any additional residential uses on these properties- MU district allows multi-family by right while CH prohibits residential uses altogether. Approximately 29.3 acres is proposed to zoned CH, which potentially may result in 8 commercial outparcels along Medical Center Parkway, 7 lots along Clari Lane and 1 along Silohill.

Transportation & Drainage:

Staff from the Public Infrastructure Department have requested the applicants provide an updated Traffic Study in order to provide comments and feedback regarding the project. Similar to the initial plan, the applicants are requesting to phase the public infrastructure work, utilities extensions, and rights-of-way extension. Staff also met with the design team to discuss opportunities for street improvements that can improve the traffic operations of the streets. Some of these improvements include turn-lane improvements, additional turn lanes and pedestrian connections.

The development team is currently working with City Engineering to evaluate the best storm drainage solutions for the site. Any on site or off-site storm drainage solutions proposed will be reviewed and approved by city staff.

Exceptions

The ordinance approving the planned development may provide for such exceptions from the non-overlay district zoning regulations governing use, density, area, bulk, parking, and such Subdivision Regulations as may be necessary or desirable to achieve the objectives of the proposed planned development, provided such exceptions are consistent with the standards and criteria contained in this section and have been specifically identified and requested in the application for a planned development. Unless the ordinance approving a planned development contains a clear statement of exceptions to them, the standards and criteria of the district zoning regulations (non-overlay) will apply to all planned developments. The only exceptions to overlay district regulations permitted in a planned development are exceptions, in the Battlefield Protection District zone and the Gateway Design Overlay District zone, to a building height, a setback, or a landscaping requirement.

The PUD program book requests several exceptions and is found on page 16 of the pattern book.

Exception

- 1. Requesting "Single Family Attached" Residential and "Single Family Detached" Residential Use be permitted (Not currently permitted in underlying MU zoning).
- 2. Requesting exception to maximum 25.6 units per acre density requirement for area 5. The average residential density allowed for the overall master plan for residential parcels in Clari Park is approximately 16.6 units per-acre.

- 3. Request adjustment to parking ratio requirement for 1-bedroom residential multifamily units of 1.5 spaces per bedroom to 1.1 per bedroom and removal of parking requirement for up to 10,000 sf of office space on first floor of each Multifamily project.
- 4. An exception to allow outdoor sales and the sale of food and beverage in park space and public open space for temporary special events.
- 5. Porches, stoops, and bay windows may extend into setbacks.

Future Land Use Map

The *Murfreesboro 2035 Future Land Use Map* indicates that Urban Commercial/ Mixed-Use Character (UC) is most appropriate for the subject property.

This designation allows a broad range of commercial, office and high density residential uses and public spaces serving surrounding neighborhoods, commercial / professional business parks and visitors from nearby communities.

If the property develops as a mixed-use development, it will be consistent with the UC character.

Future Land Use Map



Future Land Use Map

LAND USES Proposed Land Uses Undeveloped Parks Suburban Estate Suburban Residential Auto Urban Residential Multi Family Residential General Commercial Neighborhood Commercial Urban Commercial / Mixed Use Central Business District Business Park Light Industrial Heavy Industrial Public / Institutional

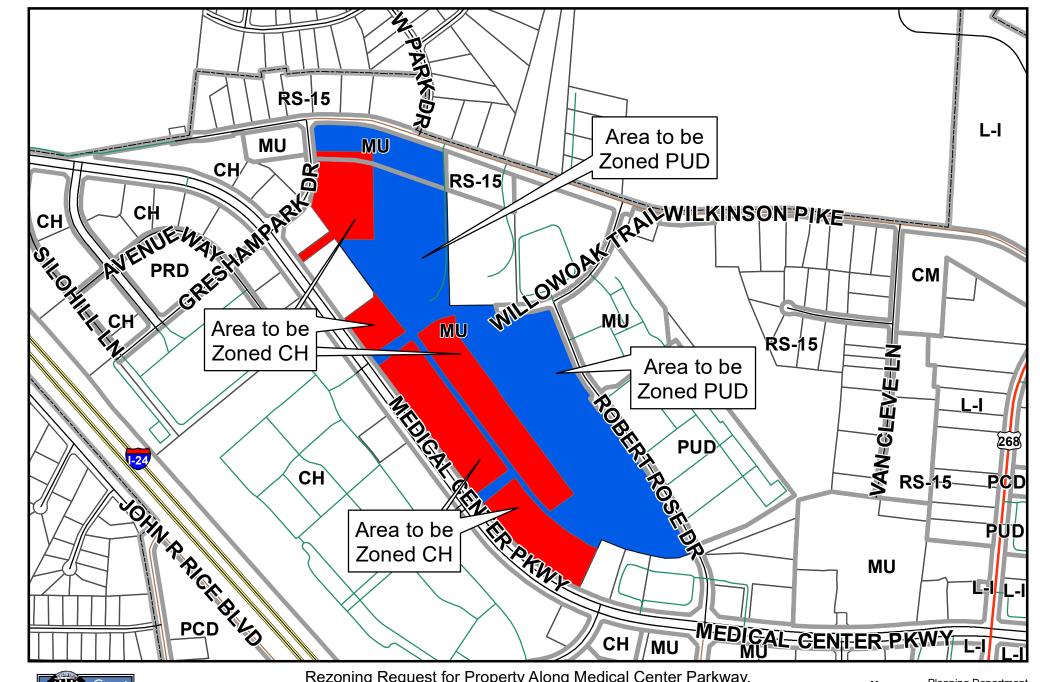
Recommendation:

The applicant held a public hearing at Murfreesboro Fire Station #4 on Tuesday, April 6, 2021.

Staff is generally supportive of this rezoning request for the following reasons:

- 1. the proposed land use will be compatible with the surrounding land uses and will create a balance of mixed uses with a strong management regime;
- 2. the proposed development will contribute to the vitality of the area by adding home ownership options within the Gateway Design Overlay District,
- 3. the development quality is generally in conformance with the GDO district standards in the *Murfreesboro Zoning Ordinance* as well as with the standards of the *Murfreesboro Design Guidelines*,
- 4. the zoning request is generally consistent with the recommendations of the *Future* Land Use Map

The Planning Commission will need to conduct a public hearing prior to making a recommendation to the City Council. A copy of the revised Clari Park pattern book and Traffic Impact Study have been included with the staff report.

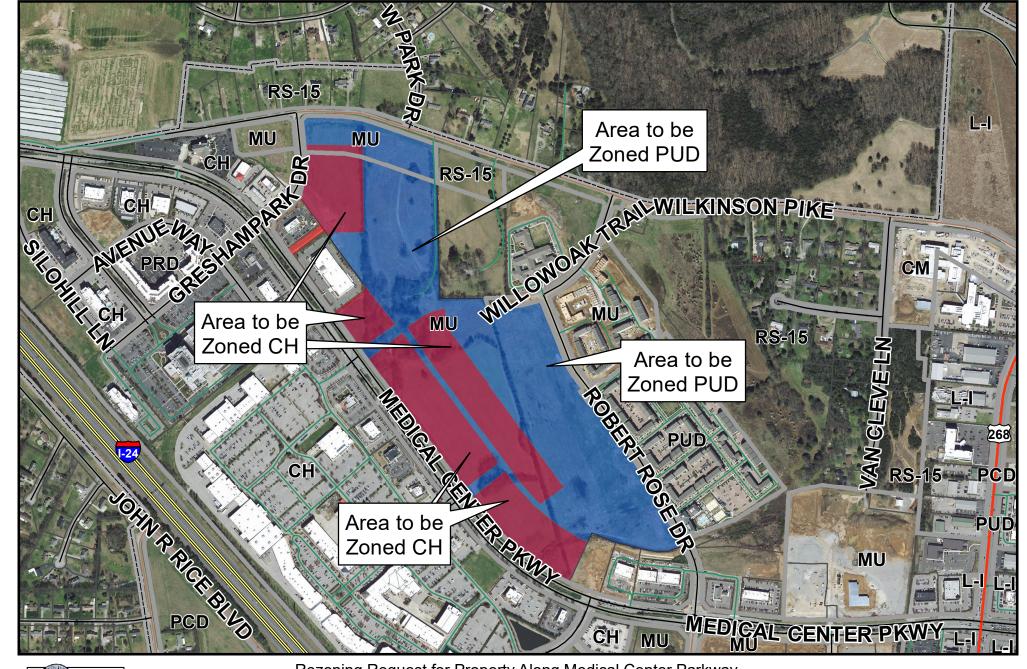




Rezoning Request for Property Along Medical Center Parkway,
Robert Rose Drive and Wilkinson Pike
MU to CH and MU to PUD (Clari Park PUD)
(The existing GDO-1 and GDO-2 boundaries are not affected by this zoning request)



Planning Department City of Murfreesboro 11 W Vine St Murfreesboro, TN 37130 www.murfreesborotn.gov





Rezoning Request for Property Along Medical Center Parkway,
Robert Rose Drive and Wilkinson Pike
MU to CH and MU to PUD (Clari Park PUD)
(The existing GDO-1 and GDO-2 boundaries are not affected by this zoning request)

2,000

500

1,000



■ Feet

3,000

Planning Department City of Murfreesboro 11 W Vine St Murfreesboro, TN 37130 www.murfreesborotn.gov



a	PROJECT INTRODUCTION Introduction Historical Cultural Context	1-2	d	 GREEN SPACE & LINEAR PARKS Green Space Connections Master Plan Clari Lane Linear Park Connection of Open Space The Grand Lawn / Honey Locust Lane / Clari Lane 	25-30
h	SITE INVENTORY/ANALYSIS Location Map	4-9		 The Grand Lawn Concept The Commercial Lawn Concept 	
D	 Surrounding Land Use & Zoning Existing Site Conditions Existing Utilities Map Overlays & Flood Zones Future Long Range Plan 		e	COMMERCIAL HIGHWAY (AREAS I & 4) General Description Photographic Examples	31-32
			C	SINGLE FAMILY ATTACHED & SINGLE FAMILY	
C	 Master Plan - Option A Master Plan - Option B Proposed Land Use Map - Option A Proposed Land Use Map - Option B Land-Use Table Land Use Parameters, Requested Exceptions Community Management & Operations Vehicular Transportation Network Pedestrian Circulation Plan Roadway Sections Phasing - General Master Plan Public Improvements 	10-24	T	 General Description Single Family Attached & Single Family Detached - Conceptual Lage Townhome Elevations and Materials (Areas 2 & 6) Greenspace/Amenity & Berm Enlargements (Area 2) Single Family Detached Amenity and Berm Enlargement Greenspace Enlargement (Area 6) Single Family Attached Private Street Network / Utilities Parking Diagrams Single Family Attached (Areas 2 & 6) Architectural Guidelines (Areas 2 & 6) Architectural Examples - Single Family Detached (Areas 2 & 6) Parking Layout Typical Lot Diagrams 	33-51
	 Public Improvements Phasing 		g	 MULTI-FAMILY RESIDENTIAL / OFFICE (AREAS General Description Office / Townhome Layout Options Office Architectural Examples Multi-Family Architectural Examples Multi-Family Architectural Perspective Multi-Family Ground Floor Office Photographic Amenity Examples Grand Lawn Images 	S 5 & 7) 52-60
				COMMERCIAL (AREA 3)	61-62

General Description / Conceptual LayoutsPhotographic Examples

Clari Park a

Introduction

Project General Description

Hines, Ragan Smith, and SEC envision creating a community that offers its residents a memorable sense of place through emphasis on parks and greenspace and a true sense of belonging through social programs that weave a fabric of community. Clari Park will be the realization of this vision.

Clari Park is a key property within the Gateway District. It is approximately 78 acres in size and will complete the majority of undeveloped land remaining along Medical Center Parkway. The master plan has been thoughtfully developed to blend into and respect the context of land uses and transportation networks that surround it.

Creation of an overall Master Plan for this parcel allows for the consideration of a mixture of uses that relate strongly to each other as well as the surrounding land uses. Program elements with higher occupancy densities and greater traffic generation are proposed at the core of the project and in relationship to Medical Center Parkway. Lower density uses proposed along the northern periphery of the site respect the adjacent residential uses on Wilkinson Pike.

Circulation within the Master plan is heavily focused on the pedestrian with the development of green spaces, linear parks, and amenities that facilitate connectivity and promote a walkable lifestyle. Clari Park aims to serve the residents and visitors of Murfreesboro with a quality of life experience that provides opportunities to live, shop, work, and enjoy all that Murfreesboro and The Avenue has to offer in one convenient and walkable location. Given these attributes, the project will appeal to a wide range of homeowners, business owners, and office and apartment tenants which will include young urban professionals, young couples just starting a family, and mature couples with children that have reached independence.



Hines is a privately owned, global real estate investment, development and management firm with a presence in 205 cities in 24 countries and \$133.3 billion of assets under management. The most valuable assets within Hines are the 4,500+ professionals that strive daily to deliver exceptional service to the communities within which we reside, the tenants whom we serve, and the partners who trust Hines with their capital. Hines' project teams strive to set the standards for quality of execution and management in their respective markets and product types. Over and above financial returns, they improve cities and pioneer new sustainable practices. Combining insights from local teams, central resources that act as the depository of decades of experience, and a commitment to long-term value creation, Hines has mastered the art of building places for people and endeavors to leave a positive legacy on the built environment in every city in which it operates.

"Hines began as a one-man operation in 1957 with the sole focus of delivering better quality services and products to tenants and investors. More than half a century later, with more than 4,500 professionals working on five continents, our philosophy has not wavered and our commitment to excellence in the built environment is stronger than ever." – Gerald D. Hines



For the past 30 years, the Hines Southeast Region team has specialized in the creation of innovative and successful mixed-use communities and buildings including several in Middle Tennessee.

- 222 2nd Avenue Nashville, TN Class A Office and Retail 98% leased and sold in 2020 for record price (\$730 psf 2.5x higher than Pinnacle Building sale at \$294 psf in 2013)
- Cool Springs Franklin TN 1,000-acre community integrating apartments, retail, office and hospitality uses that is the benchmark for suburban core development in the region
- Deerfield Alpharetta, GA 500-acre mixed-use community integrating apartments, retail and office uses is a walkable campus. Successfully attracted corporate office users to a pioneering location.
- Palencia St. Augustine, FL 2,500-acre master planned community that blended innovative land planning, a unique architectural theme and exceptional community management to create the premier mixed-use community in Northeast Florida.





158 years ago, not far from Clari Park, a historic battle was set to begin...

"Just before 'tattoo' the military bands on each side began their evening music. The still winter night carried their strains to great distance. At every pause on our side, far away could be heard the military bands of the other. Finally one of them struck up 'Home Sweet Home.' As if by common consent, all other airs ceased, and the bands of both armies as far as the ear could reach, joined in the refrain. Who knows how many hearts were bold next day by reason of that air?"



- Sam Seay, First Tennessee Infantry

The quote above describes an event which took place on the eve of the Civil War Battle of Stones River in late December 1862. It is a reminder that, despite political, economic, or philosophical differences, all people find common ground in the warmth of their memories of home.

Through thoughtful design, execution quality, and community programs, Clari Park aims to embody that notion of people coming together, in harmony.

The song played by both opposing armies, "Home Sweet Home", on that night originated in the 1828 opera "Clari". It is with a nod to the memory of this moving moment in time that Clari Park has been named.

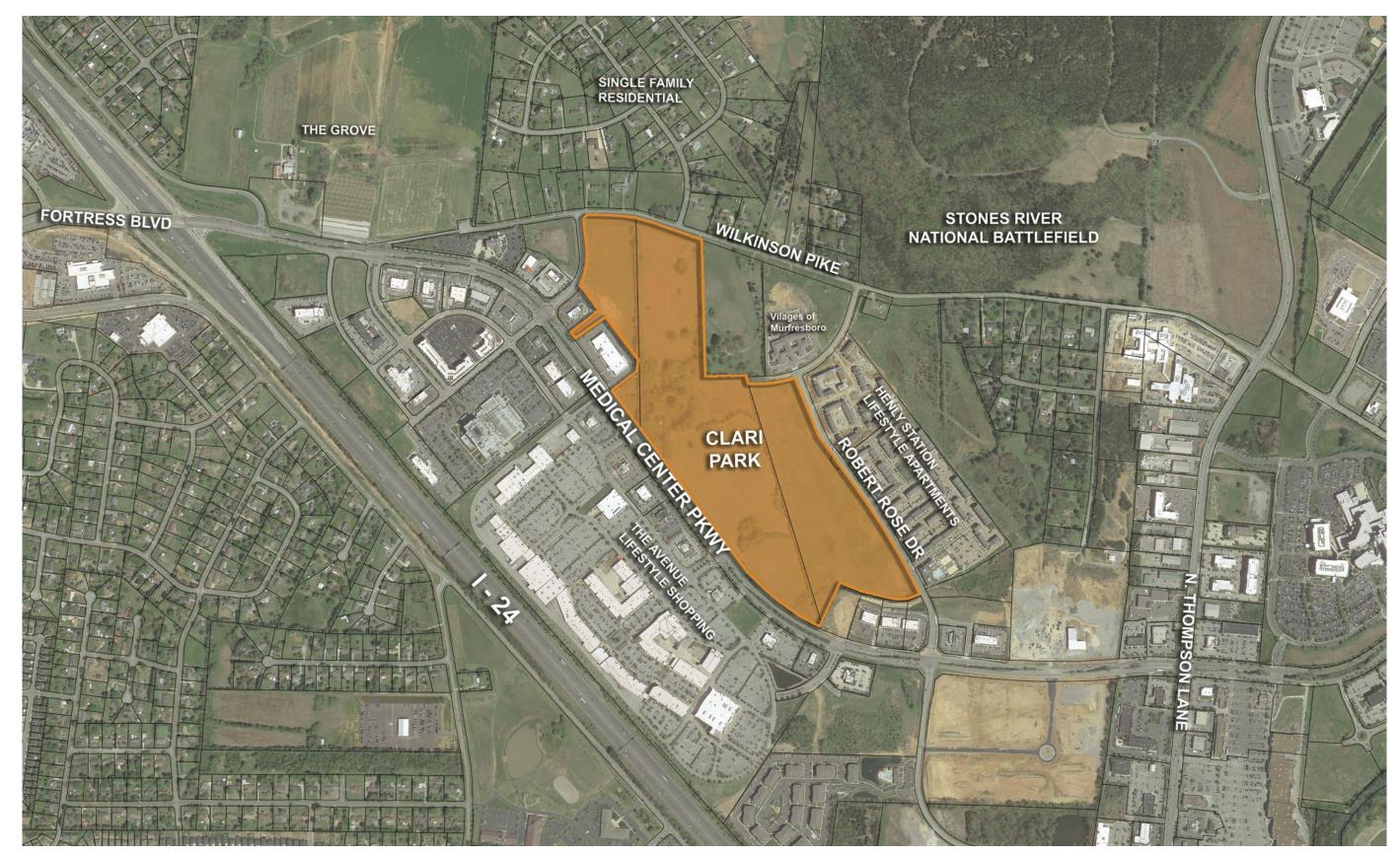
There is an opportunity in Clari Park to recognize the history of the site with historic markers placed strategically in open space and public intersections

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b Clari Park

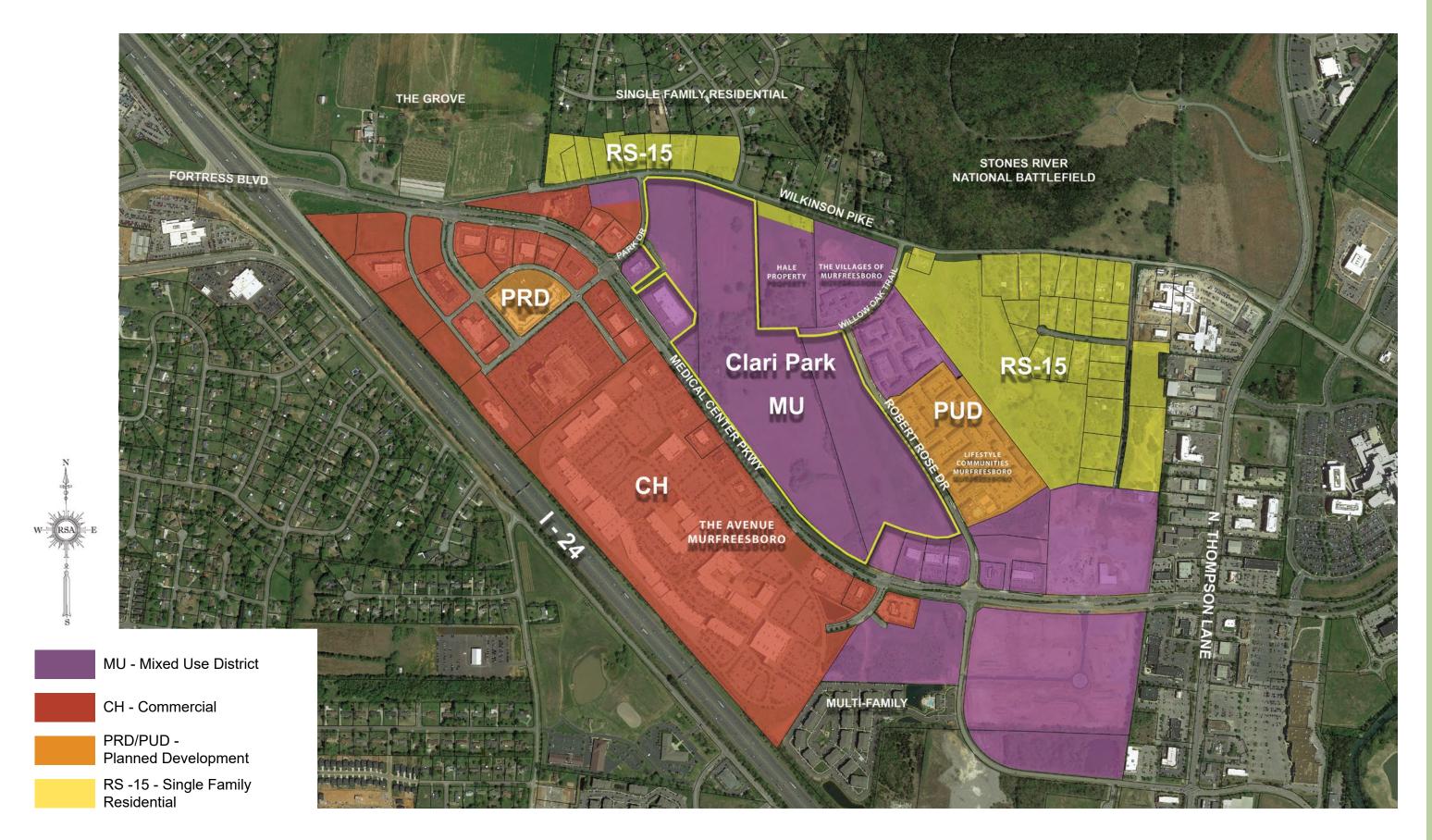
Location Map

The site for Clari Park is located in the heart of the Murfreesboro Gateway in close proximity to the Medical Center Parkway / I-24 interchange. It is surrounded by an interesting and rich mixture of existing land use with the Avenue Lifestyle mall to the south-west, high density lifestyle apartments to the east (Henley Station) and the historic Stones River Battlefield and residential neighborhoods to the north-east.



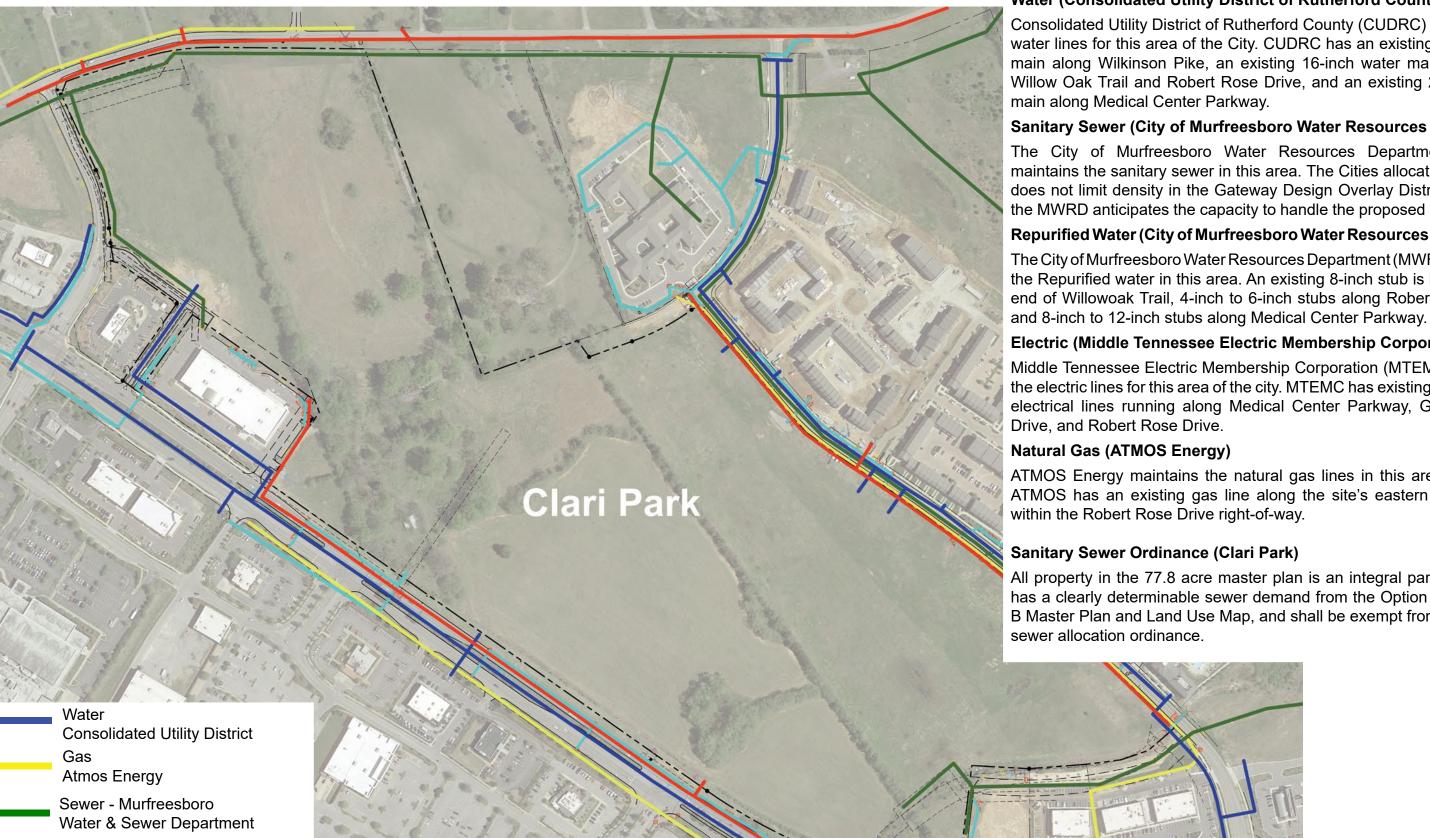


Existing zoning for the site is Mixed Use (MU) with surrounding zoning to the south, east, and west comprised of Mixed Use (MU), Commercial Highway (CH), and Planned Unit Development (PUD). RS-15 zoning is adjacent to the north side of the site.



Electric MTEC

Re Purified Water



300

Overlays and Flood Zones

Flood zone information taken from FIRM maps

Panel 255 of 479 map number 47147C0255H

Panel 260 of 457 map number 47149C0260H

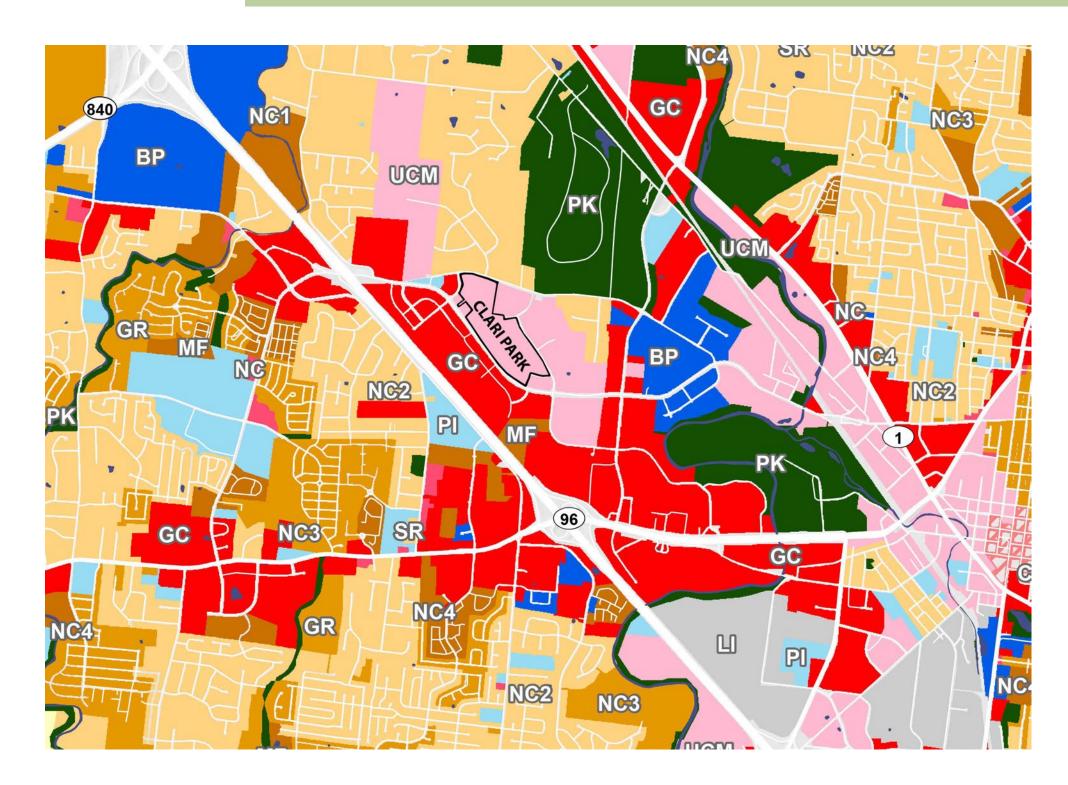
GDO - 1

300

As provided by FEMA



Future Long Range Plan



The Future Land Use Map designates the area of Clari Park as a Mixed-Use Corridor with urban, commercial, and mixed use character.

Mixed Use Corridor defined:

Allows a broad range of commercial, office and high-density residential uses and public spaces serving surrounding neighborhoods, commercial / professional business parks and visitors from nearby communities.

Suggested intensity / height guidelines for mixed use corridor in the future long range plan include:

1.85 FAR (approximately 60 DU/AC or 50-130 residents/acre), of which up to 0.50 FAR can be office or commercial / up to four stories.

City zoning districts suggested to match the mixed use corridor include:

Central Business District (CBD) Mixed Use District (MU) Planned Unit Development (PUD)

The proposed master plan for Clari Park is in keeping with the Future Long Range Land Use Map and its associated objectives as a Mixed Use Corridor. It speaks to the high level of infrastructure and quality of design that has been invested into the Murfreesboro Gateway. This location is very well suited for a mixture of high density uses and a mixture of residential options to feed into the growth and commerce of the gateway.

Legend



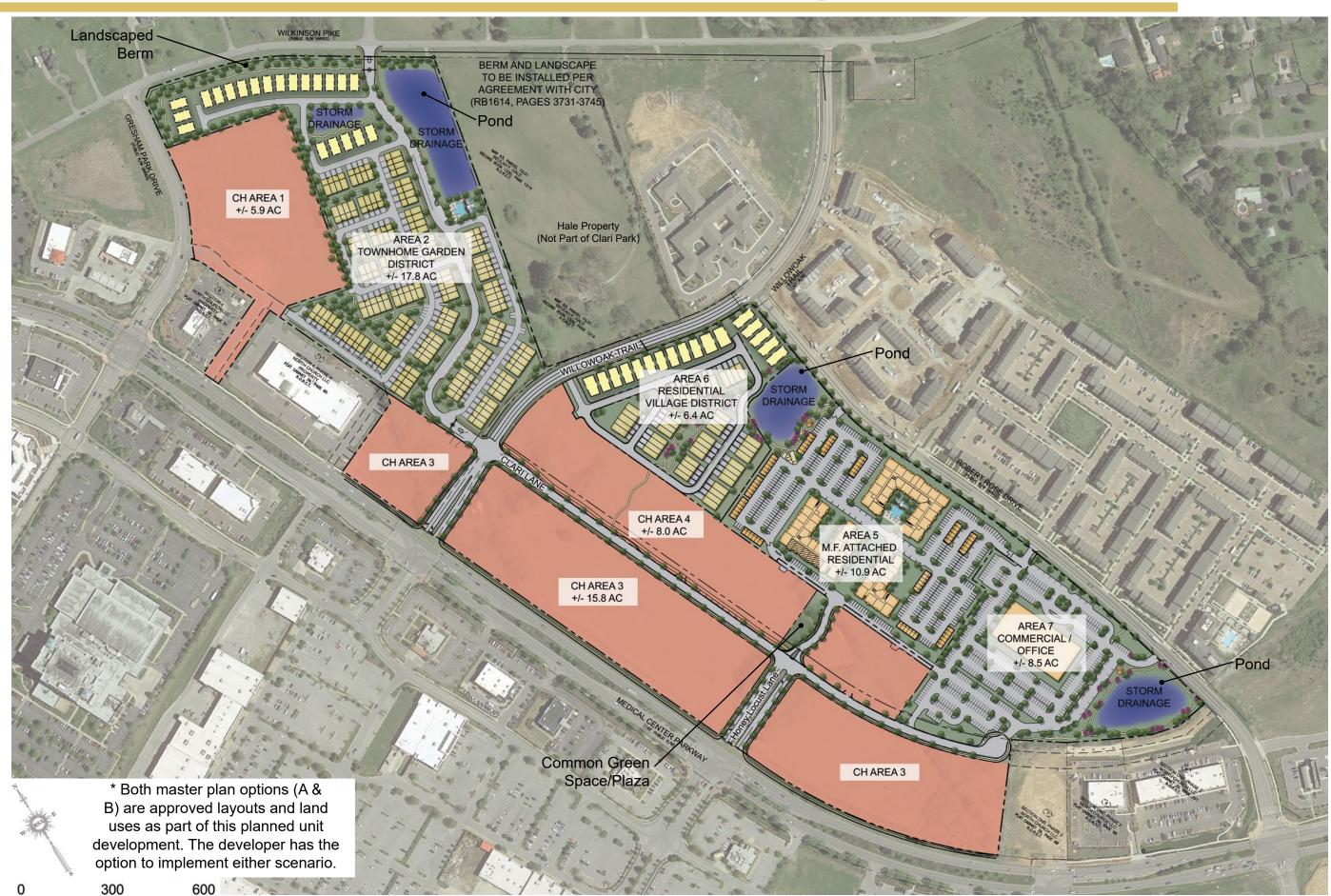


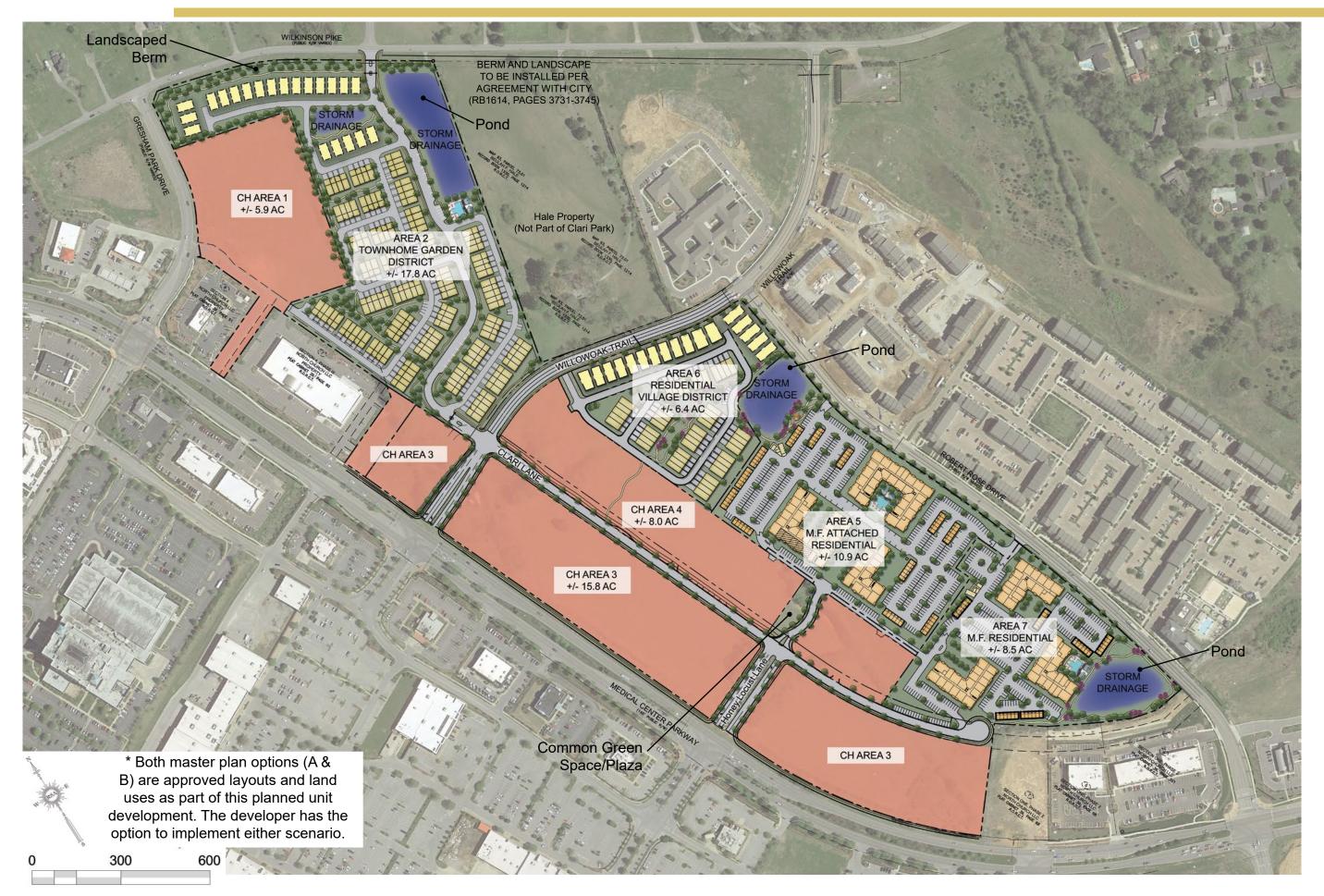
Suburban Estate

Undeveloped

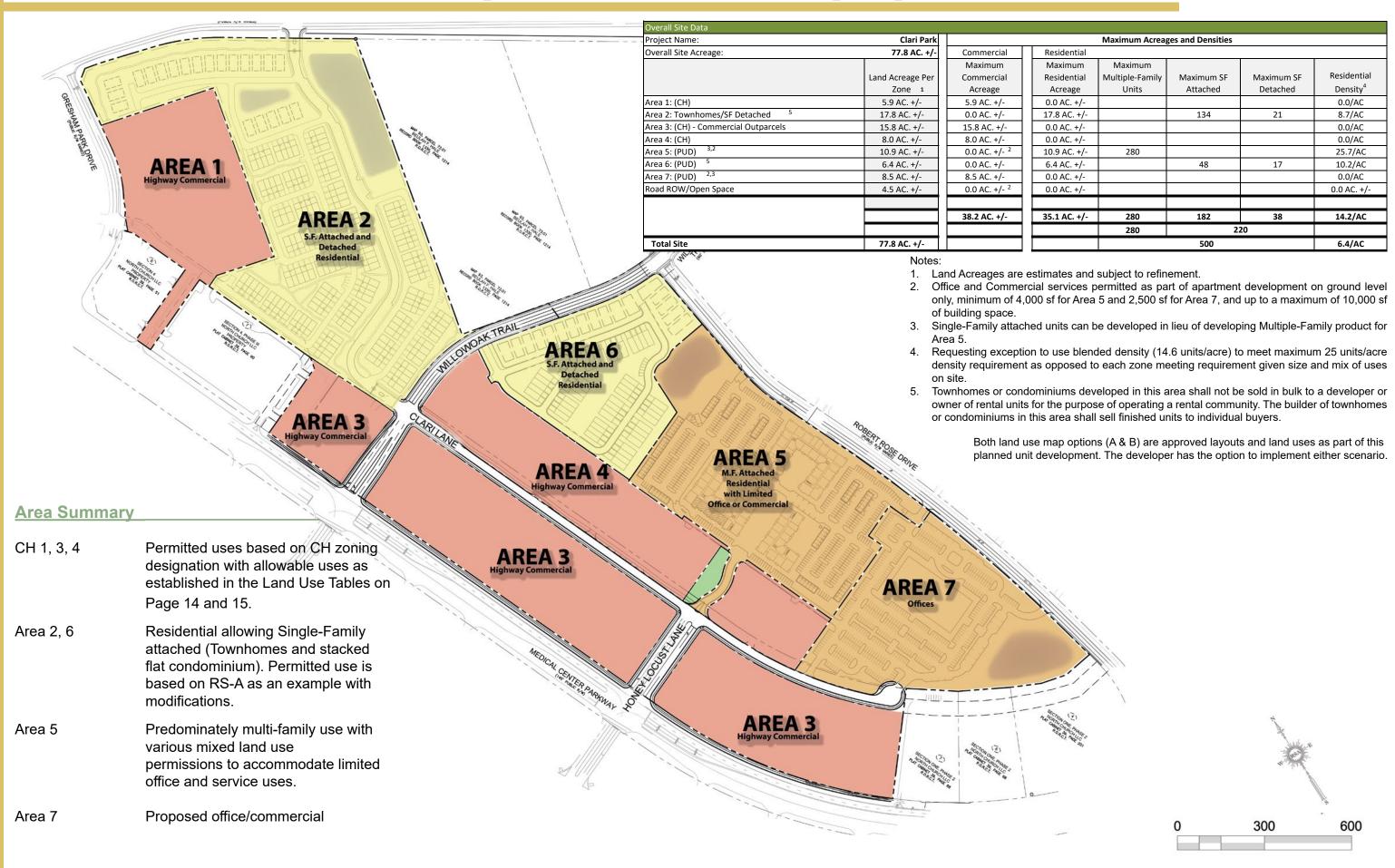
Suburban Residential

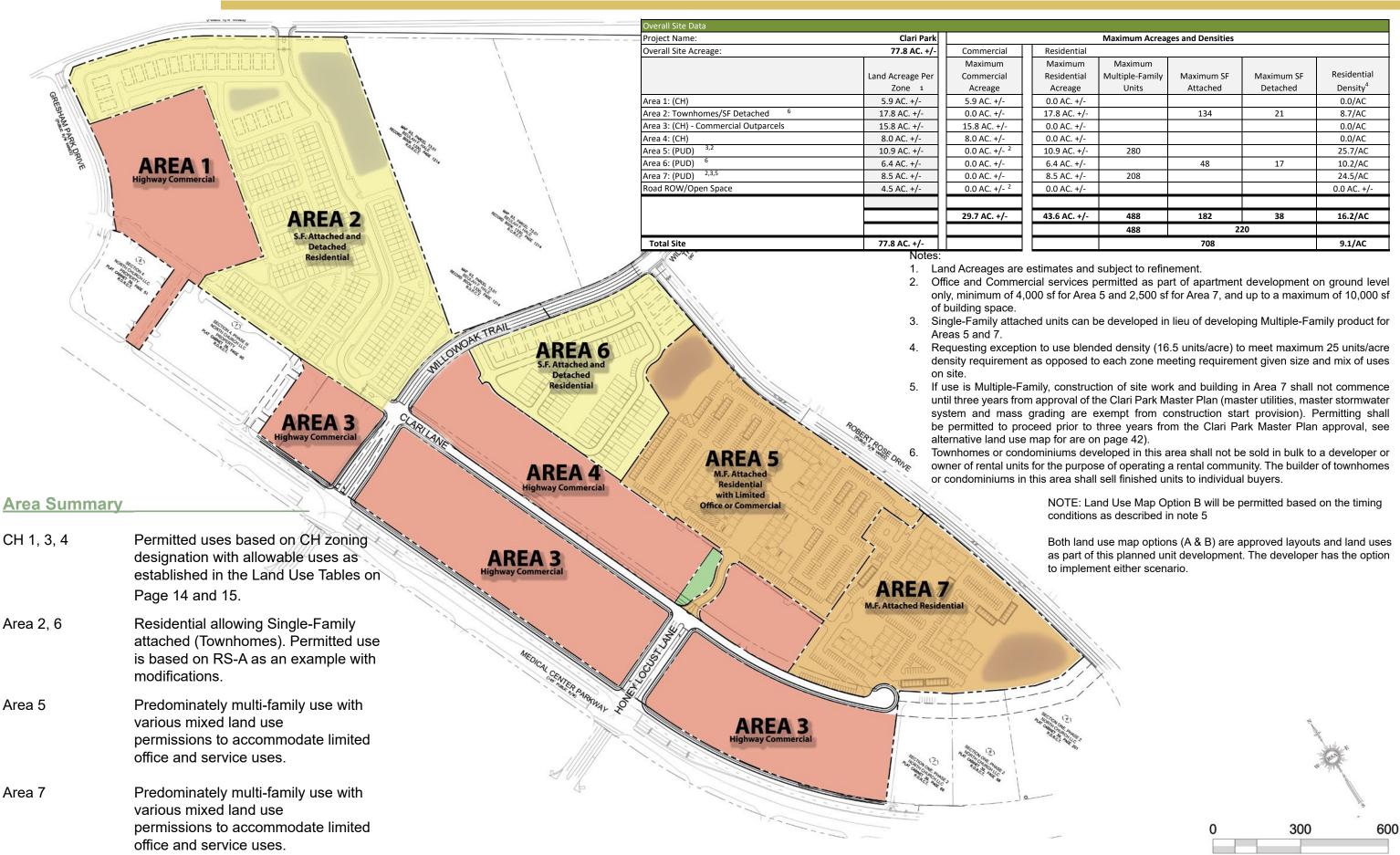
Urban Commercial / Mixed-Use





Proposed Land Use Map Option A (Office Use in Area 7)





Area 6

(PUD)

Χ

Χ

Area 7

(PUD)

Χ

Χ

Χ

Χ

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Clari Park Land Use 7						
USES PERMITTED		LAND USE AREA ¹				
	Area 1 (CH)	Area 2 (PUD)	Area 3 (CH)	Area 4 (CH)	Area 5 (PUD)	
DWELLINGS (RESIDENTIAL)						
Single-Family attached ²		Х			Х	
Multiple-Family					Х	
OTHER HOUSING						
Assisted-Care Living Facility	X			X	X	
Class III Home for the Aged	Х			Х	Χ	
Hotel	X			Χ	Χ	
INSTITUTIONS						
Adult Day Care Center	X			Χ		
Church	X			Х		
College, University	X			Х		
Day-Care Center	X		Х	Х		
Family Day-Care Home	X		Χ	Χ		
Group Day-Care Home	X		Х	Х		
Hospital	X		Х	Х		
Museum	X			Х		
Nursing Home	X			X		
Nursery School Park	X			X		
Philanthropic Institution	X	Х		X	Х	
COMMERCIAL	Х			Χ		
Amusements, Commercial Indoor	V			V		
Amusements, Commercial Outdoor excluding Motorized	X			X		
Animal Grooming Facility	X		V	X		
Art or Photo Studio or Gallery	X		X	X	Х	
Bakery, Retail	X		X	X		
Bank, Branch Office	X		X	X		
Bank, Drive-Up Electronic Teller	X		X	X		
Bank, Main Office	X		X	X		
Barber or Beauty Shop	X		X	X		
Book or Card Shop	X		X	X	Х	
Business and Communication Service	X		X	X	,	
Catering Establishment	X		Х	X		
Clothing Store	X		X	X	Х	
Coffee, Food, or Beverage Kiosk ⁶	X			X	Х	
Commercial Center	X		Х	X		
Convenience Sales and Service, maximum 5,000 sq. ft. floor area	X		Х	X		
Delicatessen	X		Х	X		
Department or Discount Store	X			Х		
Dry Cleaning	Х		Χ	Χ		
Dry Cleaning Pick-Up Station	Х		Χ	Χ	Х	

Χ

Χ

Х

Χ

Χ

Χ

Χ

Notes

- 1. Area 7 is generally based off Mixed-Use Zoning designation from 2020 Zoning Ordinance with minor modifications.
- 2. Single-Family attached generally refers to townhome and stacked flat condominium uses.
- 3. Restaurants that primarily promote food consumption within motor vehicles on the premises will not be permitted.
- 4. Financial services permitted include banks, financial advisors, investment management services, tax-preparation services and other similar type financial services. "Pay-day loan" services and cash advance facilities will not be permitted.
- 5. Garden and lawn supply operations shall display merchandise indoors. No outdoor storage shall be permitted.
- 6. Kiosk use will be restricted to "walk-up" style kiosk operations in open space or park settings. Vehicular drive-up use is prohibited.
- 7. Allowable land uses in CH Areas, 1, 3, and 4 are limited to those noted in this Land Use Table. These restrictions will also be recorded in public records via covenants & restrictions.
- 8. Gas stations and Convenience Sales will only be permitted in Area 3 for lots with frontage on Clari Lane and adjacent to Area 4 on the Master Plan.

Financial Service⁴

Flower or Plant Store

Garden Lawn Supplies and Hardware (Only in Area 3 adjacent to Area 4⁵

Land Use Table

Clari Park c

USES PERMITTED	LAND USE AREA ¹						
	Area 1 (CH)	Area 2 (PUD)	Area 3 (CH)	Area 4 (CH)	Area 5 (PUD)	Area 6 (PUD)	Area 7 (PUD)
Gas Station	X		Х				
Health Club	Х		Х	Х			Х
Interior Decorator	Х		Х	Х	Х		Х
Karate, Instruction	X		Х	Х			Х
Keys, Locksmith	X		Х	Х			Х
Laboratories, Medical - Exclude Plasma Donation Center	X		Х	Х			X
Laboratories, Testing	X		Х	Х			X
Liquor Store (No Drive-Thru)	Х		Х	Х			Х
Movie Theater	X			Х			X
Music or Dancing Academy	X		Х	Х			X
Offices	X		X	X	X		X
Optical Dispensaries	X		Х	Х			X
Personal Service Establishment (Hair, Nails)	Х		Х	Х	X		X
Pet Shops	Х		Х	Х			X
Pharmacies	Х		Х	Χ			X
Reducing and Weight Control Service	X		Х	Х			Х
Restaurant and Carry-Out Restaurant	Х		Х	X			X
Restaurant, Drive-In ³	X		X	Х			X
Restaurant, Specialty	X		Х	Х			X
Restaurant, Specialty -Limited	Х		Х	Х			X
Retail Shop, other than enumerated elsewhere	X		Х	Х			X
Shopping Center, Community	X		X	Χ			X
Shopping Center, Neighborhood	X		X	X			X
Veterinary Office	Х		Χ	Χ			Х
Veterinary Clinic	Х		Х	Х			Х
OTHER							
Home Occupations	Х	Х		Х		Χ	Х

Notes

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- 8. Gas stations and Convenience Sales will only be permitted in Area 3 for lots with frontage on Clari Lane and adjacent to Area 4 on the Master Plan.

c Clari Park

Land Use Parameters

Land-Use Parameters								
	Area 1 (CH)	Area 2 (PUD)	Area 3 (CH)	Area 4 (CH)	Area 5 (PUD)	Area 6 (PUD)	Area 7 (PUD)	
RESIDENTIAL DENSITY						, ,		
Maximum Dwelling Units Multiple-Family	0		0	0	280		208	
Maximum Dwelling Units Single-Family attached	0	134	0	0	0	48	0	
Maximum Dwelling Units Single-Family detached		21	0	0	0 17		0	
Minimum Lot Area	none	N/A - Units will have Horizontal Property Regime (HPR)	none	none	5 acres for mulitiple-family N/A for all other uses	N/A - Units will have Horizontal Property Regime (HPR)	5 acres for mulitiple-family N/A for all other uses	
Miniumum Lot Width	N/A for all other uses	N/A - Units will have Horizontal Property Regime (HPR)	50' min. lot width on Medical Center Pkwy.	N/A	N/A	N/A - Units will have Horizontal Property Regime (HPR)	N/A	
MINIMUM VARR REQUIREMENTS								
MINIMUM YARD REQUIREMENTS								
Minimum Front Yard Porches, stoops, and bay windows may extend into setbacks. Min. front yard shall be measured from all public roads on corner lots	42'	15'	42'	42'	15'	15'	15'	
Minimum Side Yard Porches, stoops, and bay windows may extend into setback S	10'	5'	10'	10'	10'	5'	5' for townhomes 10' for all other uses	
Minimum Rear Yard	20'	20'	20'	20'	20'	20'	20'	
LAND-USE INTENSITY RATIOS								
Max FAR	None	None	None	None	None	None	None	
Minimum Livable Space Ratio	None	None	None	None	None	None	None	
Minimum Open Space Requirement	20%	20%	20%	20%	20%	20%	20%	
Minimum Formal Open Space Requirement				creage and use as determined in				
Min Lot Coverage	None	None	None	None	None	None	None	
Max Height	45'	35'	75' 150' for Office, Hotel, and Hospital	75' 150' for Office, Hotel, and Hospital	45' for S.F. 75' for multiple-family uses 150' for commercial/office uses	35'	45' for S.F. 75' for multiple-family uses 150' for commercial/office uses	
Parking Ratio	Single-Family Attached and Multiple-Family Uses 1.1 space per bedroom Single-Family Detached Uses 4 spaces per unit (includes garage spaces) All Other Uses: Per "Chart 4" of 2020 Zoning Ordinance.				Parking spaces within garages for Single Family Attached Residential and Multi-Family Residenti will be considered as meeting parking count requirements. They will not be used for the parking of storage of boats, recreational vehicles, trailers, or equipment.			
	_							

Landscape Yard Minimums and Building Setbacks							
Roadway	Minimum Landscape Yard	Building Setback	Notes				
Medical Center Parkway	25'	50'	Arterial Road				
Robert Rose Drive	15'	15'	Local Road				
Wilkinson Pike	30'	30'	Berm shall be constructed within 30' buffer per Agreement with City (RB1614, pgs 3731-3745) prior to or as part of initial phase of construction No building exceeding 3 stories in height shall be erected within 100 feet of the South right of way. No apartment development shall be placed on Property (within 100' of Wilkinson Pike) unless approved by the Planning Commisssion and the City Council as a Planned Development.				
Willow Oak	15'	15'	Local Road				
Clari Lane (to be named) (Road behind outparcels)	15'	15	Local Road				

Requested Exceptions

Exception

- Requesting "Single Family Attached" Residential and "Single Family Detached" Residential Use be permitted (Not currently permitted in underlying MU zoning)
- 2. Requesting exception to maximum 25.6 units peracre density requirement for area 5. The average residential density allowed for the overall master plan for residential parcels in Clari Park is approximately 16.6 units per-acre
- 3. Request adjustment to parking ratio requirement for 1-bedroom residential multifamily units of 1.5 spaces per bedroom to 1.1 per bedroom and removal of parking requirement for up to 10,000 sf of office space on first floor of each Multifamily project.
- 4. An exception to allow outdoor sales and the sale of food and beverage in park space and public open space for temporary special events
- 5. Porches, stoops, and bay windows may extend into setbacks

Community Management & Operations

Clari Park c

As the Master Developer of Clari Park, the Hines development management team will implement development management and operations controls to ensure that the community is developed and managed in accordance with the approved Mixed Use and Commercial Highway Zoning Master Plan and to implement the vision of the Master Developer and design team. Elements of the management and operations are:

Development Management - The Master Developer will manage the design, permitting, construction and close-out of the horizontal infrastructure within Clari Park including mass grading, utilities, stormwater management and roadways. Development management by the Master Developer will be performed directly for the Hines development venture and on behalf of the town home builder.

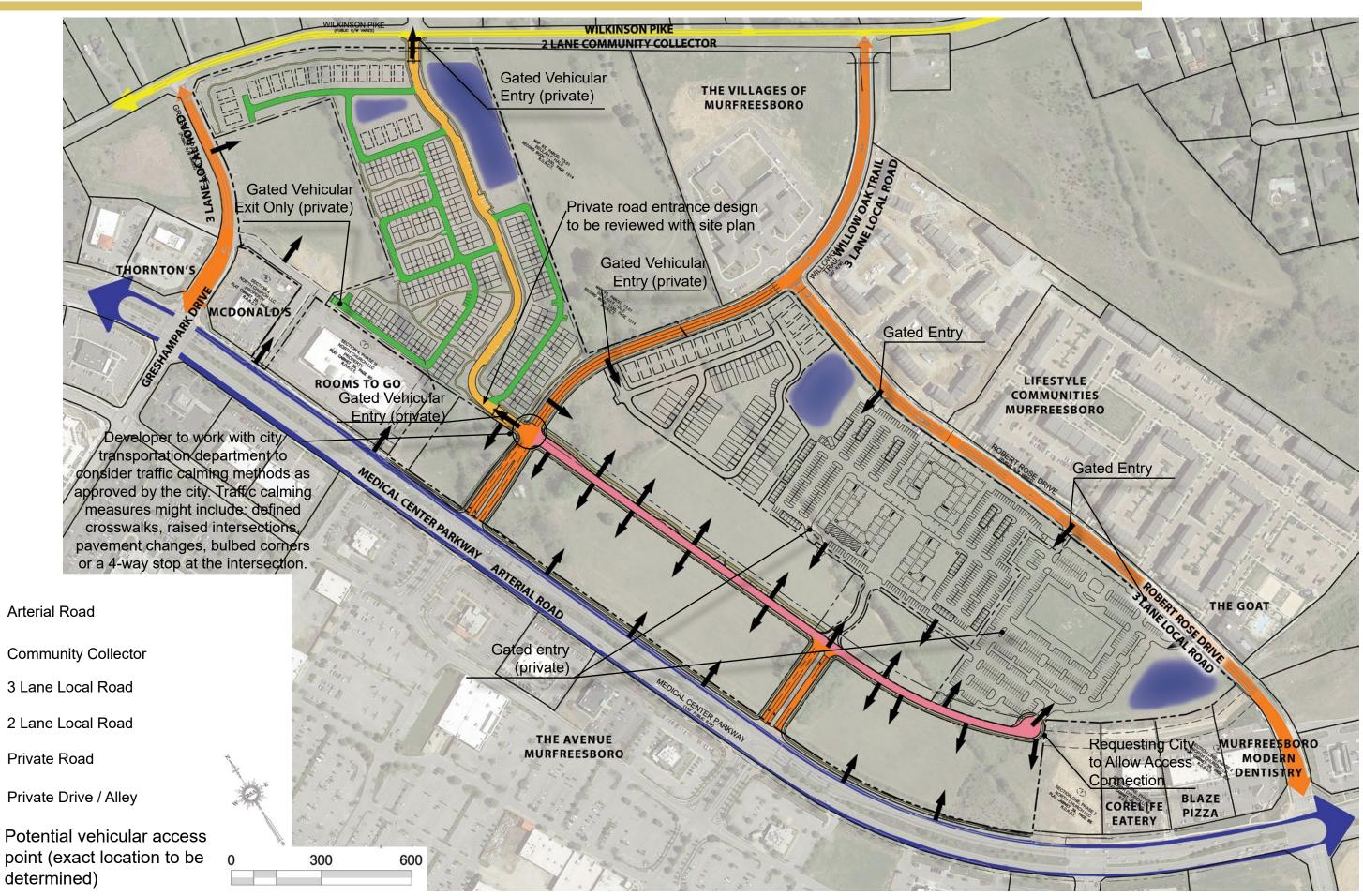
Commercial Land Sales - Hines will directly negotiate and transact all commercial land sales and incorporate provisions that require compliance with the zoning regulations within such sale contracts. Purchase agreements will obligate commercial properties to be subject to architectural review and compliance with the Clari Park Covenants and Restrictions

Site Plan Reviews - The Master Developer will work through an iterative site plan design process with the commercial parcel and town home parcel owners to ensure that all site plans are consistent with the overall site planning and landscape themes of Clari Park including strong pedestrian connectivity.

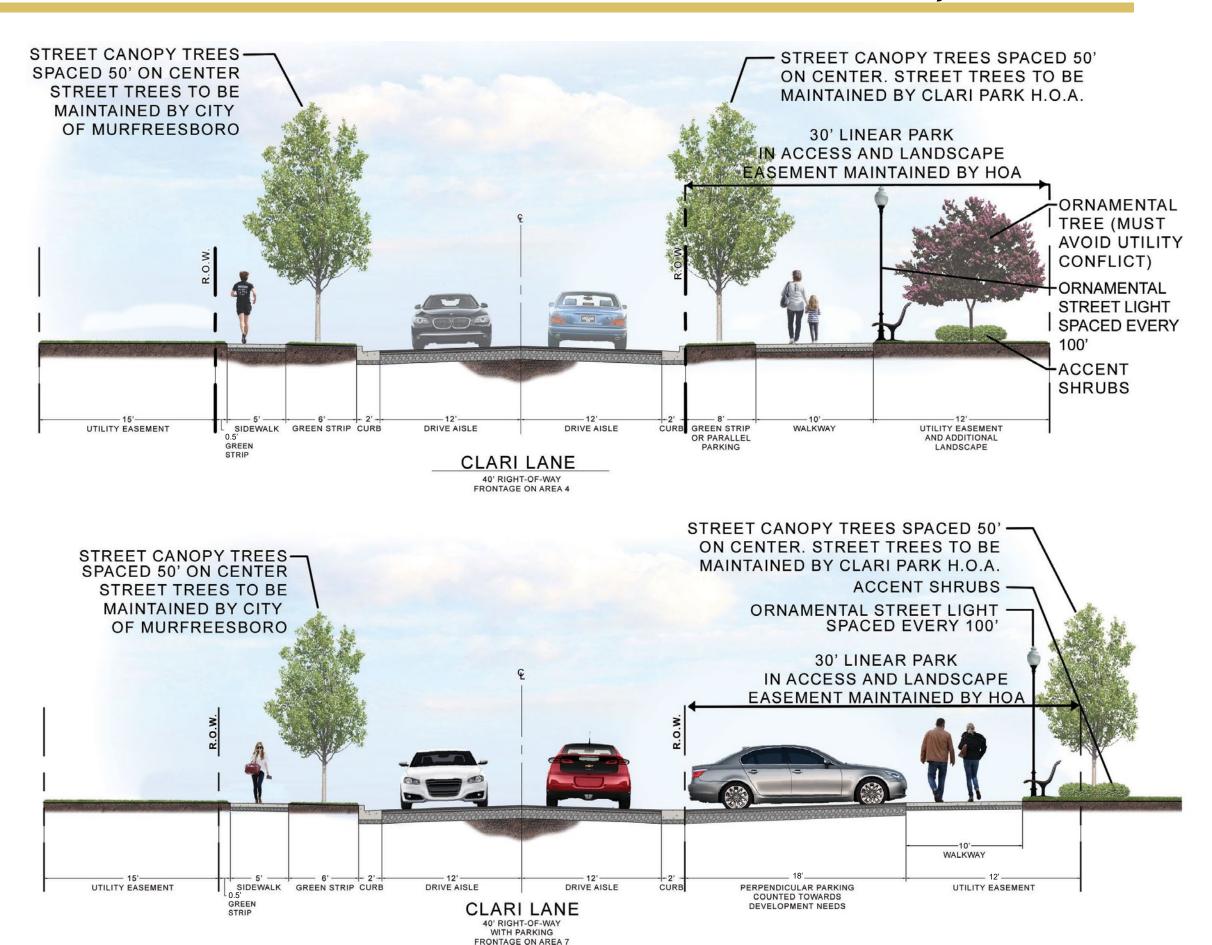
Architectural and Landscape Design Review - The Master Developer will create and coordinate the activities of an architectural review committee that will review the building plans for all commercial parcel owners within Clari Park. Commercial owners will be encouraged to submit preliminary design concepts for an initial review prior to formalizing purchase contracts with the formal review taking place thereafter. The Committee will include a registered architect and landscape architect in addition to Hines team members.

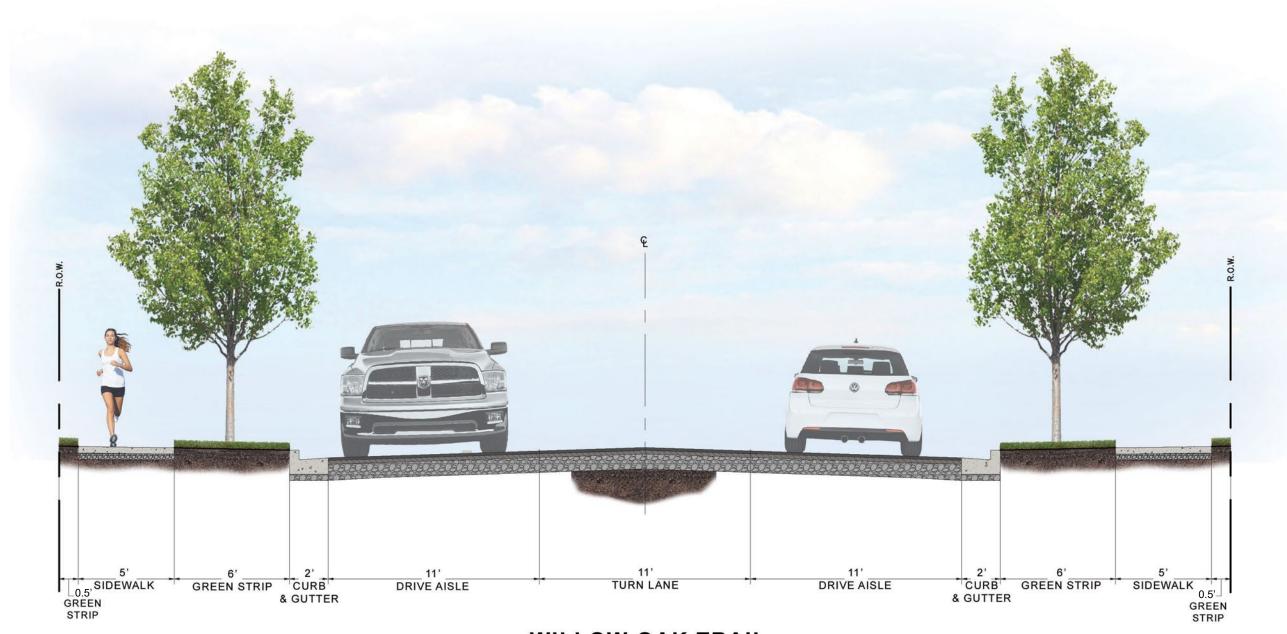
Property Management / Covenants and Restrictions - Property management associations will be created for the commercial and residential properties within Clari Park with covenants and restrictions that are enforceable by these associations. Standards for the maintenance of common area and private properties will be established in the covenants and enforced by the associations with Master Developer providing oversight and coordination throughout the development period. The covenants and restrictions shall expressly provide the right for the property management company to tow vehicles that are not parked in legal common area or private spaces and shall further obligate to the property management company exercise this right when notified by residents or by the city.

Vehicular Transportation Network







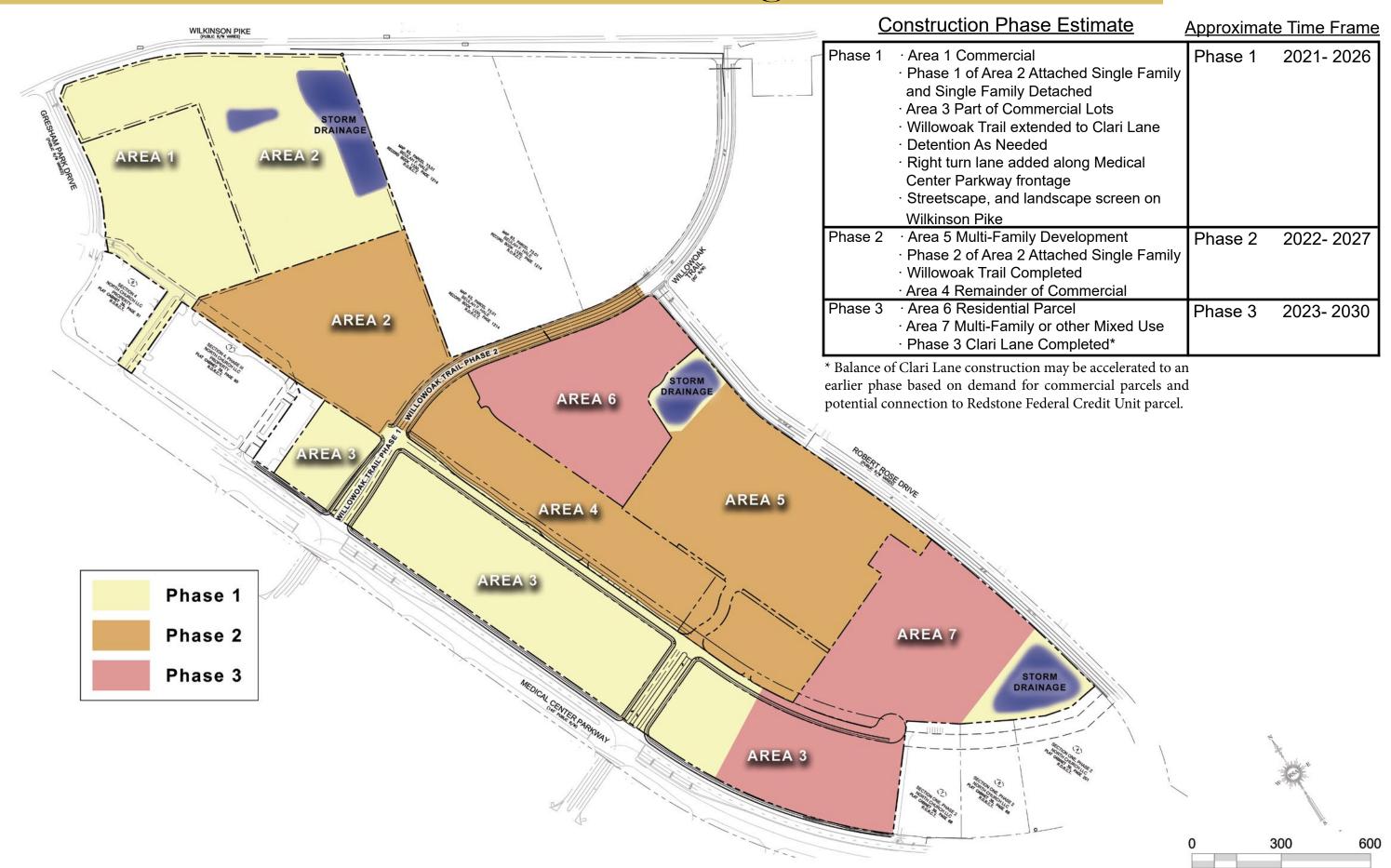


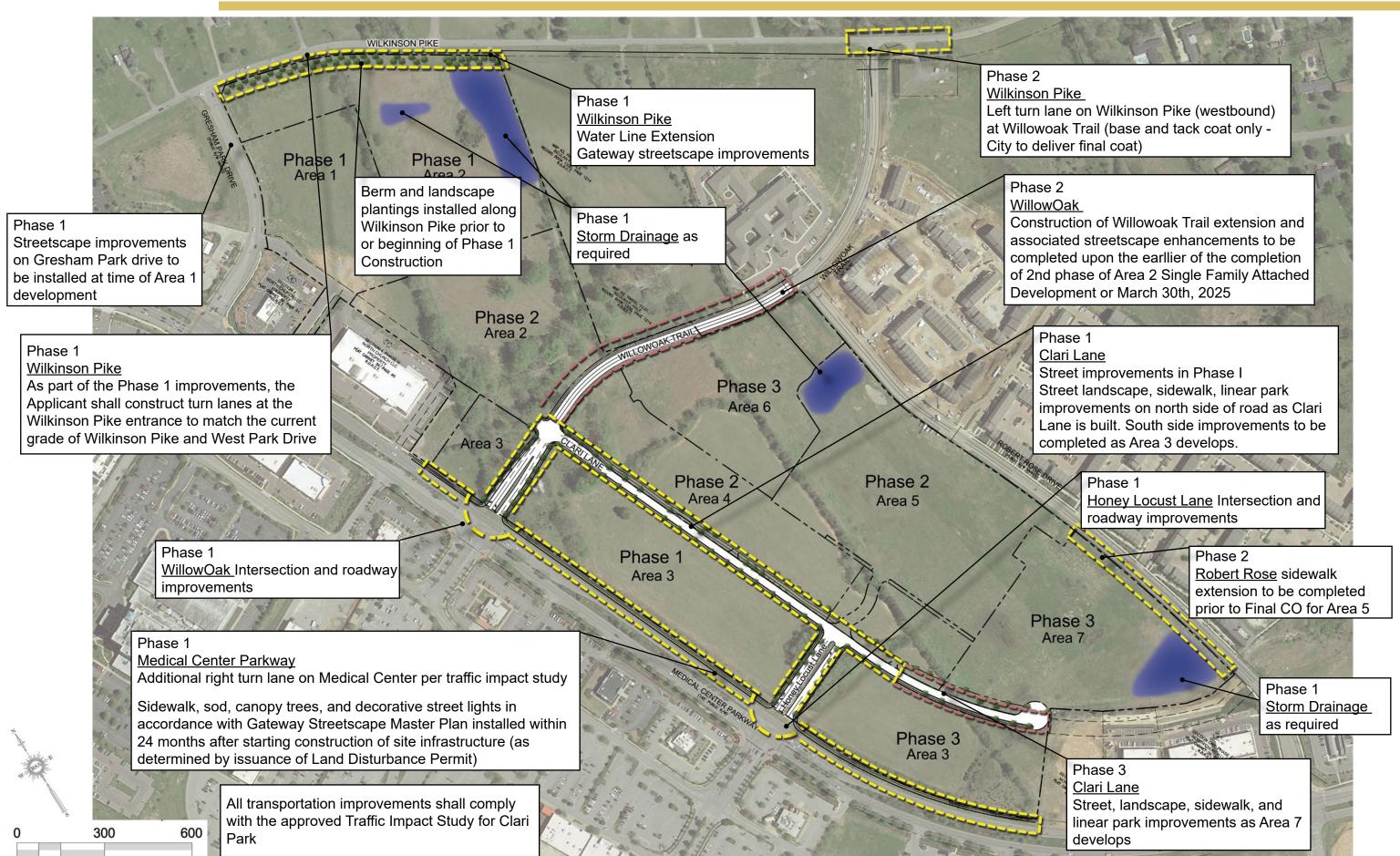
WILLOW OAK TRAIL

60' RIGHT-OF-WAY

Right of way will vary at intersections with Medical Center Parkway based on turn lane requirements

Phasing General Master Plan





Clari Park

Public Improvement Phasing

WillowOak Trail

Phase 1

- WillowOak Trail at MCP intersection improved with egress lanes / turn lanes as shown on public improvements plan.
- WillowOak Trail street improvements from Medical Center Parkway to Clari Lane intersection. Landscape and sidewalk improvements from Medical Center Parkway to Clari Lane intersection.
- WillowOak Trail and Medical Center Parkway intersection improvements in accordance with gateway streetscape master plan. (Seat wall, pedestrian plaza, and crosswalk)

Phase 2 or 3

- WillowOak Trail street improvements and public streetscape enhancement from Clari Lane to Robert Rose Drive. Construction of Willowoak Trail extension and associated streetscape enhancements to be completed upon the completion of 2nd phase of Area 2 Single Family Development or March 30th, 2025, whichever is earlier.
- Applicant will commit to connect southern end of Clari Lane with adjacent bank parcel should a legally and commercially feasible solution be presented to do so. Applicant will work with City and adjacent landowner to explore viability of such a solution.

Wilkinson Pike

Phase 1

• As part of the Phase 1 improvements, the Applicant shall construct turn lanes at the Wilkinson Pike entrance to match the current grade of Wilkinson Pike and West Park Drive and a left turn lane from Wilkinson Pike onto Willowoak Trail

Honey Locust Lane

Phase 1

- Honey Locust Lane at MCP intersection improvements with egress lanes / turn lanes as shown on public improvement plan.
- Honey Locust Lane street improvements from Medical Center Parkway to Clari Lane intersection. Landscape ornamental lighting and sidewalk improvements from Medical Center Parkway to Clari Lane intersection.
- Honey Locust Lane and Medical Center Parkway intersection improvements in accordance with gateway streetscape master plan. (Seat wall, pedestrian plaza, and crosswalk)

Clari Lane

- Phase 1 and 2 Clari Lane street improvements from Willow Oak Trail to end of phase 2. (Frontage on Area 4)
 - Streetscape and Linear Park improvements fronting Area 4 to be completed with initial construction of Clari Lane. South side improvements to be completed as Area 3 develops.

Phase 3

 Clari Lane street improvements, streetscape and linear park improvements fronting Area 7 and phase 3 commercial lots.

Medical Center Parkway / Robert Rose Drive

Phase 1

Additional right turn lane along all lots fronting Medical Center Parkway

Phase 1, 2, 3

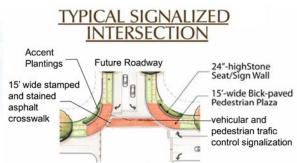
Sidewalk, sod, canopy trees, and decorative street lights in accordance with the Gateway Streetscape Master Plan installed within 24 months after starting construction of site infrastructure (as determined by issuance of Land Disturbance Permit)

Phase 2

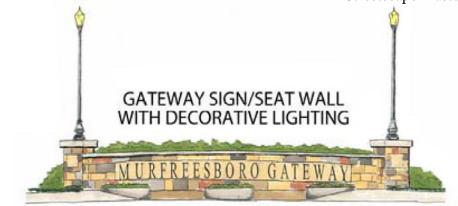
Robert Rose sidewalk extension to be completed prior to Final CO for Area 5

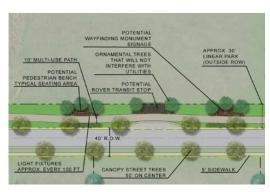


WillowOak Trail street improvements Gateway Seat wall(s) at intersection See page 22



Gateway intersection at Willow Oak and Medical Center Parkway and Honey Locust Lane and Medical Center Parkway See page 13 of the Murfreesboro Gateway Streetscape Masterplan

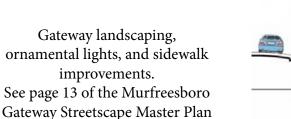


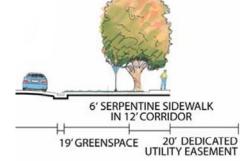


Clari Lane Linear Park **Improvements** See page 27



Clari Lane Street Improvements See page 21





Connection of open space § with linear parks in § **Garden Townhome District** Phase 1





300

600

Linear Park along east side of Clari Lane (see page 28) Phase 1



Note: All areas within Clari Park will meet the minimum formal and informal open space requirements associated with each site at the time of development.

Public pedestrian and bicycle access will be provided at entrances. Sidewalks will be available for public use

Pond

Historic Markers and locations for potential public art along **Clari Lane Corridor - Specific** design to be determined.

> Pedestrian connections to **Robert Rose Drive and Commercial areas**



Grand Lawn and Linear Park at Area 5 (see page 29)



Phase 2 **Commercial Lawn** (see page 30)

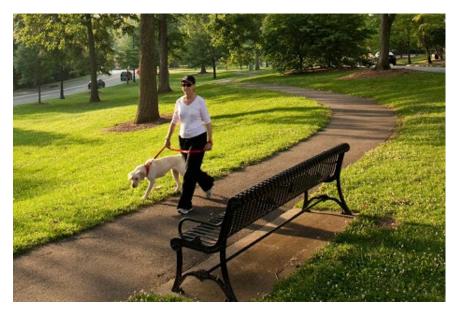
Clari Lane Linear Park

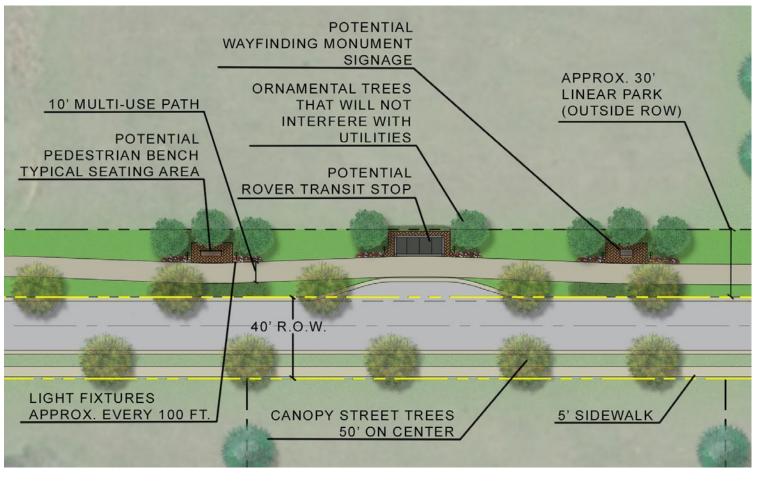
Clari Lane Linear Park

Clari Lane will function as the main street of Clari Park by connecting the different land use areas. Through the provision of green space running parallel to the street. Included in this green space will be a 10' wide multi-use path on the east side of the street, ornamental, pedestrian scaled streetlights, pocket parks with benches and site furniture. The landscape will include street canopy trees and ornamental trees. The concept design depicted for the linear park on Clari Lane is to confirm the streetscape elements and amenities that are part of the master plan. Specific layout and detailed design will occur when site plan and street improvement plans are developed.













Connection of Open Space

Open spaces and pedestrian networks will extend from the linear park along Clari Lane to other areas within the project and adjacent properties. This will facilitate pedestrian circulation within the site, to the commercial district along Medical Center Parkway, and the Avenue beyond. The pedestrian walkway between Area 5 and 6 provide a pedestrian and green space connection through the higher density portion of Clari Park. It also functions as part of the pedestrian network between Clari Lane and Robert Rose Drive, which will provide a connection to the Avenue through the project for Henley Station residents.

The connections in Area 2 (the Townhome Garden District) will include the incorporation of existing mature trees, proposed for preservation between Area 1 and Area 2. It will also connection pedestrian pathways to the front greenspace between residential buildings. This will link the front door residential homes to a comfortable pathway that leads to surrounding amenities and places of commerce.









Area 1 (Commercial Highway)

This area is reserved for mixed use related to commercial, and office. It has primary frontage on Gresham Park Drive as well as egress from Medical Center Pkwy. A larger commercial user can be accommodated on this parcel given the site's size and its visibility from the Gresham Park Drive and Medical Center Pkwy intersection and benefits from a large area for dedicated parking spaces. Residential uses are not permitted in area 1.

Area 4 (Commercial Highway)

This commercial development area provides flexibility in space and response to market conditions. The design form for this area could allow for integrated, shared parking, vertical development, and a mixture of commercial and office use. Area 4 has a strong relationship to the central spine of the project along Clari Lane and is well connected to the residential components of Clari Park. Residential uses are not permitted in area 4. This area could be well suited for the development of a hotel, community grocery, corporate office or entertainment type of development.

Commercial Uses Materials Palette (Per Murfreesboro Design Guidelines)

- Primary material
 - Brick (full thickness or thin-set)
 - Cast stone
 - Natural or synthetic stone
- Secondary materials
 - Exterior Insulation Finish System (EIFS)
 - Split-face or ground-face, or polished-face concrete masonry (integrally colored)
 - Architectural metal panels with durable finish and defined profile
 - Composite panels
 - Cementitious siding or panels
 - · Wood siding may be used on small scale buildings
 - Fabric Awnings
- Tertiary materials:
 - Metal copings, flashings, and trim
 - Wood or cementitious trim
- Prohibited materials
 - Smooth-face concrete masonry
 - Corrugated metal "R" panels





These photographic examples depict general concepts of building architectural character in Areas 1 and 4. They are not intended to depict final architecture or site design and they do not capture every use or scenario permitted in these areas.



e Clari Park Commercial Highway (CH) - Photographic Examples (Areas 1 & 4)

Architectural standards set forth in the Murfreesboro Design Guidelines and GDO standards referencing general character, heights and setbacks, building mass, scale and proportion, building composition and rhythm, transparency, articulation and expression, materials, color, and roof design will be taken into account with the design of this project.











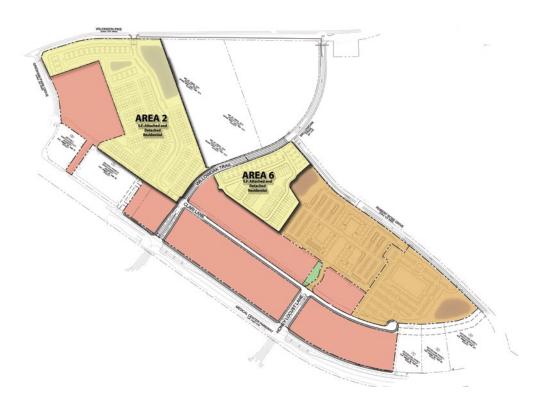


Area 2 is the residential Garden District of Clari Park. It has ingress & egress from Wilkinson Pike and an extension of the local street network off Willow Oak Trail and both access points are permitted to be gated. It provides good opportunity for a mixture of residential housing options that include attached and detached single family. Single family homes in this district will have horizontal property regimes with side by side units sharing a common lot area. Some residential units are designed to front on green spaces and parks with vehicular access through an alley network in the rear. Buildings are limited to 3 story (35') to respect the context of Wilkinson Pike and the single family residential to the north. Attached and detached single family is part of the Clari Park Master Plan to help meet the market demand for homes that integrate into the local commerce and invested infrastructure of The Gateway. This form of homes appeals to residents who want to be part of a walkable community close to the surrounding retail and restaurant amenities. Attention is given to architectural details to relate to a residential and pedestrian scale and buildings are arranged to connect to common open space and linear parkways. Street networks in area 2 have the potential for being public or private and parking is predominantly designed to be at the rear of residential units.

Area 6 (Attached & Detached Single Family Residential Village District)

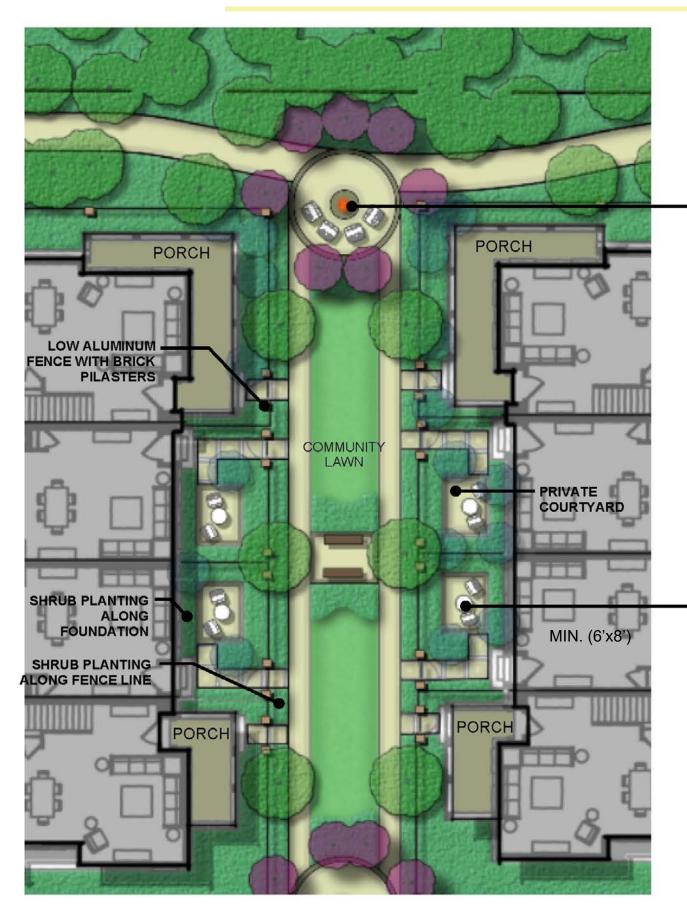
Area 6 is the Residential Village District with access points at Robert Rose Drive and Willow Oak Trail that are permitted to be gated. It serves as a transition zone from the commercial core to the surrounding land use north and east of Clari Park. Like Area 2 the attached and detached single family in Area 6 helps meet the market demand for homes that integrate into the local commerce and invested infrastructure of The Gateway. Attached and detached single family homes will have have horizontal property regimes units sharing a common lot area. Townhomes will be designed with arrangements that have a strong relationship to the street and green space. Streets are designed to have strong landscape elements and pedestrian space and may be public or private. This form of home appeals to residents who want to be part of a walkable community close to the surrounding retail and restaurant amenities and provides homeownership options for young urban professionals and "empty nesters".

All single family detached and attached residential units in Clari Park are proposed to be established with Horizontal Property Regimes



Residential Garden District







COMMUNITY FIRE-PIT / GATHERING SPACES





PRIVATE FENCED FRONT YARD WITH ON-GRADE PATIO

Note: All on-grade front patios shall be 8' wide and 6' deep and have a decorative fence

f Clari Park

Single Family Detached Amenity and Berm Enlargement

GAZEBO OVERLOOKING BERM **BY OTHERS** FIRE PIT • SEATING AREAS HOMES WITH ELEVATED FRONT PORCHES FRONT ON LANDSCAPED

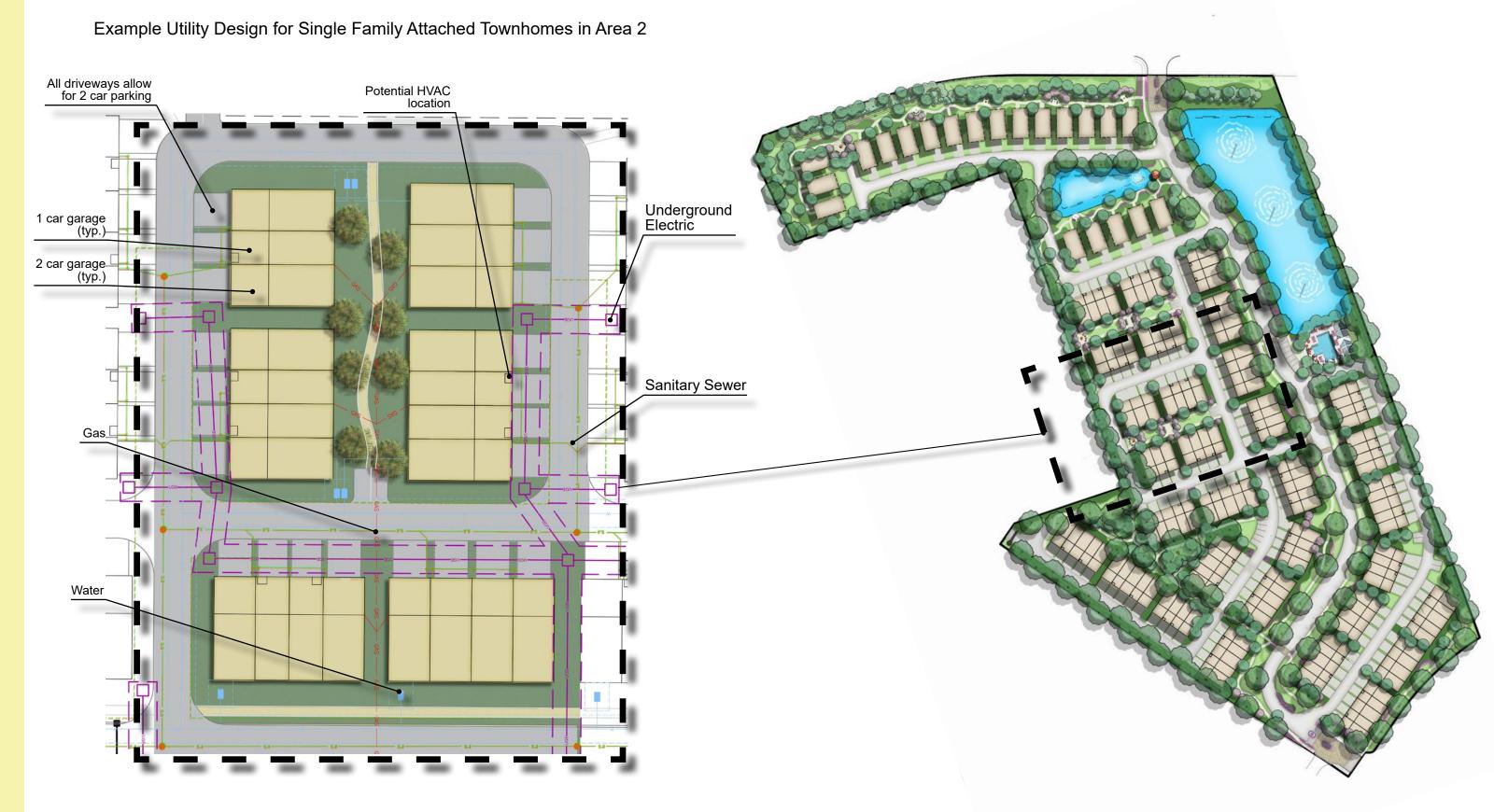
COMMUNITY FIRE-PIT

COMMON AREA GREEN SPACE PLAN

(Area 2 Residential Garden District)







Note: All single family detached units will have 2 car garages

Single Family Attached and Detached Specifications:

Building Construction and Design

- Single family homes shall be a minimum of 1,800 square feet (heated/cooled) and have a Exterior Building Materials minimum of three bedrooms. (Based on heated space and excluding garage)
- Townhomes dwelling units shall be a minimum of 1,400 square feet (heated/cooled) and have a minimum of two bedrooms. (Based on heated space and excluding garage)
- Buildings shall avoid long uninterrupted facades. Variations in the roof line including dormers and gables or wall plane shall be used to break up the mass of the building.
- Exterior details such as shutters, wall lanterns, louvers, dormers as appropriate to the architectural style shall be incorporated to add interest and richness to the front facades.
- Foundation planting landscape materials shall be provided along all four elevations as required by GDO and Murfreesboro design guidelines and front lawn areas shall be sodded per city design guidelines and GPO requirements
- Garages shall not be used for the storage of boats, recreational vehicles, trailers or equipment.
- · All dwelling units with attached garages shall locate access to the garage at the rear or side of the building with a maximum percent of 1-car units in Area 2 of 33%.
- Driveways for single family homes shall be a minimum of 16' wide to accommodate two cars
- Driveways for townhome dwelling units shall be a minimum of 16' wide to accommodate two cars and 8' wide to accommodate one car. Driveways shall be a minimum depth of 20 from access drive
- The incorporation of front patios, porches, bay windows and stoops shall be encouraged and shall be permitted to extend into the front yard and side yard setback.
- The finished floor of dwellings shall be designed such that the elevation is a minimum of 18" above the adjacent exterior grade at the front of the dwelling.
- Mechanical systems and above-grade utility elements shall be located in the rear or side of dwellings whenever possible with the exception of electrical and telecommunications equipment that will be placed in designated easements.
- Any on-grade front patios must be 8' long by 6' deep with decorative fencing
- Detached single family homes shall have a minimum 2 car garage
- Attached single family homes shall have a minimum of 1 car garage
- There shall be a maximum of 38% one car garage units for single family attached units in areas 2 and 6 at Clari Park

- The following exterior materials shall be permitted on the exterior façade:
 - 1. Brick veneer natural color or painted
 - 2. Cementitious and fiber cement composition siding (i.e. Hardie, Certainteed)
 - 3. Stone natural or manufactured stacked stone
 - 4. Wood siding in limited locations or trim elements nay be used if appropriate in context to the architectural style.
 - 5. Windows may be constructed of pre-finished aluminum, vinyl, or vinyl clad wood. Window mullions shall be provided appropriate to the architectural style.
 - 6. The use exposed concrete block, split-faced block, vinyl siding or corrugated metal siding shall be prohibited. Glass block is prohibited on the front elevation of dwellings. (Note: Vinyl may be used for exterior soffits and miscellaneous trim).
- The following exterior materials shall be permitted as roofing materials:
 - 1. Dimensional Composition Roof Shingles
 - 2. Metal roof in limited accent applications such as porches and bay windows if appropriate in context to the architectural style.
 - 3. Garage doors shall be carriage style or decorative
- All single family attached and detached units in Areas 2 and 6 will be established with a horizontal property regime

f Clari Park Architectural Examples - Townhomes Single Family Attached (Areas 2 & 6)



Architectural Examples - Townhomes Single Family Attached (Areas 2 & 6) Clari Park f







Clari Park Architectural Examples - Townhomes Single Family Attached (Areas 2 & 6)

Example Side and Rear Elevations







Decorative Garage Doors







Architectural Examples - Single Family Homes - Detached (Areas 2 & 6)

Example Side and Rear Elevations

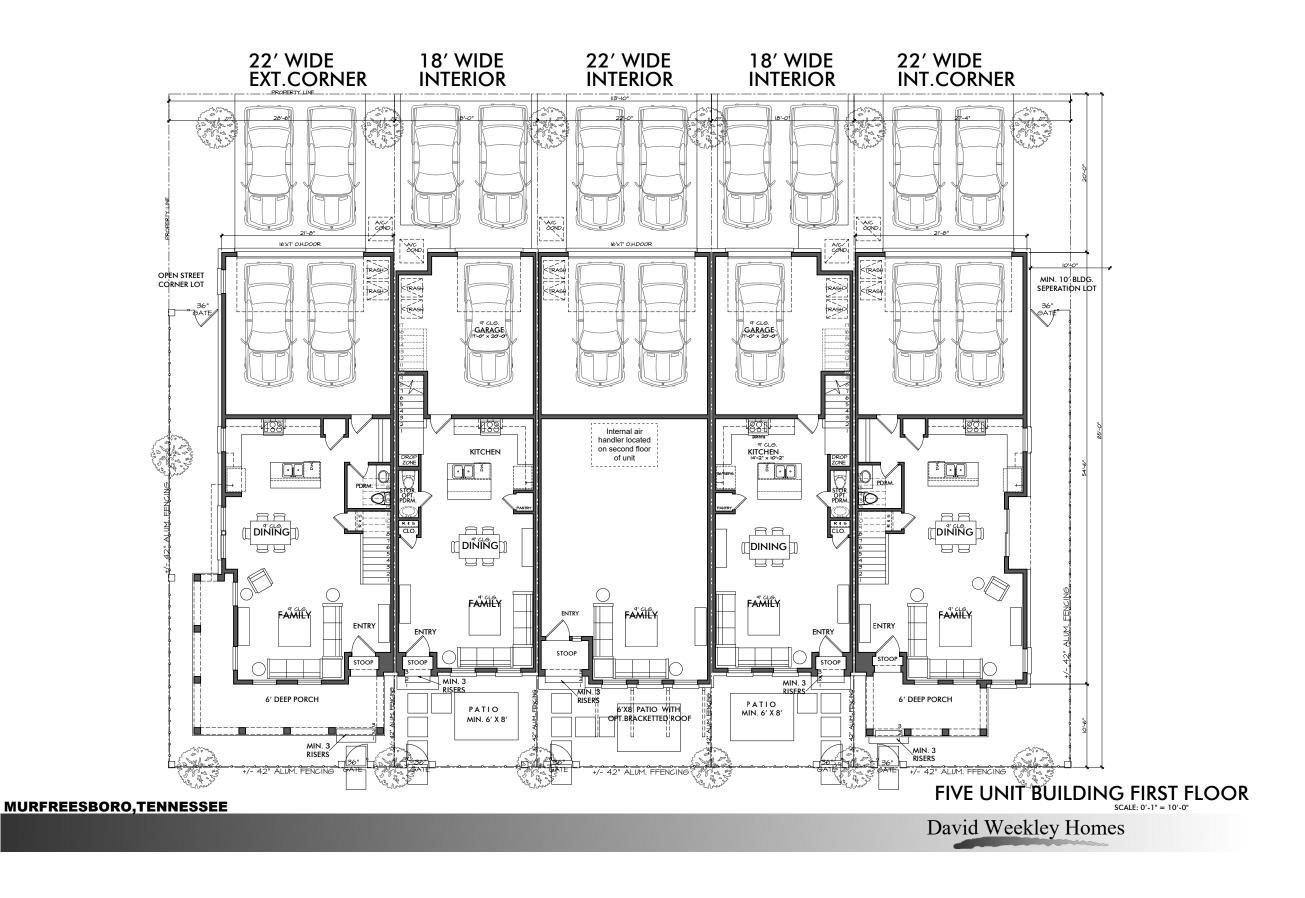


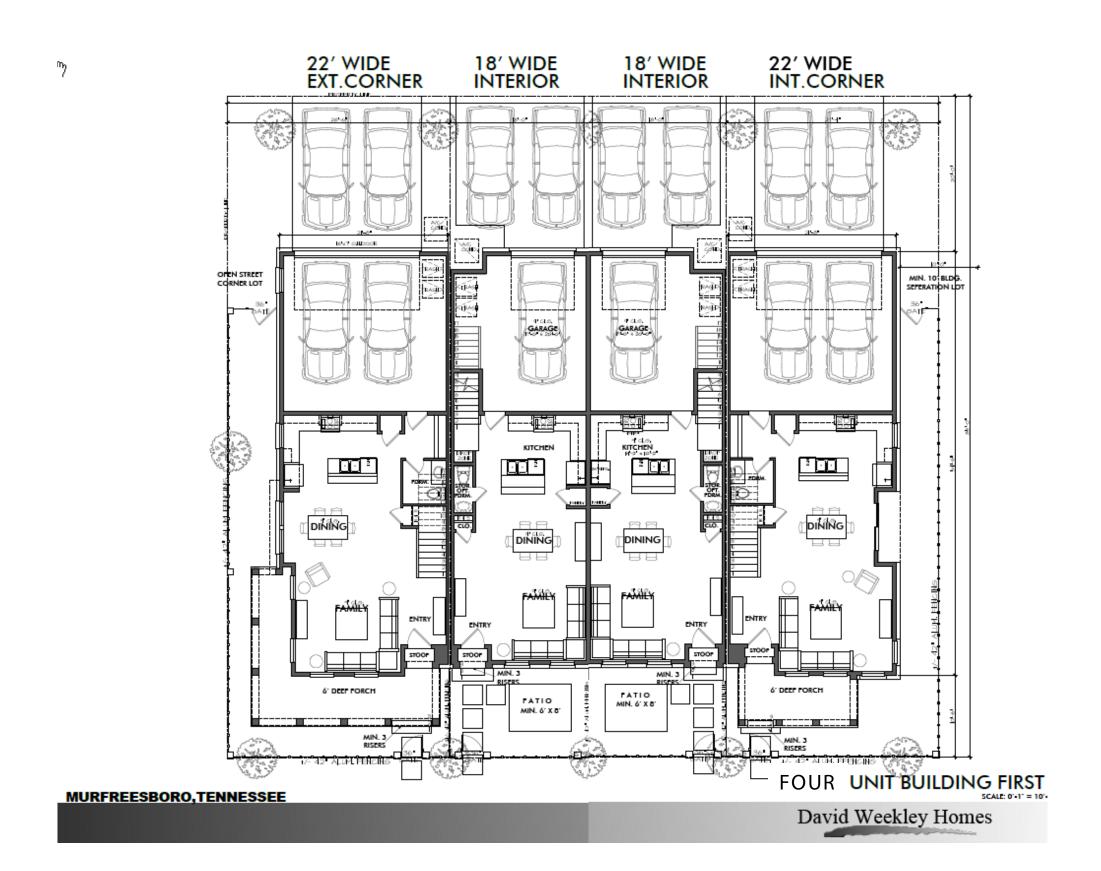




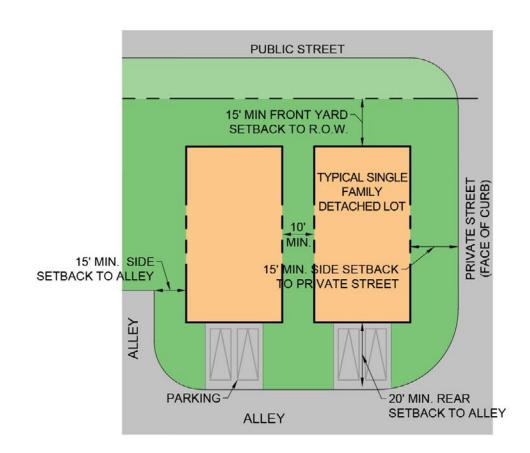
f Clari Park

Single Family Attached Typical 5 Lot Building - Parking Layout

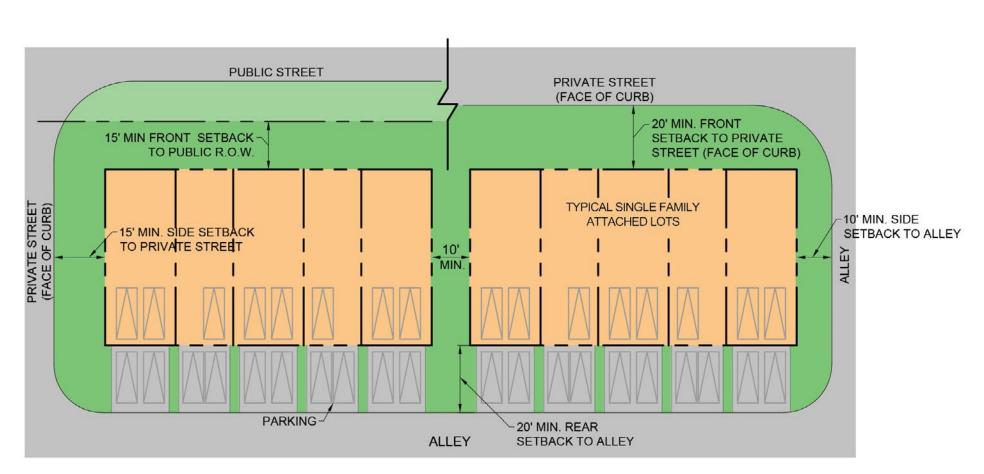




S.F. Attached / S.F. Detached Areas 2 &6 (Front, Side, and Rear Setbacks)



Typical Single Family Detached Homes



Typical Single Family
Attached Homes

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Area 5 (Multi Family Residential - Mixed Use)

Area 5 is at the center of the Clari Park Master Plan and is appropriate for a high density use that takes full advantage of the surrounding Gateway infrastructure and promotes a highly valued development. Standards for architectural height and mass allow for large signature buildings. Buildings in Area 5 are arranged to create centralized open space with social programming, recreational opportunities and leisure amenities. These spaces are designed to become the heart of social activity for the residential multi family component of Clari Park. Area 5 has good opportunities to consider shared use parking for a mixture of uses and adjoining areas. Area 5 is proposed as predominantly multi-family use with a vertically integrated mixture of limited office and service use.

Area 7 (Office or Multi Family Residential)

Area 7 is located at the south west portion of Clari Park and is adjacent to existing commercial and high density residential land use. This mixed use development area provides flexibility in space and response to market conditions for office or high density residential with commercial or office use. Area 7 has a strong relationship to Robert Rose Drive and Clari Lane. Its location provides opportunity for higher densities and taller buildings. Multi family residential is permitted, which would include a mix of office/commercial uses, and this area could be a strong extension of the proposed high density residential proposed for Area 5.



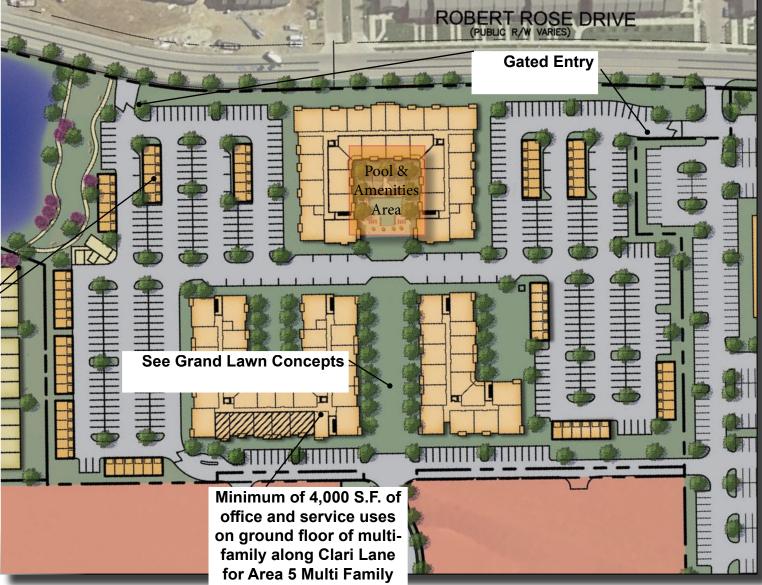
AREA 5 Tollarida Tollarida

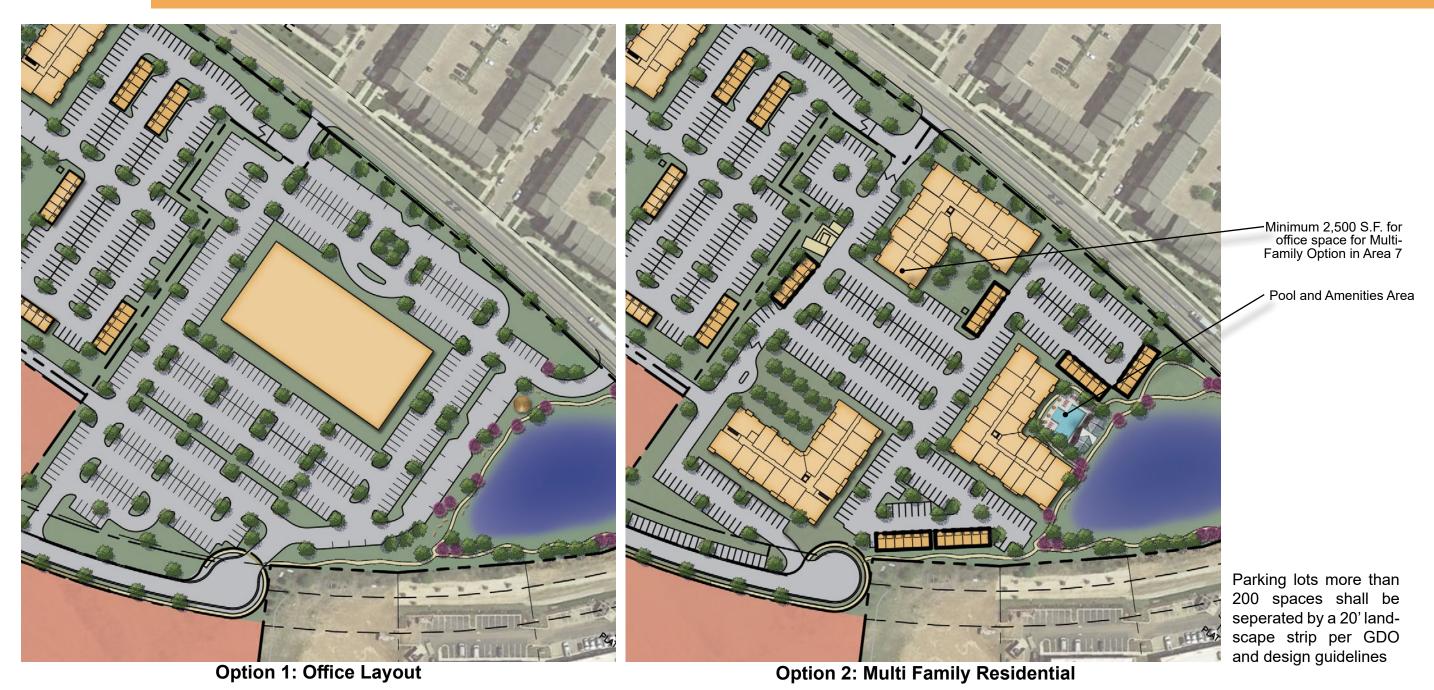
Stand-alone Garages

Parking lots with more than 200 spaces will be seperated by 20' landscape stripps per GDO and design guidelines

This conceptual layout and photographic examples for the potential multi-family development for Area 5 also apply to the potential multi-family for Area 7 as it relates to mixed use, multi-family opportunities.

Area 5 Enlargement

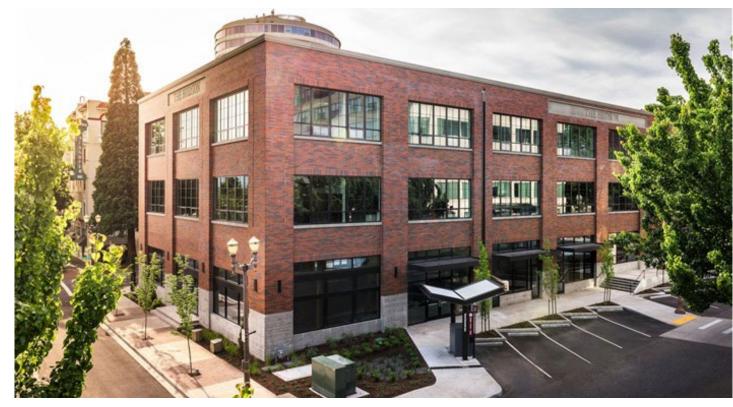




*Note: Office shall be the promoted use for Area 7, after 3 years from the final approval of the Clari Park planned unit development / master plan if no user / developer has been approved for this area, then multi family residential shall be a permitted use for Area 7. If use is Multiple-Family, construction of site work and building in Area 7 shall not commence until three years from approval of the Clari Park Master Plan (master utilities, master stormwater system and mass grading are exempt from construction start provision). Permitting shall be permitted to proceed prior to three years from the Clari Park Master Plan approval).









*Note: Commercial uses material palette referenced on page 29 and 52 should be applied to office use.



Multi-Family Attached Specifications:

Building Construction and Design

- Buildings shall avoid long uninterrupted facades. Variations in the roof line or wall plane shall be used to break up the mass of the building.
- Detached garages shall be permitted and shall count toward meeting the required parking per the Zoning Code. The architecture of the detached garages shall reflect the architectural style of the primary structure.
- The incorporation of exterior balconies shall be encouraged. Balcony railings shall be aluminum, metal or stainless steel cable-stayed construction.
- Metal and canvas awnings shall be permitted to extend in to the front and side building setback.
- Foundation planting landscape materials shall be required on all four sides and all lawn areas shall be sodded.
- Mechanical systems and above-grade utility elements shall be located on the rooftop or in the rear or side of dwellings whenever possible with the exception of electrical and telecommunications equipment that will be placed in designated easements.
- Roof top mechanical equipment shall be screed by parapet walls.

Exterior Building Materials

- The following exterior materials shall be permitted on the exterior façade:
- 1. Brick natural color or painted
- 2. Cementitious composition siding
- 3. Stone natural or manufactured stacked stone
- 4. Wood siding in limited locations or trim elements nay be used if appropriate in context to the architectural style.
- 5. The use exposed concrete block, split-faced block, vinyl siding or corrugated metal siding shall be prohibited. (Note: Vinyl may be used for exterior soffits and miscellaneous trim).
- The following exterior materials shall be permitted as roofing materials:
- 1. TPO single-ply roofing membrane
- 2. Dimensional composition roof shingles
- 3. Metal roof in limited accent applications such as porches and bay windows if appropriate in context to the architectural style.

g Clari Park Architectural Examples - Multi Family (Areas 5 & 7)

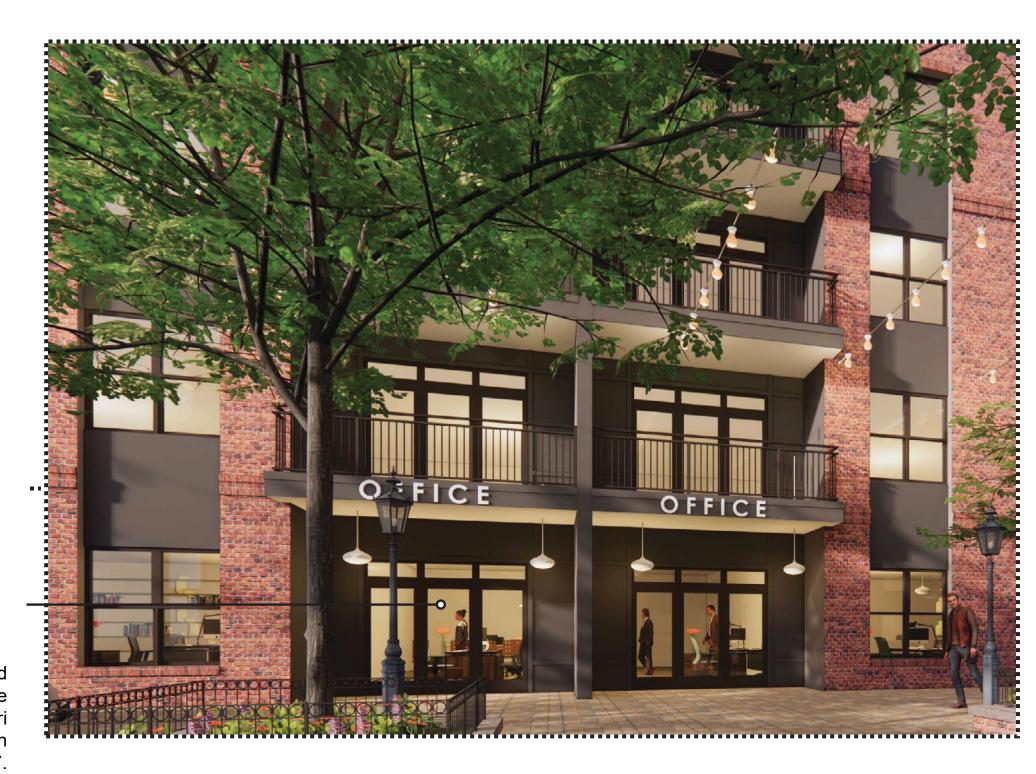


Multi-Family Attached Specifications:

It is recognized that new materials and new uses for materials will continue to be developed. Materials not specifically approved herein may be considered for us on buildings if samples and supporting information are provided to the Planning Staff and the Planning Commission for consideration

Architectural standards set forth in the Murfreesboro Design Guidelines and GDO requirements referencing general character, heights and setbacks, building mass, scale and proportion, building composition and rhythm, transparency, articulation and expression, materials, color, and roof design will be taken into account and the project will meet GDO and Murfreesboro design guidelines.





Minimum of 4,000 square feet of office space on ground floor of apartments for multi-family in Area 5 will integrate with residential uses to create a mix of uses along Clari Lane. Minimum of 2,500 square feet of office space on ground floor of apartments for multi-family option in Area 7.

Photographic Amenity Examples - Multi Family (Area 5 & 7)













These photographic examples depict general concepts of building architectural character in Area 5 and the multi-family option for Area 7. They are not intended to depict final architecture or site design and they do not capture every use or scenario permitted in these areas.

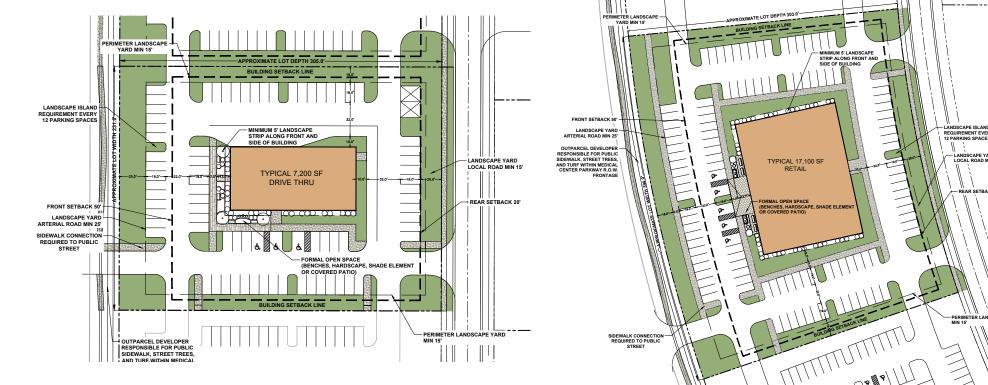
Each phase of the Multi-Family For-Rent Residential at Clari Park (Area 5 and multli family option for Area 7) shall include a comprehensive amenity program for the residents. Elements of the amenity program for each phase shall include:

- Exterior resort pool with a large colored concrete or paver deck area designed with open air and shaded seating areas;
- Exterior gathering areas that incorporate fire pits, grills, and outdoor games;
- A dog park with a water and wash down station;
- Over 6,500 square feet of interior amenity areas in Area 5 and 5,000 S.F. of interior amenity areas in Area 7 will be provided. They will include a resident lounge that connects directly to the pool area, fitness facility with full complement of aerobic and strength equipment, a spin and yoga studio, a dog care room, conference rooms, and work spaces.



Commercial General Description - (CH Commercial Area 3) Clari Park In

- Commercial parcels in Area 3 range in size from approximately 1 to 3 acres and parcel boundaries are subject to change
- Commercial parcels in Area 3 will relate to existing commercial land patterns established along Medical Center Parkway.
- Commercial establishments in Area 3 will connect to the proposed residential and mixed use components of Clari Park and encourage a local, walkable lifestyle to occur in the gateway.
- Proposed residential densities that are part of the Clari Park plan, contribute to the commercial value and viability of Area 3.







- These typical outparcel layouts depict possible scenarios to illustrate how commercial developments may fit into Clari Park.
- Layouts are conceptual in nature. Final site design for each specific parcel will be provided at the time of actual development.
- Street improvements on Medical Center Parkway will follow the Gateway Streetscape Master Plan and City of Murfreesboro Design Guidelines including a 12' landscape area with a minimum 6' wide serpentine sidewalk

h Clari Park Commercial Photographic Examples (Area 3 & Area 4)







Architectural standards set forth in the Murfreesboro Design Guidelines and GDO requirements referencing general character, heights and setbacks, building mass, scale and proportion, building composition and rhythm, transparency, articulation and expression, materials, color, and roof design will be taken into account with the design of this project.

Commercial Uses Materials Palette (Per Murfreesboro Design Guidelines)

- Primary material
 - Brick (full thickness or thin-set)
 - Cast stone
 - Natural or synthetic stone
- Secondary materials
 - Exterior Insulation Finish System (EIFS)
 - Split-face or ground-face, or polished-face concrete masonry (integrally colored)
 - Architectural metal panels with durable finish and defined profile
 - Composite panels
 - Cementitious siding or panels
 - Wood siding may be used on small scale buildings
 - Fabric Awnings
- Tertiary materials:
 - Metal copings, flashings, and trim
 - Wood or cementitious trim
- Prohibited materials
 - Smooth-face concrete masonry
 - Corrugated metal "R" panels

TRAFFIC IMPACT STUDY

for

Clari Park Development

City of Murfreesboro, Tennessee

March 25, 2020 Updated March 29, 2021 Updated April 28, 2021

Prepared for:

Hines 5 Ravinia Drive NE Atlanta, Georgia 30346



Prepared by:



RAGAN-SMITH ASSOCIATES, INC. 315 Woodland Street, P.O. Box 60070 Nashville, Tennessee 37206-0070 (615) 244-8591

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EXECUTIVE SUMMARY

INTRODUCTION

The Clari Park development site is located to the east of Medical Center Parkway between Greshampark Drive and Robert Rose Drive in the City of Murfreesboro, Tennessee. The site will consist of a mix of residential, office, retail, hotel, and other commercial uses. The study area of this report is bound by Medical Center Parkway, Greshampark Drive, Wilkinson Pike, and Robert Rose Drive.

SITE TRAFFIC

CLARI PARK										
TRIP GENERATION SUMMARY										
		Daily Trips	A.M. Peak Hour			P.M. Peak Hour				
Land Use	Total Units		Enter	Exit	Total	Enter	Exit	Total		
Single Family Detached Housing	36 homes	406	7	23	30	24	14	38		
Townhomes (Low-Rise)	186 units	1,622	23	78	101	75	44	119		
Apartments (Mid-Rise)	280 units	1,524	24	70	94	73	46	119		
Hotel	240 rooms	2,283	68	47	115	79	75	154		
Bowling Alley	49,000 sf	-	38	2	40	35	19	54		
General Office	100,000 GSF	1,061	103	17	120	18	96	114		
Medical Office	15,000 sf	489	32	9	41	15	38	53		
Convenience Market w/Gas	4,700 GSF	2,934	95	96	191	116	116	232		
Pumps	(Pass-By Trip %)	60%	63%	63%	63%	66%	66%	66%		
Furniture Store	10,000 GSF	98	2	1	3	3	3	6		
Drive-in Bank	14,000 sf	1,277	77	56	133	143	143	286		
Fast Casual Restaurant	11,667 GSF	3,677	14	10	24	91	74	165		
Quality Restaurant	11,667 GSF	978	5	4	9	54	37	91		
High-Turnover (Sit-Down) Restaurant	11,666 GSF	1,309	64	52	116	71	43	114		
Unadjusted Total Trip	Generation	17,658	552	465	1,017	797	748	1,545		
Internal Trip Redu	ction %	15%		15%			20%	%		
Internal Trip	s	2,649	83	70	153	159	150	309		
Adjusted Total Trip (15,009	469	395	864	638	598	1,236			
Pass-By Trips (from Conv (Trip Generation x Pass-By Trip % x In	1,496	51	51	102	61	61	122			
Adjusted Total Primary T	13,513	418	344	762	577	537	1,114			
Source: ITE Trip Generation N	Manual, 10 th Edition									

CONCLUSIONS AND RECOMMENDATIONS

Medical Center Parkway at Greshampark Drive

• The extension of Willowoak Trail as part of Clari Park provides an additional connection between Medical Center Parkway and Wilkinson Pike that will allow traffic flow at this intersection to continue to be characterized by level of service D during the a.m. and p.m. peak hours. No improvements are recommended at the intersection due to the improvement that is provided by the construction of the Willowoak Trail extension.

Medical Center Parkway at Willowoak Trail

- The new east approach of Willowoak Trail to Medical Center Parkway should include one westbound right turn lane, one westbound through lane, two westbound left turn lanes, and two eastbound lanes to receive the double left turn lanes from Medical Center Parkway. A median should be included so that the through lanes on Willowoak Trail are aligned on each side of the intersection. The minimum length of the westbound left and right turn lanes should be 150 feet with minimum taper lengths of 150 feet.
- The existing pavement provided for the southbound left turn lanes on Medical Center Parkway is appropriate to accommodate the left turn movement needs. The pavement markings should be modified to provide southbound double left turn lanes on Medical Center Parkway with 225 feet of storage and a taper length of 175 feet.
- The existing pavement markings on the west approach of Willowoak Trail should be modified to remove the channelization between the left turn lanes and right turn lane and provide a through lane to the extension of Willowoak Trail.
- A continuous northbound right turn lane should be constructed on Medical Center Parkway along
 the frontage of Clari Park. To be consistent with similar improvements on Medical Center Parkway,
 the continuous right turn lane should extend approximately 950 feet and ending prior to the next
 upstream signalized intersection at Honeylocust Lane.
- Traffic signal modifications will be required as part of the new approach of Willowoak Trail. The signal modifications should include the new pole and mast arm for the westbound approach signal heads and other components required to ensure a fully operational traffic signal. New timings for the intersection will also be required with the traffic signal modification plan.

Medical Center Parkway at Honeylocust Lane

- The new east approach of Honeylocust Lane to Medical Center Parkway should include one westbound right turn lane, one westbound through lane, two westbound left turn lanes, and two eastbound lanes to receive the left turn lanes lane from Medical Center Parkway. A median should be included so that the through lanes on Honeylocust Lane are appropriately aligned on each side of the intersection. The minimum length of the westbound left and right turn lanes should be 150 feet of storage with minimum taper lengths of 150 feet.
- The existing pavement provided for the southbound left turn lanes on Medical Center Parkway is appropriate to accommodate the left turn movement needs. The pavement markings should be

modified to provide southbound double left turn lanes on Medical Center Parkway with 225 feet of storage and a taper length of 175 feet.

- The existing pavement markings on the west approach of Honeylocust Lane should be modified to remove the channelization between the left turn lanes and right turn lane and provide a through lane to the extension of Honeylocust Lane.
- A continuous northbound right turn lane should be constructed on Medical Center Parkway along
 the frontage of Clari Park. To be consistent with similar improvements on Medical Center Parkway,
 the continuous right turn lane should extend approximately 890 feet and tie to the existing
 continuous right turn lane that ends at the Redstone Federal Credit Union access.
- Traffic signal modifications will be required as part of the new approach of Honeylocust Lane. The signal modifications should include the new pole and mast arm for the westbound approach signal heads and other components required to ensure a fully operational traffic signal. New timings for the intersection will also be required with the traffic signal modification plan.
- With the connection provided by the extension of Willowoak Trail to Robert Rose Drive and the
 expected levels of service along Willowoak Trail and Robert Rose Drive, the extension of
 Honeylocust Lane is not required to connect through Clari Park to Robert Rose Drive. Appropriate
 local network connectivity is provided by Willowoak Trail and Robert Rose Drive.

Medical Center Parkway at Maplegrove Drive

• No improvements or traffic control modifications are recommended at the intersection of Medical Center Parkway and Maplegrove Drive.

Medical Center Parkway at Robert Rose Drive

• No improvements or traffic control modifications are recommended at the intersection of Medical Center Parkway and Robert Rose Drive.

Wilkinson Pike at Greshampark Drive

 The City of Murfreesboro's planned reconstruction of Wilkinson Pike to a three-lane roadway with a two-way continuous center turn lane will provide acceptable and appropriate traffic operations at this intersection in the future.

Wilkinson Pike at West Park Drive / Clari Park Access #2

- The City of Murfreesboro's planned reconstruction of Wilkinson Pike to a three-lane roadway with a two-way continuous center turn lane will provide acceptable and appropriate traffic operations at this intersection in the future.
- The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

- The access to Clari Park at this location may be a gated access. The Clari Park Master Plan pattern book (page 34) states "Access design subject to review and approval by Murfreesboro Planning Commission during site plan approval" for this location in the residential garden district.
- Based on discussions with City staff, left turn lanes will be constructed on Wilkinson Pike at West Park Drive and the proposed access to Clari Park with the improvements being for a local road section and brought to the top of binder. The City will complete the surface course of asphalt as part of an upcoming resurfacing project on Wilkinson Pike.
- Traffic volumes at this intersection are not forecasted to satisfy traffic signal warrants even after full build-out of Clari Park. However, a 50-ft. easement would be needed on the project access if a signal were to be installed by others in the future.

Wilkinson Pike at Willowoak Trail

- The City of Murfreesboro's planned reconstruction of Wilkinson Pike to a three-lane roadway with a two-way continuous center turn lane will provide acceptable and appropriate traffic operations at this intersection in the future.
- Based on discussions with City staff, a left turn lane will be constructed on Wilkinson Pike at Willowoak Trail with the improvements being for a local road section and brought to the top of binder. The City will complete the surface course of asphalt as part of an upcoming resurfacing project on Wilkinson Pike.

Willowoak Trail at Robert Rose Drive

 No improvements or traffic control modifications are recommended at the intersection of Willowoak Trail and Robert Rose Drive.

Robert Rose Drive at Grassington Street / Clari Park Access #7

• The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

Robert Rose Drive at Marylebone Street / Clari Park Access #8

 The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

Robert Rose Drive at Sculling Street / Clari Park Access #9

• The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

Greshampark Drive at Clari Park Access #1

- A minimum of one access to Clari Park from Greshampark Drive should be provided south of Wilkinson Pike. Access to the section of Clari Park on the southeast corner of Greshampark Drive and Wilkinson Pike should include two lanes for traffic exiting the site and one lane for traffic entering the site.
- The location of this access will be determined at the site plan level and will subject to review and approval by the Murfreesboro Planning Commission at that time. Pavement marking modifications on Greshampark Drive that be be necessary due to the location of the access will be provided as part of the site plan.

Willowoak Trail at Clari Park Access #3 and #4

- Clari Park Access #3 to the north side of Willowoak Trail at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.
- Clari Park Access #4 to the south side of Willowoak Trail at this intersection should include two lanes for traffic exiting the site and one lane for traffic entering the site.
- One eastbound lane on Willowoak Trail can be a right turn only lane into Clari Park.
- The approaches of Clari Park Access #3 and #4 will be stop-controlled at this intersection. Stop control will not be placed on the Willowoak Trail approaches to this intersection.

Honeylocust Lane at Clari Park Access #5, #6, and #11

- Honeylocust Lane is proposed to intersect Clari Lane approximately 400 feet east of Medical Center Parkway. The proposed intersection should include a minimum of two lanes on each approach of Clari Lane and Clari Park access drives to provide one lane for traffic entering the intersection and one lane for traffic exiting the intersection. Additional lanes may be provided on the approach of Honeylocust Lane to the intersection to align with the intersection at Medical Center Parkway.
- The approaches of Clari Park Access #5, #6, and #11 will be stop-controlled at this intersection to prioritize the flow of traffic away from the intersection of Medical Center Parkway and Honeylocust Lane. Stop control will not be placed on the Honeylocust Lane approach to this intersection.

Willowoak Trail at Clari Park Access #10

 The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

I. INTRODUCTION

The purpose of this study is to analyze the transportation related impacts of Clari Park, a mixed-use development along Medical Center Parkway in the City of Murfreesboro, Tennessee. The development will include residential, office, retail, hotel, and other commercial uses. This report has been requested by City of Murfreesboro staff as part of the development process.

In order to evaluate the development, an inventory of the existing transportation system was carried out, along with an assessment of its adequacy. Based on the project schedule, a build-out horizon year was established, and future traffic growth was added to existing traffic volumes. Transportation analyses were performed to assess any site or non-site related impacts on the roadway. Finally, recommendations for roadway improvements and/or transportation system improvements were offered.

II. PROJECT DESCRIPTION

A. Proposed Development

As shown in Figure 1, Clari Park is located to the east of Medical Center Parkway between Greshampark Drive and Robert Rose Drive in the City of Murfreesboro, Tennessee. The study area of this report includes Medical Center Parkway, Greshampark Drive, Wilkinson Pike, and Robert Rose Drive. Clari Park will be a mixed-use development with the planned densities and land uses shown below.

- Single Family Homes 36 homes
- Townhomes 186 units
- Apartments 280 units
- Hotel 240 rooms
- Office 100,000 square feet
- Medical Office 15,000 square feet
- Convenience Store with Gas Pumps 4,700 square feet
- Furniture Store 10,000 square feet
- Drive-In Bank 14,000 square feet
- Restaurant 35,000 square feet

The Master Plan for Clari Park is shown in Figures 2A and 2B.

B. Site Access

Access to Clari Park is proposed from Medical Center Parkway, Robert Rose Drive, Greshampark Drive, Willowoak Trail, and Wilkinson Pike.

The following existing roadways will be extended to provide access to Clari Park.

- Willowoak Trail begins at Avenue Way and ends at Wilkinson Pike with a total planned length of 2,600 feet. Currently, the portion of Willowoak Trail between Medical Center Parkway and Robert Rose Drive, approximately 1,300 linear feet, is not constructed. This portion of Willowoak Trail passes through the Clari Park site and will be constructed as part of the Clari Park development.
- Honeylocust Lane begins within the Avenues development, intersects Avenue Way and ends at Medical Center Parkway with a total length of 1,100 feet. Honeylocust Lane will be extended into Clari Park and will end approximately 400 feet east of Medical Center Parkway to provide access to Clari Park.

Table 1 below provides a summary of the Clari Park access locations grouped by the existing road that will be accessed.

TABLE 1										
	ACCESS LOCATION SUMMARY									
Access Name	Location	Public or Private	Control	Spacing to Adjacent Driveway/Intersection	Width					
	G	ereshamparl	k Drive ⁽¹⁾							
Access #1	South of Wilkinson Pike	Private	Stop	To be determined at site p	olan phase					
Wilkinson Pike										
Access #2	Existing Intersection	Private	Stop	Aligns with West Park Drive	24 ft.					

TABLE 1									
ACCESS LOCATION SUMMARY									
Access Name	Location	Public or Private	Control	Spacing to Adjacent Driveway/Intersection	Width				
		Willowoal	c Trail						
Access #3	North side, 400 ft. east of Medical Center Pkwy	Private	Stop	400 feet, aligns with Access #4	24 ft.				
Access #4	South side, 400 ft. east of Medical Center Pkwy	Public	Stop	400 feet, aligns with Access #3	36 ft.				
Access #10	South side, 850 ft. east of Medical Center Pkwy	Private	Stop	450 feet	20 ft.				
		Honeylocus	st Lane						
Access #5	North side, 400 ft. east of Medical Center Pkwy	Public	Stop	400 feet, aligns with Access #6	24 ft.				
Access #6	South side, 400 ft. east of Medical Center Pkwy	Public	Stop	400 feet, aligns with Access #5	24 ft.				
Access #11	400 feet east of Medical Center Parkway	Private	Stop	400 feet, aligns with Honeylocust Lane	24 ft.				
		Robert Ros	e Drive						
Access #7	170 ft. south of Willowoak Trail	Private	Stop	150 feet	20 ft.				
Access #8	1,175 ft. south of Willowoak Trail	Private	Stop	250 feet, aligns with Marylebone Street	20 ft.				
Access #9	1,535 ft. south of Willowoak Trail	Private	Stop	250 feet, aligns with Sculling Street	24 ft.				
	Medical Center Parkway (2)								
n/a	Outparcel Boundaries	Private	Stop	To be determined at site p	lan phase				

⁽¹⁾ For proposed access on Greshampark Drive, one or more driveways is anticipated to the Commercial/Entertainment section of the development. The precise location of these driveways is not known at this time but will be determined during the site plan phase.

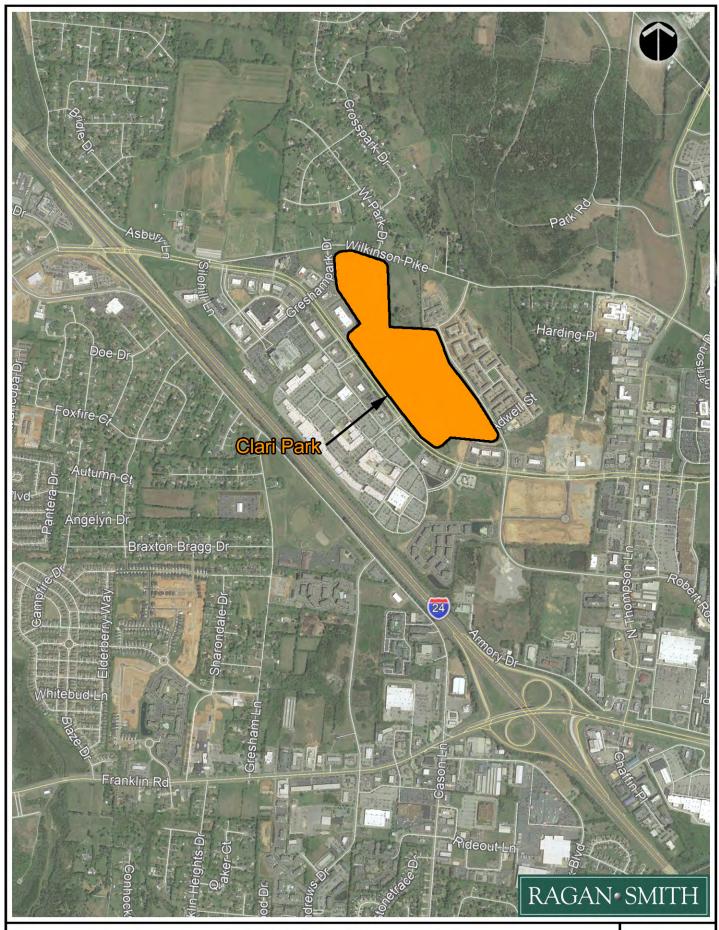
The demarcation between public street and private streets/areas will be achieved using concrete driveway ramps, stamped/colored asphalt, use of different curb types, and other design elements that will be addressed and specified as part of the construction plans.

The access locations for Clari Park as designated in this report are shown in Figure 2C.

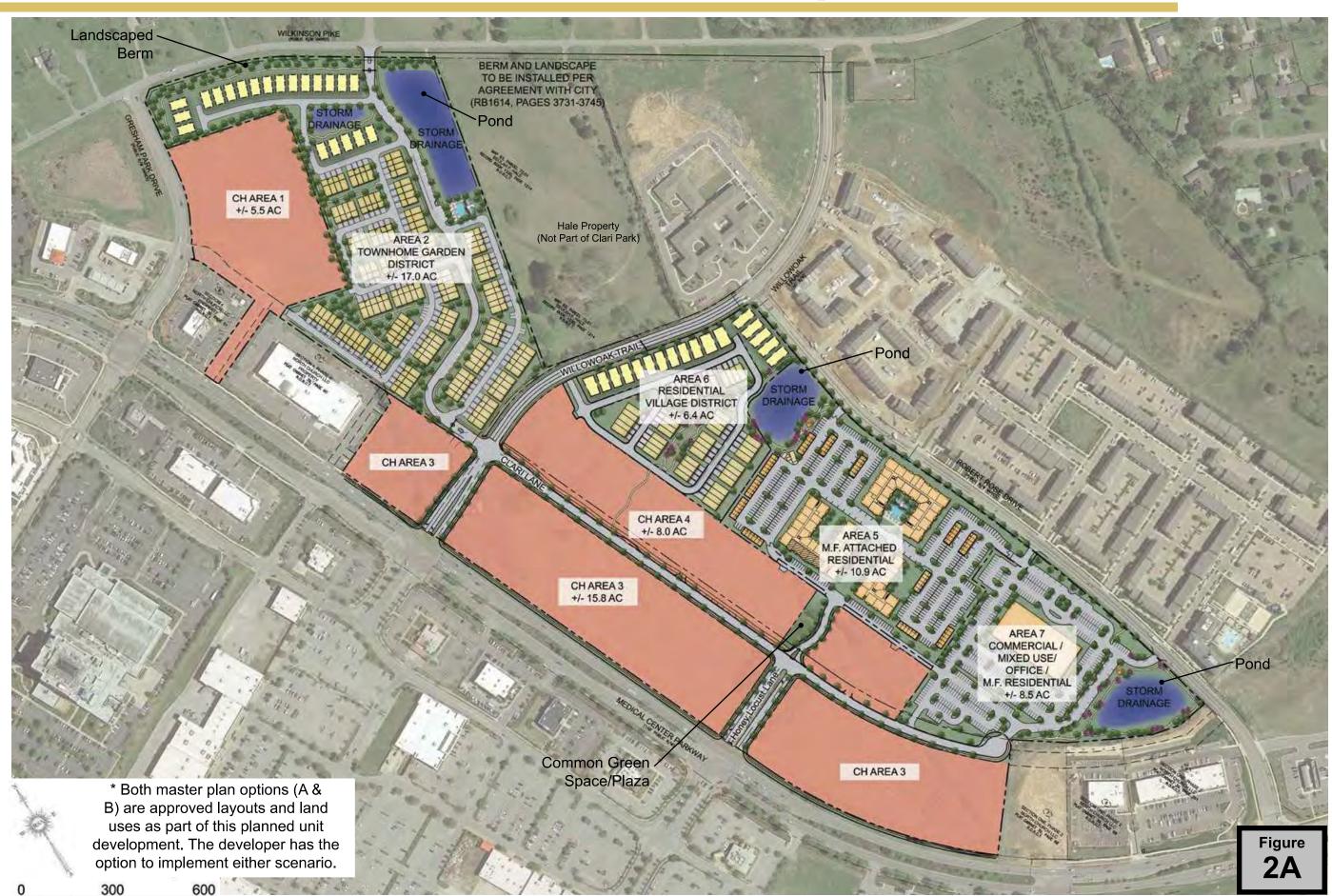
C. Development Buildout Timeline

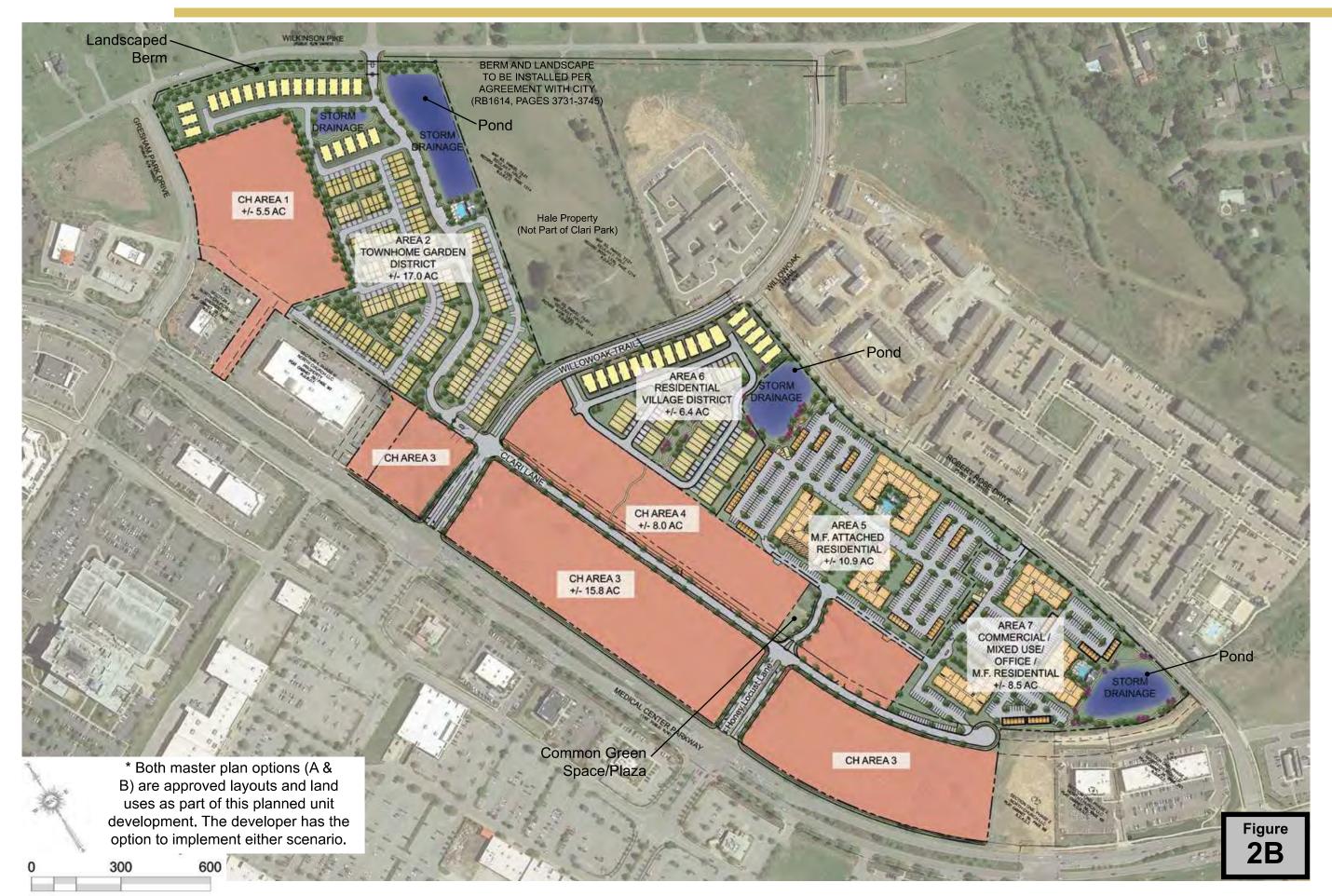
The Clari Park Master Plan pattern book (page 22) indicates that the construction phase estimate will begin in 2021 with approximate time frames completing as soon as 2023 or extending to 2030. The analysis of this report will evaluate the full build-out of Clari Park with a horizon year of 2027 that targets the midpoint of the approximate construction phase time frame.

The commercial outparcels are anticipated to have right-in/right-out driveways on Medical Center Parkway. The precise location of these driveways is not known at this time but will be determined during the site plan phase for each outparcel.

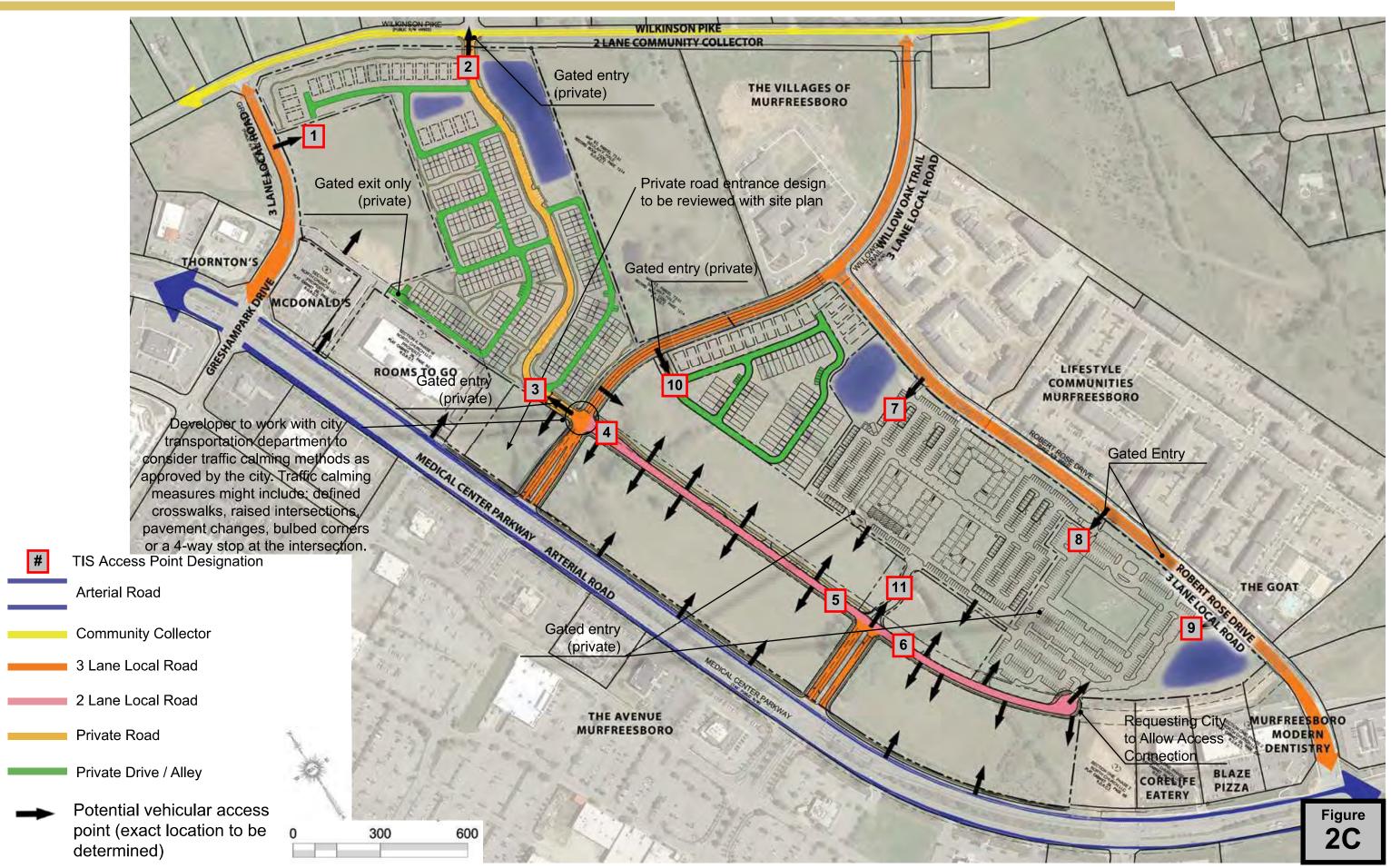


Clari Park Location Map Figure





Vehicular Transportation Network



III. EXISTING CONDITIONS

A. Transportation System

The existing transportation system in the area that is scheduled for development consists of major arterials, an expressway and local roadways. The following roadways will comprise the study area in which traffic mitigation measures are being considered based on the impact of the mixed-use development.

TABLE 2										
EXISTING ROADWAY CHARACTERISTICS										
Name	Designation	Classification (1)	Speed Limit	Lanes	Multimodal					
Medical Center Parkway	City Street	Major Arterial	40	4	Sidewalks (2)					
Robert Rose Drive	City Street	Community Collector	35	3	Sidewalks (2)					
Wilkinson Pike	City Street	Community Collector	40	2	None					
Greshampark Drive	City Street	Community Collector	30	3	Sidewalks (2)					
Willowoak Trail	City Street	Local	30	3	Sidewalks (2)					
Honeylocust Lane	City Street	Local	30	4	Sidewalks (2)					
Maplegrove Drive	City Street	Local	30	4	Sidewalks (2)					
,	1 ci oky or Marineesboro 2040 Major Morouginare i lan									

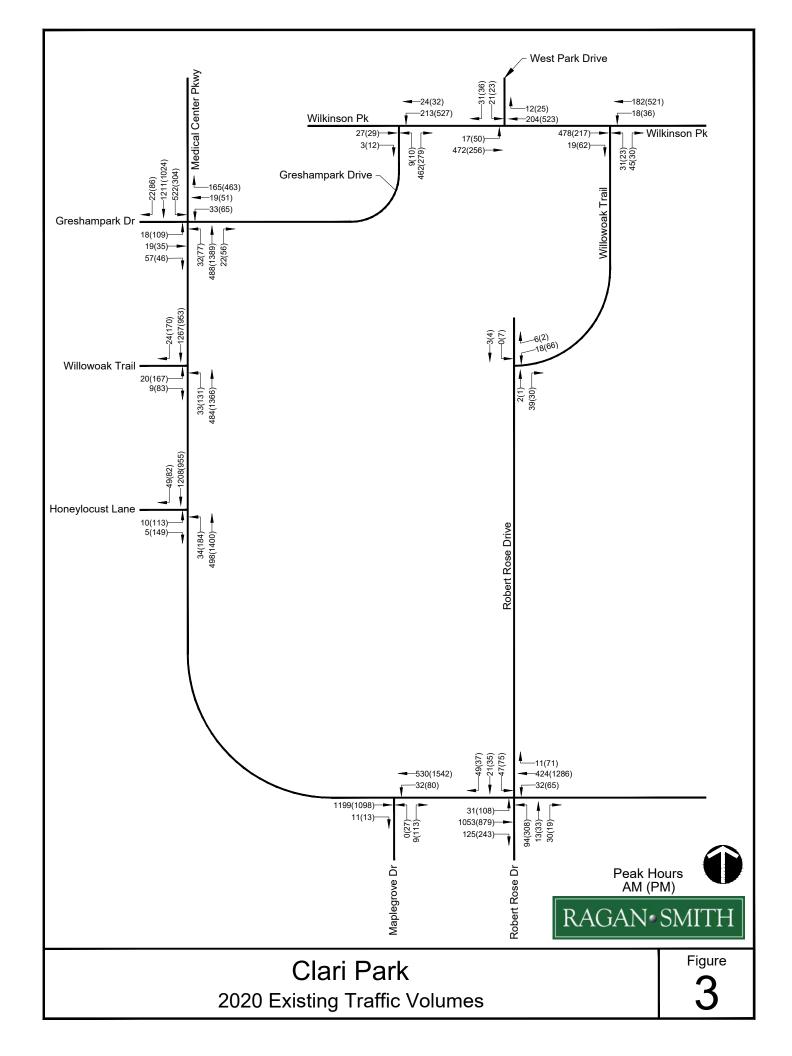
B. Traffic Volumes

In order to assess the adequacy of the local transportation system and establish base traffic conditions for this study, an evaluation of the current operational quality of intersections within the study area was required. The peak hour of the adjacent street traffic was used to determine the impact of Clari Park on the existing transportation system.

The peak periods for analysis were identified by conducting traffic counts in February of 2020 at the following intersections:

- Medical Center Parkway at Greshampark Drive
- Medical Center Parkway at Willowoak Trail
- Medical Center Parkway at Honeylocust Lane
- Medical Center Parkway at Maplegrove Drive
- Medical Center Parkway at Robert Rose Drive
- Wilkinson Pike at Greshampark Drive
- Wilkinson Pike at W Park Drive
- Wilkinson Pike at Willowoak Trail
- Willowoak Trail at Robert Rose Drive
- Robert Rose Drive at Grassington Street
- Robert Rose Drive at Sculling Street

Figure 3 shows the existing peak hour traffic volumes in the study area.



IV. FORECASTED BACKGROUND TRAFFIC

A. Introduction

The year 2027 will be used to analyze the traffic impact within the study area. Before any impacts to the study area could be addressed, some estimate of background traffic volumes for the horizon year 2027 had to be established. Background traffic volumes were established by estimating potential growth due to small scale development and/or general population growth in the area.

B. Specific Development Growth

Based on discussions with City of Murfreesboro staff related to the scope of the traffic impact study, there is one specific development in the study area that is expected to be open prior to the completion of Clari Park. The specific development is a proposed hotel on Greshampark Drive south of Wilkinson Pike. No specific traffic impact study was completed for the proposed hotel so the traffic growth from the hotel has been accounted for as part of the annual growth rate applied to existing traffic volumes as described below.

C. Annual Growth

To establish traffic growth due to population growth or small-scale development, Tennessee Department of Transportation (TDOT) historical traffic count data was obtained at locations within the general project vicinity. The TDOT historical traffic count data includes traffic volume counts conducted annually on Medical Center Parkway, Wilkinson Pike, and Thompson Lane and in the general vicinity of the site. The available historical count data was tabulated for each location and analyzed to identify patterns or growth trends.

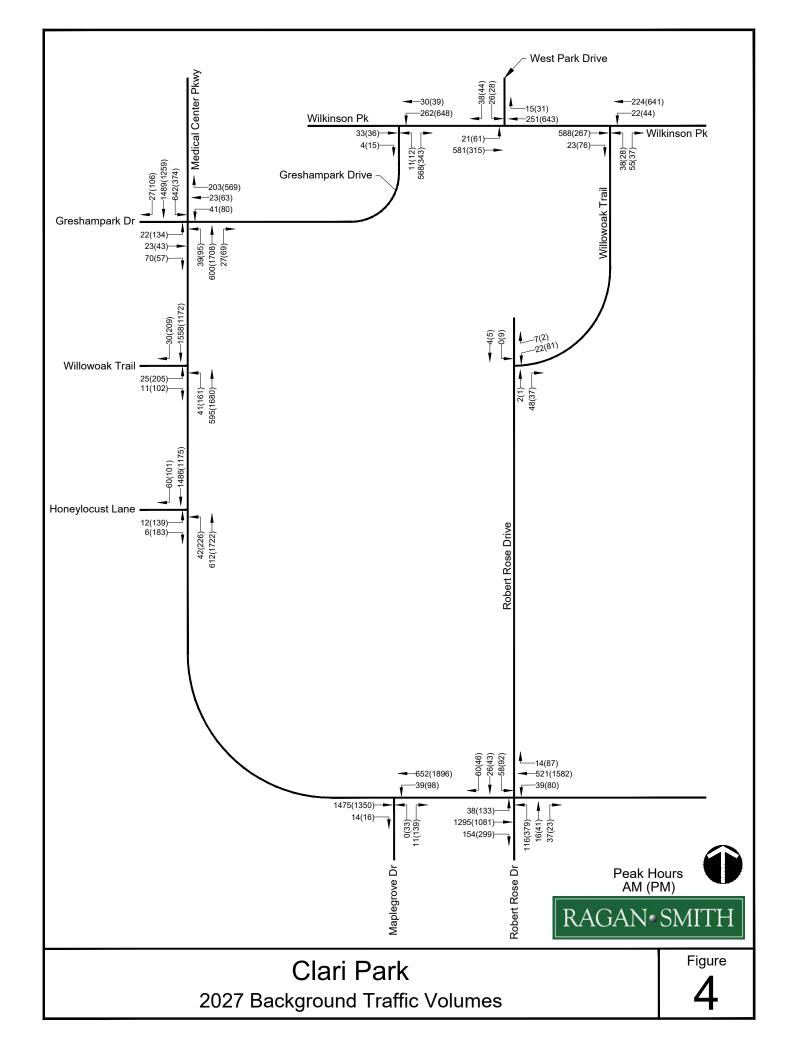
Based upon linear regression analysis of the TDOT historical traffic count data and based upon the growth from specific developments that will be considered, we will use a **3.0 percent annual growth rate** as the base growth for the existing traffic volumes.

D. <u>Background Traffic</u>

Background traffic for the future traffic forecasts was compiled based on the following:

- 2020 existing traffic volumes
- 3.0% annual increase of traffic volumes for the period from 2020 to 2027

Background traffic volumes on the future roadway, representing existing traffic volumes plus background growth, for the year 2027 is shown in Figure 4.



V. PROPOSED SITE TRAFFIC

A. Site Trip Generation

In order to quantify site-related impacts within the study area, estimates of site trip generation and traffic assignment had to be established. Trip generation rates for Clari Park were established using information for the weekday a.m. and p.m. peak hour of the adjacent street as shown in the *ITE Trip Generation Manual*, 10th Edition.

Additionally, the *National Cooperative Highway Research Program Report 684* provides guidance for estimating how many internal trips will be generated in mixed-use developments, for which the origin and destination are within the development. These trips can be subtracted from the total number of trips anticipated to produce a more accurate estimation of the trips that will be entering and exiting the mixed-use development.

Trip generation for Clari Park is shown in Table 3 below.

TABLE 3									
CLARI PARK TRIP GENERATION									
Land Use	Total Units	Daily	A.M. Peak Hour			P.M. Peak Hour			
Land Use	Total Units	Trips	Enter	Exit	Total	Enter	Exit	Total	
Single Family Detached Housing	36 homes	406	7	23	30	24	14	38	
Townhomes (Low-Rise)	186 units	1,622	23	78	101	75	44	119	
Apartments (Mid-Rise)	280 units	1,524	24	70	94	73	46	119	
Hotel	240 rooms	2,283	68	47	115	79	75	154	
Bowling Alley	49,000 sf	-	38	2	40	35	19	54	
General Office	100,000 GSF	1,061	103	17	120	18	96	114	
Medical Office	15,000 sf	489	32	9	41	15	38	53	
Convenience Market	4,700 GSF	2,934	95	96	191	116	116	232	
w/Gas Pumps	Pass-By Trip %	60%	63%	63%	63%	66%	66%	66%	
Furniture Store	10,000 GSF	98	2	1	3	3	3	6	
Drive-in Bank	14,000 sf	1,277	77	56	133	143	143	286	
Fast Casual Restaurant	11,667 GSF	3,677	14	10	24	91	74	165	
Quality Restaurant	11,667 GSF	978	5	4	9	54	37	91	
High-Turnover (Sit-Down) Restaurant	11,666 GSF	1,309	64	52	116	71	43	114	
Unadjusted Total ⁻	Гrip Generation	17,658	552	465	1,017	797	748	1,545	
Internal Trip R	eduction %	15%		15%	•	20%			
Internal	Trips	2,649	83	70	153	159	150	309	
Adjusted Total T	Adjusted Total Trip Generation			395	864	638	598	1,236	
Pass-By Trips (from C (Trip Generation x Pass-By Trip %	1,496	51	51	102	61	61	122		
Adjusted Total Genera	13,513	418	344	762	577	537	1,114		
Source: ITE Trip Gene	eration Manual, 10 th	Edition							

A master development plan including a different mix of land uses and densities was previously prepared for the development site. As part of the evaluation of the Clari Park master plan, the estimates of site trip generation for both the previous master plan and current, proposed master plan have been compared.

Trip generation for the previous master plan at the development site is shown in Table 4 below.

TABLE 4										
PREVIOUS MASTER PLAN TRIP GENERATION										
Land Use	Total Units	Daily	A.M	l. Peak H	lour	P.M. Peak Hour				
Land Use	Total Units	Trips	Enter	Exit	Total	Enter	Exit	Total		
Multifamily (Low-Rise)	489 units	3,656	49	166	215	153	90	243		
Hotel	193 Rooms	1,752	54	37	91	61	58	119		
General Office	297,500 GSF	3,055	263	43	306	51	270	321		
Shopping Center	45,600 GSF	3,525	109	66	175	146	158	304		
Supermarket	29,500 GSF	3,304	68	45	113	160	154	314		
Drive-in Bank	6 Lanes	749	32	21	53	80	83	163		
Fast Casual Restaurant	14,081 GSF	4,438	17	12	29	109	90	199		
Quality Restaurant	14,080 GSF	1,180	n/a	n/a	10	65	45	110		
High-Turnover (Sit-Down) Restaurant	14,080 GSF	1,579	77	63	140	86	52	138		
Unadjusted Total Trip Generation		23,238	669	453	1,132	911	1,000	1,911		
Internal Trip Red	15%		15%		20%					
Internal Tri	3,486	100	68	168	182	200	382			
Adjusted Total Trip	19,752	569	385	964	729	800	1,529			
Source: ITE Trip Genera	tion Manual, 10 th	Edition	•		•					

A comparison of the proposed Clari Park trip generation and the previous master plan trip generation is provided below in Table 5.

TABLE 5										
TRIP GENERATION COMPARISON										
Development Comorio	Daily	AM Peak Hour			PM Peak Hour					
Development Scenario		Enter	Exit	Total	Enter	Exit	Total			
Clari Park Master Plan	13,513	418	344	762	577	537	1,114			
Previous Master Plan	19,752	569	385	964	729	800	1,529			
Net Change	- 6,239	- 151	- 41	- 202	- 152	- 263	- 415			
% Change	- 32%	- 27%	- 11%	- 21%	- 21%	- 33%	- 27%			

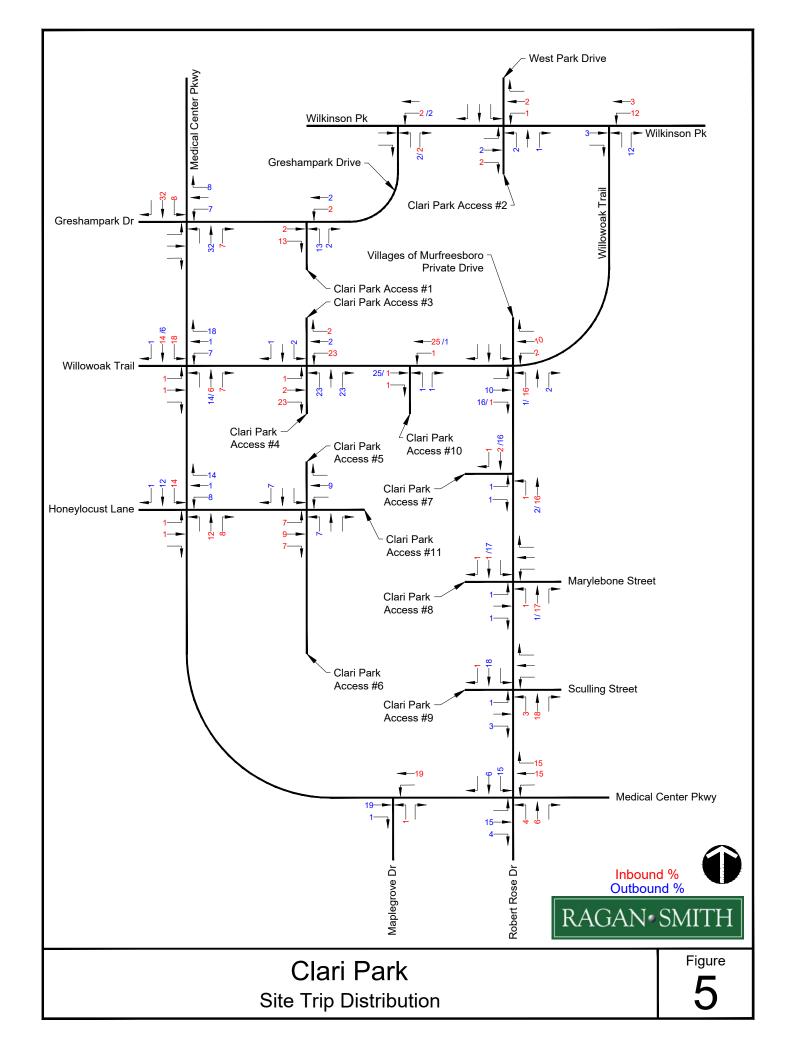
As presented in Table 5, the Clari Park master plan is expected to generate fewer trips than the previously proposed master plan for the development site.

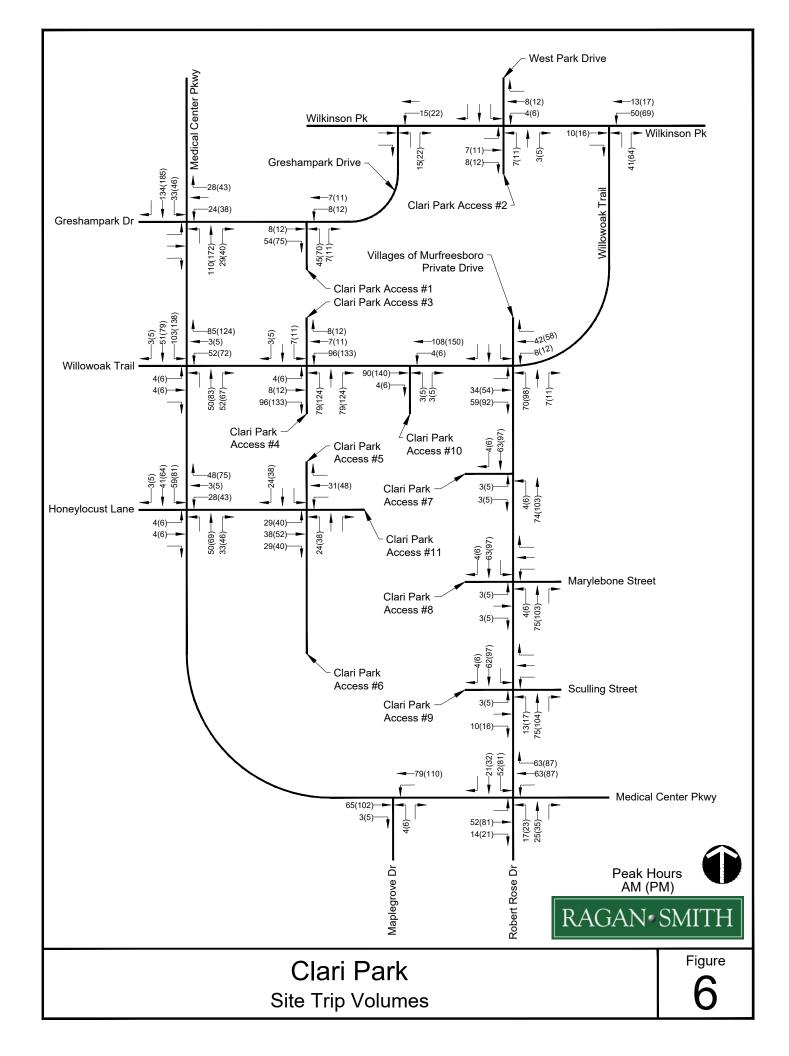
B. Site Trip Distribution

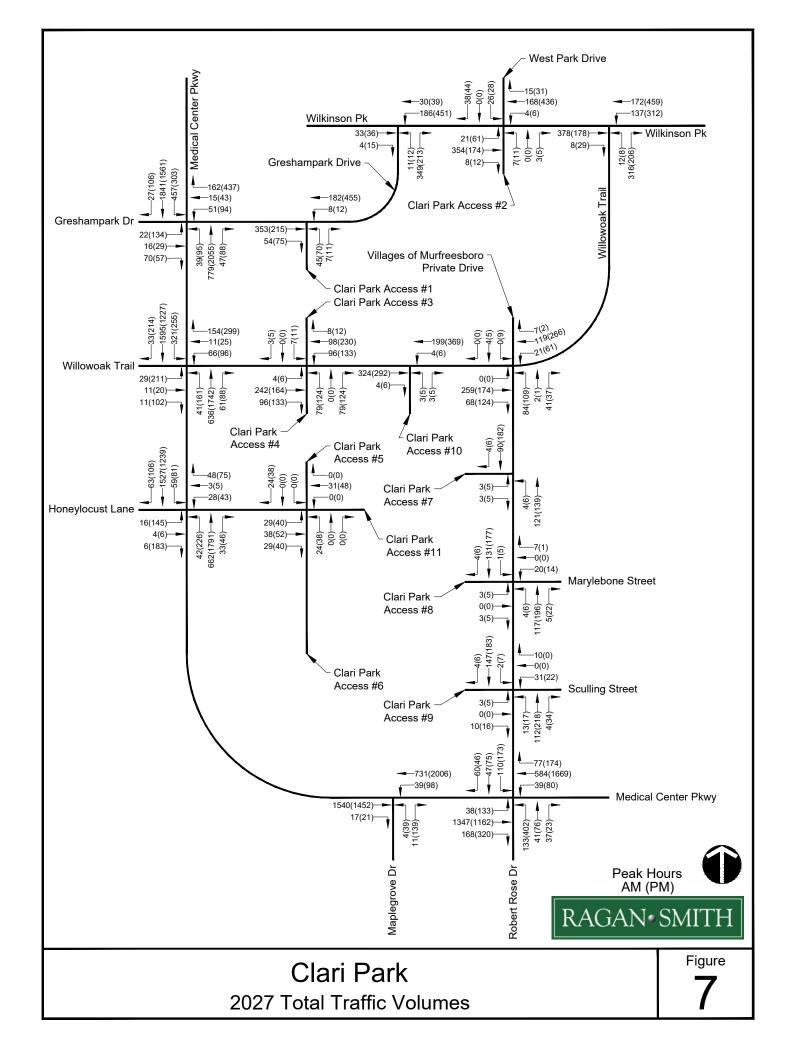
The distribution of the site generated trips for Clari Park is shown in Figure 5. Site generated traffic volumes are shown in Figure 6. Total traffic volumes representing the accumulation of existing, background, and site generated traffic on the external roadway network in the horizon year 2027 are shown in Figure 7.

C. Willowoak Trail Extension

As part of Clari Park, Willowoak Trail will be extended to provide a direct connection between Medical Center Parkway and Robert Rose Drive and a link to Wilkinson Pike from Medical Center Parkway. The proposed route between Medical Center Parkway and Wilkinson Pike is a viable alternative route to Greshampark Drive between Medical Center Parkway and Wilkinson Pike. Based on a review of the existing daily and peak hour traffic volumes on Medical Center Parkway, Wilkinson Pike, and Greshampark Drive as well as the capacity analysis of this report, the reassignment of approximately 3,500 trips per day from Greshampark Drive to Willowoak Trail is anticipated after the Willowoak Trail extension is complete. The future conditions analysis of this report includes the reassignment of traffic to the Willowoak Trail extension and the expected reduction in traffic on Wilkinson Pike and Greshampark Drive near Medical Center Parkway.







VI. TRANSPORTATION ANALYSIS

A. Intersection Capacity Analysis

In order to gauge the site impact and identify capacity deficient locations, capacity analyses were conducted at intersections within the area of Clari Park as well as the proposed access points. Capacity analyses were conducted according to the methodology and procedures outlined in the *Highway Capacity Manual*, 2010, published by the Transportation Research Board.

Level of service (LOS) criteria for signalized intersections is shown in Table 6.

	TABLE 6										
LEVEL OF SERVICE DESCRIPTIONS FOR SIGNALIZED INTERSECTIONS											
Level of Service	Description	Control Delay (sec. /veh.)									
Α	Free Flow	≤10									
В	B Stable Flow (slight delays)										
С	Stable Flow (acceptable delays)	> 20 - 35									
D	Approaching unstable flow (tolerable delay)	> 35 - 55									
Е	Unstable flow (intolerable delay)	> 55 - 80									
F	F Forced flow (congested and queues fail to clear) > 80										
Source: Highway C	Capacity Manual, 2010 Edition										

Level of service (LOS) criteria for unsignalized intersections is shown in Table 7.

	TABLE 7												
LEVEL OF SERVICE DESCRIPTIONS FOR UNSIGNALIZED INTERSECTIONS													
Level of Service	Description												
А	Usually no conflicting traffic	0 - 10											
В	B Occasionally some delay due to conflicting traffic												
С	Delay is noticeable but not inconveniencing	> 15 - 25											
D	Delay is noticeable and irritating, increased risk taking	> 25 - 35											
Е	Delay approaches tolerance level, risk taking likely	> 35 - 50											
F	F Delay exceeds tolerance level, high likelihood of risk taking > 50												
Source: Highway	Capacity Manual, 2010 Edition												

Intersection capacity analysis results for the a.m. peak hour are shown in Table 8.

	TABL	_E 8									
INTERSEC	CTION CAPACITY ANALY	SIS RESULTS – A.M. PEA	AK HOUR								
Turning Movement (1)	Level of Se	rvice (Avg. Delay per Vel	nicle – sec.)								
ruming movement	2020 Existing	2027 Background	2027 Total								
	Medical Center Parkway	at Greshampark Drive									
Overall Signalized Intersection	B (15.7)	D (37.2)	D (35.9)								
	Medical Center Parkw	ay at Willowoak Trail									
Overall Signalized Intersection	A (2.6)	A (2.9)	C (27.1)								
	Medical Center Parkway	y at Honeylocust Lane									
Overall Signalized Intersection	A (5.9)	A (2.7)	A (8.2)								
	Medical Center Parkwa	y at Maplegrove Drive									
Overall Signalized Intersection	Overall Signalized A (5.1) A (6.0)										
	Medical Center Parkway	y at Robert Rose Drive									
Overall Signalized Intersection	B (11.5)	B (12.3)	B (15.0)								
	Wilkinson Pike at G	reshampark Drive									
WB Left	A (7.7)	A (7.9)	A (7.7)								
TWSC NB Left	B (14.2)	C (16.7)	B (13.4) B (12.8) with improvements								
TWSC NB Right	B (12.3)	B (14.9)	B (10.8)								
Will	kinson Pike at West Park	Drive / Clari Park Access	#2								
EB Left	A (7.8)	A (8.0)	A (7.7)								
WB Left	-	-	A (8.2)								
TWSC NB	-	-	B (14.9) B (14.9) with improvements								
TWSC SB	B (13.2)	C (16.0)	B (12.9) B (12.9) with improvements								
	Wilkinson Pike at	Willowoak Trail									
WB Left	A (8.7)	A (9.1)	A (8.8)								
TWSC NB Left	C (16.8)	C (21.5)	C (20.8) C (15.0) with improvements								
TWSC NB Right	B (12.6)	B (14.4)	C (18.8)								

	TABL	.E 8	
INTERSEC	TION CAPACITY ANALY	SIS RESULTS – A.M. PEA	K HOUR
Turning Mayamant (1)	Level of Se	rvice (Avg. Delay per Veh	icle – sec.)
Turning Movement (1)	2020 Existing	2027 Background	2027 Total
	Willowoak Trail at F	Robert Rose Drive	
WB Left	A (7.2)	A (7.3)	A (8.1)
EB Left	-	-	A (0.0)
TWSC NB Left	1	-	B (14.9)
TWSC NB Thru/Right	A (8.5)	A (8.6)	B (10.6)
TWSC SB	A (9.3)	A (9.4)	B (13.4)
	Robert Rose Drive at	Clari Park Access #7	
NB Left	-	-	A (7.5)
TWSC EB	-	-	A (9.6)
Robert	Rose Drive at Marylebon	e Street / Clari Park Acces	ss #8
NB Left	-	-	A (7.5)
SB Left	-	-	A (7.5)
TWSC EB	-	-	A (9.8)
TWSC WB	-	-	B (10.3)
Robe	rt Rose Drive at Sculling	Street / Clari Park Access	#9
NB Left	-	-	A (7.6)
SB Left	-	-	A (7.5)
TWSC EB	-	-	A (9.6)
TWSC WB	-	-	B (10.7)
	Greshampark Drive at	Clari Park Access #1	
SB Left	-	-	A (8.2)
TWSC WB	<u>-</u>	<u>-</u>	B (12.3)
	Willowoak Trail at Clar	i Park Access #3 / #4	
EB Left	-	-	A (7.5)
WB Left	-	-	A (8.3)
TWSC NB (Access #4)	-	-	B (13.6)
TWSC SB (Access #3)	-	-	B (14.8)

	TABI	.E 8												
INTERSEC	CTION CAPACITY ANALY	SIS RESULTS – A.M. PEA	K HOUR											
Turning Mayamant (1)	Turning Movement (1) Level of Service (Avg. Delay per Vehicle – sec.)													
2020 Existing 2027 Background 2027 Total														
	Willowoak Trail at Cl	ari Park Access #10												
WB Left	WB Left A (8.01)													
TWSC NB	-	-	B (10.9)											
	Honeylocust Lane at Cl	ari Park Access #5 / #6												
EB Approach	-	-	A (2.2)											
NB Approach	-	-	A (9.9)											
SB Approach	-	-	A (8.6)											
(1) TWSC = Two-Way Stop	Control													

Intersection capacity analysis results for the p.m. peak hour are shown in Table 9.

	TABL	E 9										
INTERSECTION CAPACITY ANALYSIS RESULTS – P.M. PEAK HOUR Level of Service (Avg. Delay per Vehicle – sec.)												
Turning Movement (1)	Level of Se	rvice (Avg. Delay per Vel	nicle – sec.)									
Turning Movement	2020 Existing	2027 Background	2027 Total									
	Medical Center Parkway	at Greshampark Drive										
Overall Signalized Intersection												
	Medical Center Parkw	ay at Willowoak Trail										
Overall Signalized Intersection	Overall Signalized B (10.2) B (10.4)											
Overall Signalized Intersection	B (11.8)	B (12.2)	D (39.6)									
	Medical Center Parkwa	y at Maplegrove Drive										
Overall Signalized Intersection	A (9.3)	B (10.7)	C (33.2)									
	Medical Center Parkway	at Robert Rose Drive										
Overall Signalized Intersection	C (21.8)	C (27.4)	C (31.5)									
	Wilkinson Pike at G	reshampark Drive										
WB Left	WB Left A (8.6) A (9.1)											
TWSC NB Left	F (59.1)	D (27.0) C (23.9) with improvements										
TWSC NB Right	A (9.9)	B (10.4)	A (9.5)									

	TAB	LE 9											
INTERSEC	TION CAPACITY ANALY	YSIS RESULTS – P.M. PE	AK HOUR										
Turning Mayor and (1)	Level of S	ervice (Avg. Delay per Ve	hicle – sec.)										
Turning Movement (1)	2020 Existing	2027 Background	2027 Total										
Wilki	nson Pike at West Park	Drive / Clari Park Access	s #2										
EB Left	A (8.8)	A (9.4)	A (8.6)										
WB Left	-	-	A (7.6)										
TWSC NB	-	-	C (17.0) C (16.8) with improvements										
TWSC SB	C (16.2)	C (21.4)	C (15.9) C (15.8) with improvements										
	Wilkinson Pike a	t Willowoak Trail											
WB Left A (7.9) A (8.1) A (8.5)													
TWSC NB Left	C (23.1)	C (23.5)	E (39.7) C (23.9) with improvements										
TWSC NB Right	B (10.0)	B (10.3)	B (10.8)										
	Willowoak Trail at	Robert Rose Drive											
WB Left	A (7.3)	A (7.4)	A (8.2)										
EB Left	-	-	A (0.0)										
TWSC NB Left	-	-	D (26.9)										
TWSC NB Thru/Right	A (8.5)	A (8.5)	B (10.3)										
TWSC SB	B (10.2)	B (10.6)	C (19.0)										
	Robert Rose Drive at	Clari Park Access #7											
NB Left	-	-	A (7.6)										
TWSC EB	-	-	A (9.9)										
Robert	Rose Drive at Marylebo	ne Street / Clari Park Acc	ess #8										
NB Left	-	-	A (7.6)										
SB Left	-	-	A (7.7)										
TWSC EB	-	-	B (10.7)										
TWSC WB	-	-	B (12.0)										
Rober	t Rose Drive at Sculling	Street / Clari Park Acces	ss #9										
NB Left	-	-	A (7.7)										
SB Left	-	-	A (7.8)										
TWSC EB	-	-	B (10.3)										
TWSC WB	-	-	B (13.4)										
	Greshampark Drive a	t Clari Park Access #1											
SB Left	-	-	A (7.9)										
TWSC WB	-	-	B (13.8)										

	TABL	.E 9	
INTERSEC	CTION CAPACITY ANALY	SIS RESULTS – P.M. PEAK	HOUR
Turning Movement (1)	Level of Se	rvice (Avg. Delay per Vehic	cle – sec.)
rurning Movement	2020 Existing	2027 Background	2027 Total
	Willowoak Trail at Clar	i Park Access #3 / #4	
EB Left	-	-	A (7.8)
WB Left	-		A (8.3)
TWSC NB (Access #4)	-	-	C (18.3)
TWSC SB (Access #3)	-	-	C (19.5)
	Willowoak Trail at Cla	ari Park Access #10	
WB Left	-	-	A (8.0)
TWSC NB	-	-	B (11.6)
	Honeylocust Lane at Cla	ari Park Access #5 / #6	
EB Approach	-	-	A (2.2)
NB Approach	-	-	B (10.7)
SB Approach	-	-	A (8.7)
(1) TWSC = Two-Way Stop	Control		

B. Queue Length Review

The intersection capacity analysis also included the calculation of 95th percentile queue lengths at the signalized intersections in the study area. Results of the queue analysis for the a.m. and p.m. peak hours and the length of existing and/or proposed turn lanes are shown in Table 10.

		TA	BLE 10											
QUEUE ANALYSIS RESULTS 95 th Percentile Queue Length (ft)														
			95 th P	ercentile C	ueue Leng	jth (ft)								
Turning Movement	Turn Lane Storage Length (ft)	2020 E	xisting	2027 Bac	kground	2027 Tot	al Traffic							
MOVEMENT	Length (it)	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.							
	Medical Ce	enter Parkv	vay at Gres	hampark [Drive	<u> </u>								
NB Left														
NB Right	820	< 25	< 25	< 25	< 25	< 25	< 25							
SB Left	365	488	566	1006	785	502	617							
SB Right	1000	< 25	< 25	< 25	< 25	< 25	< 25							
EB Left	65	< 25	93	25	111	25	111							
EB Right	65	< 25	< 25	< 25	< 25	< 25	< 25							
WB Left	110	34	62	39	73	46	84							
WB Right	110	69	372	75	545	68	346							
	Medical	Center Par	kway at Wi	illowoak Tr	ail									
NB Left	225	36	111	30	137	44	132							
NB Right	-	-	-	-	-	< 25	< 25							
SB Left	-	-	-	-	-	220	229							
SB Right	100	< 25	28	< 25	< 25	< 25	60							
EB Left	165	< 25	130	27	154	34	239							
EB Right	165	< 25	55	< 25	60	< 25	< 25							
WB Left	-	-	-	-	-	62	84							
WB Right	-	-	-	-	1	< 25	131							
	Medical C	enter Park	way at Hor	eylocust L	ane									
NB Left	210	34	132	40	170	44	168							
NB Right	-	-	-	-	-	< 25	< 25							
SB Left	-	-	-	-	-	38	74							
SB Right	850	< 25	83	< 25	< 25	< 25	< 25							
EB Left	190	< 25	61	< 25	112	< 25	153							
EB Right	190	< 25	< 25	< 25	80	< 25	90							
WB Left	-	-	-	-	-	32	47							
WB Right	-	-	-	-	-	< 25	< 25							

TABLE 10 QUEUE ANALYSIS RESULTS													
	QI	UEUE ANA	LYSIS RE	SULTS									
			95 th P	Percentile C	lueue Lenç	gth (ft)							
Turning Movement	Turn Lane Storage	Turn Lane Storage Length (ft) 2020 Existing 2027 Background											
		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.						
	Medical C	enter Park	way at Ma	plegrove D	rive								
NB Left	NB Left 140 < 25 31 < 25 36 < 25												
NB Right	350	< 25	64	< 25	70	< 25	82						
EB Right	850	< 25	< 25	< 25	< 25	< 25	< 25						
WB Left	205	30	63	39	61	40	62						
	Medical Co	enter Park	way at Rob	ert Rose D	rive								
NB Left	85	72	216	88	310	98	303						
NB Right	85	< 25	< 25	< 25	< 25	< 25	< 25						
SB Left	100	43	69	51	81	85	132						
SB Right	100	100	100	100	< 25	< 25	< 25	< 25	< 25	< 25			
EB Left	150	< 25	48	< 25	151	< 25	239						
EB Right	100	< 25	100	28	52	34	107						
WB Left	250	< 25	39	< 25	45	< 25	47						
WB Right	350	< 25	< 25	< 25	< 25	< 25	49						

VII. CONCLUSIONS AND RECOMMENDATIONS

A. Medical Center Parkway at Greshampark Drive

The construction of the Willowoak Trail extension as part of Clari Park provides an additional connection between Medical Center Parkway and Wilkinson Pike. This new connection provides an alternate route and effectively adds capacity to the intersection of Medical Center Parkway and Greshampark Drive.

• The extension of Willowoak Trail as part of Clari Park provides an additional connection between Medical Center Parkway and Wilkinson Pike that will allow traffic flow at this intersection to continue to be characterized by level of service D during the a.m. and p.m. peak hours. No improvements are recommended at the intersection due to the improvement that is provided by the construction of the Willowoak Trail extension.

B. Medical Center Parkway at Willowoak Trail

The extension of Willowoak Trail will add a fourth leg to this existing three-leg intersection. The addition of the fourth leg and the Clari Park development result in overall traffic flow at this intersection that is characterized by level of service C during the a.m. peak hour and level of service D during the p.m. peak hour.

- The new east approach of Willowoak Trail to Medical Center Parkway should include one westbound right turn lane, one westbound through lane, two westbound left turn lanes, and two eastbound lanes to receive the double left turn lanes from Medical Center Parkway. A median should be included so that the through lanes on Willowoak Trail are aligned on each side of the intersection. The minimum length of the westbound left and right turn lanes should be 150 feet with minimum taper lengths of 150 feet.
- The existing pavement provided for the southbound left turn lanes on Medical Center Parkway is appropriate to accommodate the left turn movement needs. The pavement markings should be modified to provide southbound double left turn lanes on Medical Center Parkway with 225 feet of storage and a taper length of 175 feet.
- The existing pavement markings on the west approach of Willowoak Trail should be modified to remove the channelization between the left turn lanes and right turn lane and provide a through lane to the extension of Willowoak Trail.
- A continuous northbound right turn lane should be constructed on Medical Center Parkway
 along the frontage of Clari Park. To be consistent with similar improvements on Medical
 Center Parkway, the continuous right turn lane should extend approximately 950 feet and
 ending prior to the next upstream signalized intersection at Honeylocust Lane.
- Traffic signal modifications will be required as part of the new approach of Willowoak Trail.
 The signal modifications should include the new pole and mast arm for the westbound
 approach signal heads and other components required to ensure a fully operational traffic
 signal. New timings for the intersection will also be required with the traffic signal
 modification plan.

C. Medical Center Parkway at Honeylocust Lane

The extension of Honeylocust Lane will add a fourth leg to this existing three-leg intersection. The addition of the fourth leg and the Clari Park development result in overall traffic flow at this intersection that is characterized by level of service A during the a.m. peak hour and level of service D during the p.m. peak hour.

- The new east approach of Honeylocust Lane to Medical Center Parkway should include one westbound right turn lane, one westbound through lane, two westbound left turn lanes, and two eastbound lanes to receive the left turn lanes from Medical Center Parkway. A median should be included so that the through lanes on Honeylocust Lane are appropriately aligned on each side of the intersection. The minimum length of the westbound left and right turn lanes should be 150 feet of storage with minimum taper lengths of 150 feet.
- The existing pavement provided for the southbound left turn lanes on Medical Center Parkway is appropriate to accommodate the left turn movement needs. The pavement markings should be modified to provide southbound double left turn lanes on Medical Center Parkway with 225 feet of storage and a taper length of 175 feet.
- The existing pavement markings on the west approach of Honeylocust Lane should be modified to remove the channelization between the left turn lanes and right turn lane and provide a through lane to the extension of Honeylocust Lane.
- A continuous northbound right turn lane should be constructed on Medical Center Parkway
 along the frontage of Clari Park. To be consistent with similar improvements on Medical
 Center Parkway, the continuous right turn lane should extend approximately 890 feet and
 tie to the existing continuous right turn lane that ends at the Redstone Federal Credit Union
 access.
- Traffic signal modifications will be required as part of the new approach of Honeylocust Lane. The signal modifications should include the new pole and mast arm for the westbound approach signal heads and other components required to ensure a fully operational traffic signal. New timings for the intersection will also be required with the traffic signal modification plan.
- With the connection provided by the extension of Willowoak Trail to Robert Rose Drive and the expected levels of service along Willowoak Trail and Robert Rose Drive, the extension of Honeylocust Lane is not required to connect through Clari Park to Robert Rose Drive. Appropriate local network connectivity is provided by Willowoak Trail and Robert Rose Drive.

D. Medical Center Parkway at Maplegrove Drive

After Clari Park is complete, traffic flow at the intersection of Medical Center Parkway and Maplegrove Drive will be characterized by level of service A during the a.m. peak hour and level of service C during the p.m. peak hour.

 No improvements or traffic control modifications are recommended at the intersection of Medical Center Parkway and Maplegrove Drive.

E. Medical Center Parkway at Robert Rose Drive

After Clari Park is complete, traffic flow at the intersection of Medical Center Parkway and Robert Rose Drive will be characterized by level of service B during the a.m. peak hour and level of service C during the p.m. peak hour.

 No improvements or traffic control modifications are recommended at the intersection of Medical Center Parkway and Robert Rose Drive.

F. Wilkinson Pike at Greshampark Drive

The findings and recommendations below are offered for this intersection.

 The City of Murfreesboro's planned reconstruction of Wilkinson Pike to a three-lane roadway with a two-way continuous center turn lane will provide acceptable and appropriate traffic operations at this intersection in the future.

G. Wilkinson Pike at West Park Drive / Clari Park Access #2

The findings and recommendations below are offered for this intersection.

- The City of Murfreesboro's planned reconstruction of Wilkinson Pike to a three-lane roadway with a two-way continuous center turn lane will provide acceptable and appropriate traffic operations at this intersection in the future.
- The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.
- The access to Clari Park at this location may be a gated access. The Clari Park Master Plan pattern book (page 34) states "Access design subject to review and approval by Murfreesboro Planning Commission during site plan approval" for this location in the residential garden district.
- Based on discussions with City staff, left turn lanes will be constructed on Wilkinson Pike
 at West Park Drive and the proposed access to Clari Park with the improvements being for
 a local road section brought to the top of binder. The City will complete the surface course
 of asphalt as part of an upcoming resurfacing project on Wilkinson Pike.
- Traffic volumes at this intersection are not forecasted to satisfy traffic signal warrants even after full build-out of Clari Park. However, a 50-ft. easement would be needed on the project access if a signal were to be installed by others in the future.

H. Wilkinson Pike at Willowoak Trail

The findings and recommendations below are offered for this intersection.

- The City of Murfreesboro's planned reconstruction of Wilkinson Pike to a three-lane roadway with a two-way continuous center turn lane will provide acceptable and appropriate traffic operations at this intersection in the future.
- Based on discussions with City staff, a left turn lane will be constructed on Wilkinson Pike
 at Willowoak Trail with the improvements being for a local road section and brought to the
 top of binder. The City will complete the surface course of asphalt as part of an upcoming
 resurfacing project on Wilkinson Pike.

I. Willowoak Trail at Robert Rose Drive

The findings and recommendations below are offered for this intersection.

 No improvements or traffic control modifications are recommended at the intersection of Willowoak Trail and Robert Rose Drive.

J. Robert Rose Drive at Grassington Street / Clari Park Access #7

The findings and recommendations below are offered for this intersection.

 The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

K. Robert Rose Drive at Marylebone Street / Clari Park Access #8

The findings and recommendations below are offered for this intersection.

The Clari Park access at this intersection should include one lane for traffic exiting the site
and one lane for traffic entering the site.

L. Robert Rose Drive at Sculling Street / Clari Park Access #9

The findings and recommendations below are offered for this intersection.

 The Clari Park access at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.

M. Greshampark Drive at Clari Park Access #1

The findings and recommendations below are offered for this intersection.

- A minimum of one access to Clari Park from Greshampark Drive should be provided south
 of Wilkinson Pike. Access to the section of Clari Park on the southeast corner of
 Greshampark Drive and Wilkinson Pike should include two lanes for traffic exiting the site
 and one lane for traffic entering the site.
- The location of this access will be determined at the site plan level and will subject to review
 and approval by the Murfreesboro Planning Commission at that time. Pavement marking
 modifications on Greshampark Drive that be be necessary due to the location of the access
 will be provided as part of the site plan.

N. Willowoak Trail at Clari Park Access #3 and #4

The findings and recommendations below are offered for this intersection.

- Clari Park Access #3 to the north side of Willowoak Trail at this intersection should include one lane for traffic exiting the site and one lane for traffic entering the site.
- Clari Park Access #4 to the south side of Willowoak Trail at this intersection should include two lanes for traffic exiting the site and one lane for traffic entering the site.
- One eastbound lane on Willowoak Trail can be a right turn only lane into Clari Park.
- The approaches of Clari Park Access #3 and #4 will be stop-controlled at this intersection.
 Stop control will not be placed on the Willowoak Trail approaches to this intersection.

O. Honeylocust Lane at Clari Park Access #5, #6, and #11

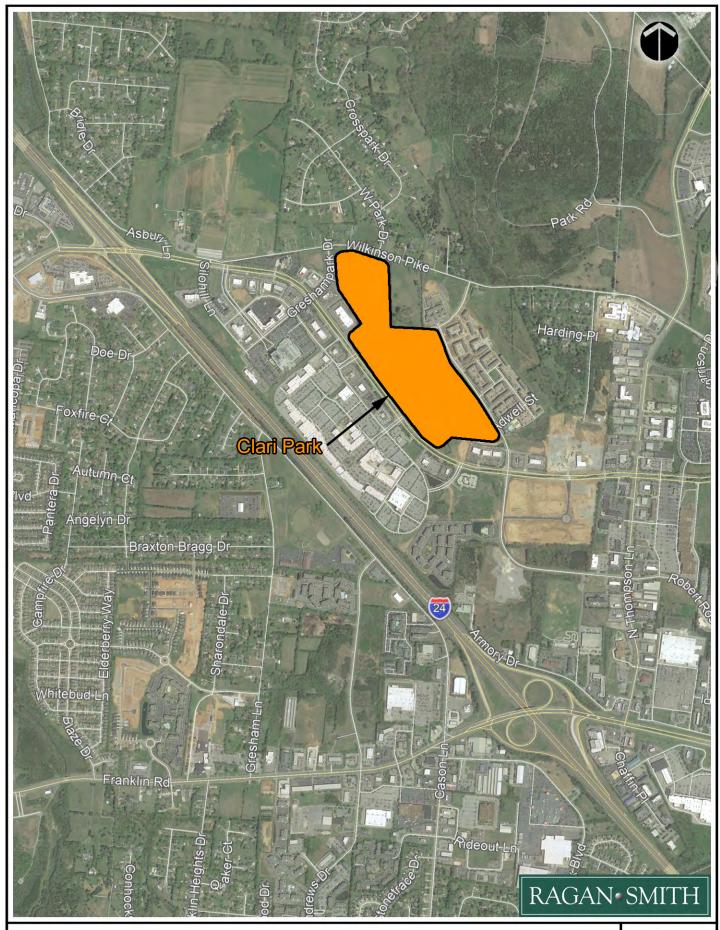
The findings and recommendations below are offered for this intersection.

- Honeylocust Lane is proposed to intersect Clari Lane approximately 400 feet east of Medical Center Parkway. The proposed intersection should include a minimum of two lanes on each approach of Clari Lane and Clari Park access drives to provide one lane for traffic entering the intersection and one lane for traffic exiting the intersection. Additional lanes may be provided on the approach of Honeylocust Lane to the intersection to align with the intersection at Medical Center Parkway.
- The approaches of Clari Park Access #5, #6, and #11 will be stop-controlled at this
 intersection to prioritize the flow of traffic away from the intersection of Medical Center
 Parkway and Honeylocust Lane. Stop control will not be placed on the Honeylocust Lane
 approach to this intersection.

P. Willowoak Trail at Clari Park Access #10

The findings and recommendations below are offered for this intersection.

The Clari Park access at this intersection should include one lane for traffic exiting the site
and one lane for traffic entering the site.



Clari Park Location Map Figure

APPENDIX A TRAFFIC COUNTS

Classified Turn Movement Count

Site 1 of 11

Medical Center Pkwy (North) Grashampark Dr (East) Medical Center Pkwy (South) Grashampark Dr (West)

Lat/Long 35.865444 °, -86.447212°

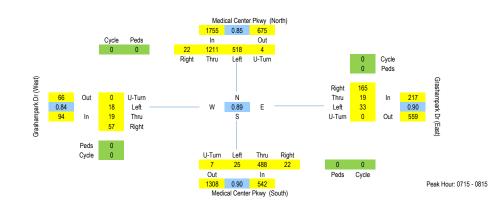
Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020)

Classification: ALL



				South	nbound				Westbound						North	bound				Eastbound							
			Med	ical Cente	erPkwy (i	North)			G	rashampa	ark Dr (Ea	st)			Med	ical Cente	r Pkwy (S	South)			G	rashampa	ırk Dr (We	st)			
_		U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	Int	Rolling
	TIME	1.1	1.2	1.3	1.4	1a	Total	1.5	1.6	1.7	1.8	1b	Total	1.9	1.10	1.11	1.12	1c	Total	1.13	1.14	1.15	1.16	1d	Total	Total	Hour
	0600 - 0615	0	47	84	0	0	131	0	5	1	20	0	26	0	1	71	1	0	73	0	2	3	3	0	8	238	1362
	0615 - 0630	0	62	108	1	0	171	0	7	3	28	0	38	0	1	80	5	0	86	0	2	3	6	0	11	306	1622
	0630 - 0645	0	87	149	1	0	237	0	5	3	42	0	50	1	7	84	1	0	93	0	4	1	5	0	10	390	1914
	0645 - 0700	0	111	161	4	0	276	0	6	3	36	0	45	1	8	87	1	0	97	0	3	2	5	0	10	428	2225
	Hourly Total	0	307	502	6	0	815	0	23	10	126	0	159	2	17	322	8	0	349	0	11	9	19	0	39	1362	-
	0700 - 0715	0	102	199	3	0	304	0	7	3	45	0	55	1	4	107	4	0	116	0	8	6	9	0	23	498	2531
	0715 - 0730	1	122	277	2	0	402	0	6	2	39	0	47	1	5	119	4	0	129	0	2	4	14	0	20	598	2608
	0730 - 0745	1	157	323	8	0	489	0	10	5	44	0	59	3	8	111	4	0	126	0	4	6	17	0	27	701	2533
	0745 - 0800	1	137	377	4	0	519	0	8	1	42	0	51	3	3	122	8	0	136	0	3	6	19	0	28	734	2378
	Hourly Total	3	518	1176	17	0	1714	0	31	11	170	0	212	8	20	459	20	0	507	0	17	22	59	0	98	2531	-
	0800 - 0815	1	102	234	8	0	345	0	9	11	40	0	60	0	9	136	6	0	151	0	9	3	7	0	19	575	2171
	0815 - 0830	2	87	227	3	0	319	0	8	4	51	0	63	0	13	110	4	0	127	0	6	1	7	0	14	523	
	0830 - 0845	2	82	232	9	0	325	0	7	2	34	0	43	2	9	135	8	0	154	0	9	5	10	0	24	546	
	0845 - 0900	1	67	222	7	0	297	0	14	8	45	0	67	1	7	125	7	0	140	0	10	7	6	0	23	527	
	Hourly Total	6	338	915	27	0	1286	0	38	25	170	0	233	3	38	506	25	0	572	0	34	16	30	0	80	2171	
	Grand Total	9	1163	2593	50	0	3815	0	92	46	466	0	604	13	75	1287	53	0	1428	0	62	47	108	0	217	6064	
																											-
	Approach (%)	0.24	30.48	67.97	1.31	0.00		0.00	15.23	7.62	77.15	0.00	1	0.91	5.25	90.13	3.71	0.00		0.00	28.57	21.66	49.77	0.00			
	Total (%)	0.15	19.18	42.76	0.82	0.00	62.91	0.00	1.52	0.76	7.68	0.00	9.96	0.21	1.24	21.22	0.87	0.00	23.55	0.00	1.02	0.78	1.78	0.00	3.58		
	P/Cycle	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0		
	Cars	9	1153	2568	49	-	3779	0	90	44	462	-	596	13	69	1259	52	1 -	1393	0	61	46	106	-	213		
	Truck	0	10	25	1	1 -	36	0	2	2	4	-	8	0	6	28	1	-	35	0	1	1	2	-	4		
					•	-												_						_			
	P/Cycle (%)	0.00	0.00	0.00	0.00	1 -	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00] -	0.00	0.00	0.00	0.00	0.00] -	0.00		
	Cars (%)	100.00	99.14	99.04	98.00	1 -	99.06	0.00	97.83	95.65	99.14	-	98.68	100.00	92.00	97.82	98.11	1 -	97.55	0.00	98.39	97.87	98.15	1 -	98.16		
	Truck (%)	0.00	0.86	0.96	2.00	1 -	0.94	0.00	2.17	4.35	0.86	-	1.32	0.00	8.00	2.18	1.89	1 -	2.45	0.00	1.61	2.13	1.85	-	1.84		

			South	nbound			Westbound								North	bound			Eastbound						
		Med	ical Cente	er Pkwy (N	lorth)			G	rashampa	ark Dr (Ea	st)			Medi	cal Center	r Pkwy (S	outh)			G	rashampa	rk Dr (We	st)		
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	Int
TIME	1.1	1.2	1.3	1.4	1a	Total	1.5	1.6	1.7	1.8	1b	Total	1.9	1.10	1.11	1.12	1c	Total	1.13	1.14	1.15	1.16	1d	Total	Total
0715 - 0730	1	122	277	2	0	402	0	6	2	39	0	47	1	5	119	4	0	129	0	2	4	14	0	20	598
0730 - 0745	1	157	323	8	0	489	0	10	5	44	0	59	3	8	111	4	0	126	0	4	6	17	0	27	701
0745 - 0800	1	137	377	4	0	519	0	8	1	42	0	51	3	3	122	8	0	136	0	3	6	19	0	28	734
0800 - 0815	1	102	234	8	0	345	0	9	11	40	0	60	0	9	136	6	0	151	0	9	3	7	0	19	575
Grand Total	4	518	1211	22	0	1755	0	33	19	165	0	217	7	25	488	22	0	542	0	18	19	57	0	94	2608
			-	-	-	•			-	-						-	-	-			-	-	-		
Approach (%)	0.23	29.52	69.00	1.25	0.00	1	0.00	15.21	8.76	76.04	0.00		1.29	4.61	90.04	4.06	0.00		0.00	19.15	20.21	60.64	0.00	1	
Total (%)	0.15	19.86	46.43	0.84	0.00	67.29	0.00	1.27	0.73	6.33	0.00	8.32	0.27	0.96	18.71	0.84	0.00	20.78	0.00	0.69	0.73	2.19	0.00	3.60	
DUE		85	5%			,		90)%					90)%					84	1%				89%
PHF	100%	82%	80%	69%	1		0%	83%	43%	94%			58%	69%	90%	69%	1		0%	50%	79%	75%			•
P/Cycle	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
Cars	4	512	1200	22	-	1738	0	32	17	163	-	212	7	23	480	22	-	532	0	18	18	57	-	93	2575
Truck	0	6	11	0	-	17	0	1	2	2	-	5	0	2	8	0	-	10	0	0	1	0	-	1	33
P/Cycle (%)	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00
Cars (%)	100.00	98.84	99.09	100.00	-	99.03	0.00	96.97	89.47	98.79	-	97.70	100.00	92.00	98.36	100.00	-	98.15	0.00	100.00	94.74	100.00	-	98.94	98.73
Truck (%)	0.00	1.16	0.91	0.00	-	0.97	0.00	3.03	10.53	1.21	-	2.30	0.00	8.00	1.64	0.00	-	1.85	0.00	0.00	5.26	0.00	-	1.06	1.27

Site 2 of 11

Medical Center Pkwy (North)

Medical Center Pkwy (South)

Willowoak Trail

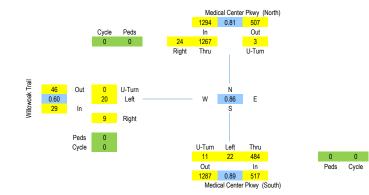
Lat/Long 35.862894°, -86.444927°

Wednesday, February 12, 2020

Weather Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020)

Classification: ALL



Peak Hour: 0715 - 0815

	Southbound									
		Med	ical Cente	r Pkwy (N	lorth)					
	U-Turn		Thru	Right	Peds	App				
TIME	2.1		2.2	2.3	2a	Total				
0600 - 0615	0		93	0	0	93				
0615 - 0630	0		116	2	0	118				
0630 - 0645	1		145	2	0	148				
0645 - 0700	0		166	7	0	173				
Hourly Total	1		520	11	0	532				
0700 - 0715	0		213	3	0	216				
0715 - 0730	0		268	3	0	271				
0730 - 0745	1		362	5	0	368				
0745 - 0800	- 1		389	8	0	398				
Hourly Total	2		1232	19	0	1253				
0800 - 0815	1		248	8	0	257				
0815 - 0830	0		224	6	0	230				
0830 - 0845	3		235	9	0	247				
0845 - 0900	0		233	9	0	242				
Hourly Total	4		940	32	0	976				
Grand Total	7		2692	62	0	2761				
						.				
Approach (%)	0.25		97.50	2.25	0.00					
Total (%)	0.17		64.10	1.48	0.00	65.74				
P/Cycle	0		0	0	-	0				
Cars	7		2663	62	-	2732				
Truck	0		29	0	-	29				
P/Cycle (%)	0.00		0.00	0.00	l	0.00				
Cars (%)	100.00		98.92	100.00	_	98.95				
	0.00		1.08	0.00	-	1.05				
Truck (%)	0.00		1.08	0.00	-	1.05				

		North	bound			Eastbound							
	Medi	cal Cente	r Pkwy (S	outh)				Willowo	ak Trail				
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	Int	Rolling
2.4	2.5	2.6		2c	Total	2.7	2.8		2.9	2d	Total	Total	Hour
0	2	60	1	0	62	0	6		0	0	6	161	868
0	4	81	1	0	85	0	3		0	0	3	206	1038
0	3	74	1	0	77	0	6		0	0	6	231	1238
1	4	86		0	91	0	5		1	0	6	270	1490
1	13	301		0	315	0	20		1	0	21	868	-
0	2	107		0	109	0	5		1	0	6	331	1758
2	2	130		0	134	0	0		1	0	1	406	1840
3	3	104		0	110	0	3		2	0	5	483	1796
3	9	116		0	128	0	9		3	0	12	538	1719
8	16	457		0	481	0	17		7	0	24	1758	-
3	8	134		0	145	0	8		3	0	11	413	1574
0	7	117		0	124	1	6		1	0	8	362	
2	7	135		0	144	0	9		6	0	15	406	
0	7	134		0	141	0	7		3	0	10	393	
5	29	520		0	554	1	30		13	0	44	1574	
14	58	1278		0	1350	1	67		21	0	89	4200	
1.04	4.30	94.67		0.00		1.12	75.28		23.60	0.00			
0.33	1.38	30.43		0.00	32.14	0.02	1.60		0.50	0.00	2.12		
0	0	0	ļ	-	0	0	0		0	-	0		
14	54	1247	ļ	-	1315	1	65		17	-	83		
0	4	31	l	-	35	0	2		4	-	6		
L			1					ı		ı			
0.00	0.00	0.00	ł	-	0.00	0.00	0.00		0.00	-	0.00		
100.00	93.10	97.57	l	-	97.41	100.00	97.01		80.95	-	93.26		
0.00	6.90	2.43	j	-	2.59	0.00	2.99		19.05	-	6.74		

						_
				bound		
		Med	ical Cente	r Pkwy (N	lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	2.1		2.2	2.3	2a	Total
0715 - 0730	0		268	3	0	271
0730 - 0745	1		362	5	0	368
0745 - 0800	1		389	8	0	398
0800 - 0815	1		248	8	0	257
		JI.				
Grand Total	3		1267	24	0	1294
Approach (%)	0.23		97.91	1.85	0.00	
Total (%)	0.16		68.86	1.30	0.00	70.33
PHF		81	1%			
PHF	75%		81%	75%		
P/Cycle	0		0	0	-	0
Cars	3		1255	24	-	1282
Truck	0		12	0	-	12
		ļ!				
P/Cycle (%)	0.00		0.00	0.00	-	0.00
Cars (%)	100.00		99.05	100.00	-	99.07
Truck (%)	0.00		0.95	0.00	-	0.93
		1				

			ound	Eastb					bound	North		
			ak Trail	Willowo				outh)	r Pkwy (S	cal Center	Medi	
Int	App	Peds	Right		Left	U-Turn	App	Peds		Thru	Left	U-Tum
Total	Total	2d	2.9		2.8	2.7	Total	2c		2.6	2.5	2.4
406	1	0	1		0	0	134	0		130	2	2
483	5	0	2		3	0	110	0		104	3	3
538	12	0	3		9	0	128	0		116	9	3
413	11	0	3		8	0	145	0		134	8	3
1840	29	0	9		20	0	517	0		484	22	11
		0.00	31.03		68.97	0.00		0.00		93.62	4.26	2.13
	1.58	0.00	0.49		1.09	0.00	28.10	0.00		26.30	1.20	0.60
86%)%	60					9%	89	
			75%		56%	0%				90%	61%	92%
0	0	-	0		0	0	0			0	0	0
1815	27	-	8		19	0	506	-		475	20	11
25	2	-	1		1	0	11	-		9	2	0
0.00	0.00	-	0.00		0.00	0.00	0.00	-		0.00	0.00	0.00
98.64	93.10	-	88.89		95.00	0.00	97.87	-		98.14	90.91	100.00
1.36	6.90		11.11		5.00	0.00	2.13			1.86	9.09	0.00

Classified Turn Movement Count

Site 3 of 11

Medical Center Pkwy (North)

Medical Center Pkwy (South) Honeylocust Ln

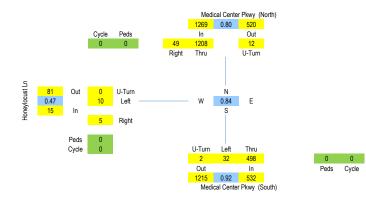
Lat/Long 35.860246°, -86.442494°

Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020) Classification: ALL



Peak Hour: 0715 - 0815

			South	bound		
		Med	ical Cente	r Pkwy (N	lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	3.1		3.2	3.3	3a	Total
0600 - 0615	0	1	88	2	0	90
0615 - 0630	0	1	112	0	0	112
0630 - 0645	0	1	138	5	0	143
0645 - 0700	1	1	159	5	0	165
Hourly Total	1	1	497	12	0	510
0700 - 0715	2	1	208	6	0	216
0715 - 0730	3	1	258	4	0	265
0730 - 0745	4	1	340	9	0	353
0745 - 0800	1		372	22	0	395
Hourly Total	10	1	1178	41	0	1229
0800 - 0815	4	1	238	14	0	256
0815 - 0830	2	1	216	9	0	227
0830 - 0845	1	1	222	6	0	229
0845 - 0900	3	1	225	12	0	240
Hourly Total	10	1	901	41	0	952
		_				
Grand Total	21		2576	94	0	2691
Approach (%)	0.78]	95.73	3.49	0.00	
Total (%)	0.51	1	62.55	2.28	0.00	65.35
P/Cycle	0	ļ	0	0	-	0
Cars	21	ļ	2543	94	-	2658
Truck	0	l	33	0	-	33
P/Cycle (%)	0.00]	0.00	0.00	-	0.00
Cars (%)	100.00	l	98.72	100.00	-	98.77
Truck (%)	0.00]	1.28	0.00	-	1.23

		North	bound					East	oound				
	Medi	cal Center	r Pkwy (S	outh)				Honeylo	ocust Ln				
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	Int	Rolling
3.4	3.5	3.6		3c	Total	3.7	3.8		3.9	3d	Total	Total	Hour
0	0	58		0	58	0	8		0	0	8	156	845
0	2	83		0	85	0	1		1	0	2	199	1012
0	5	79	1	0	84	0	0		2	0	2	229	1216
1	2	91	1	0	94	0	1		1	0	2	261	1452
1	9	311		0	321	0	10		4	0	14	845	-
0	1	100	1	0	101	0	5		1	0	6	323	1734
2	6	128		0	136	0	2		0	0	2	403	1816
0	6	105		0	111	0	1		0	0	1	465	1778
0	- 11	133		0	144	0	2		2	0	4	543	1692
2	24	466	1	0	492	0	10		3	0	13	1734	-
0	9	132		0	141	0	5		3	0	8	405	1539
0	14	120	1	0	134	0	1		3	0	4	365	
1	6	139	1	0	146	0	2		2	0	4	379	
1	8	135	1	0	144	0	1		5	0	6	390	
2	37	526	1	0	565	0	9		13	0	22	1539	
			_										-
5	70	1303		0	1378	0	29		20	0	49	4118	
0.36	5.08	94.56	l	0.00		0.00	59.18		40.82	0.00			
0.12	1.70	31.64	J	0.00	33.46	0.00	0.70	ļ	0.49	0.00	1.19		
0	0	0	1	_	0	0	0		0	١.	0		
5	69	1268	1	-	1342	0	29		14	-	43		
0	1	35	1	-	36	0	0		6	-	6		
			_										
								1		1			
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00		
100.00	98.57	97.31		-	97.39	0.00	100.00		70.00	-	87.76		
0.00	1.43	2.69	İ	-	2.61	0.00	0.00		30.00	-	12.24		

Peak Rolling Hour Flow Rates

Classification: ALL

			South	bound		
		Med	ical Cente	rPkwy (N	lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	3.1		3.2	3.3	3a	Total
0715 - 0730	3		258	4	0	265
0730 - 0745	4		340	9	0	353
0745 - 0800	1		372	22	0	395
0800 - 0815	4		238	14	0	256
Grand Total	12		1208	49	0	1269
					=	•
Approach (%)	0.95		95.19	3.86	0.00	
Total (%)	0.66		66.52	2.70	0.00	69.88
PHF		80)%			
1111	75%		81%	56%		
P/Cycle	0		0	0	-	0
Cars	12		1195	49	-	1256
Truck	0		13	0	-	13
P/Cycle (%)	0.00		0.00	0.00	-	0.00
Cars (%)	100.00		98.92	100.00	-	98.98
Truck (%)	0.00		1.08	0.00	-	1.02

_												
			bound						oound			
		cal Center	r Pkwy (S					Honeylo	ocust Ln			
U-Tum	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	Int
3.4	3.5	3.6		3c	Total	3.7	3.8		3.9	3d	Total	Total
2	6	128		0	136	0	2		0	0	2	403
0	6	105		0	111	0	1		0	0	1	465
0	11	133		0	144	0	2		2	0	4	543
0	9	132		0	141	0	5		3	0	8	405
			•					•				
2	32	498		0	532	0	10		5	0	15	1816
0.38	6.02	93.61	l	0.00	1	0.00	66.67	l	33.33	0.00	1	
0.11	1.76	27.42		0.00	29.30	0.00	0.55		0.28	0.00	0.83	
	92	2%					47	%				84%
25%	73%	94%		•		0%	50%		42%			
0	0	0		-	0	0	0		0	-	0	0
2	31	487		-	520	0	10		4	-	14	1790
0	1	11		-	12	0	0		1	-	1	26
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00
100.00	96.88	97.79		-	97.74	0.00	100.00		80.00	-	93.33	98.57
0.00	3.13	2.21		-	2.26	0.00	0.00		20.00	-	6.67	1.43

Site 4 of 11

Medical Center Pkwy (East) Maplegrove Dr Medical Center Pkwy (West)

Lat/Long 35.858245 °, -86.439332°

Date Wednesday, February 12, 2020

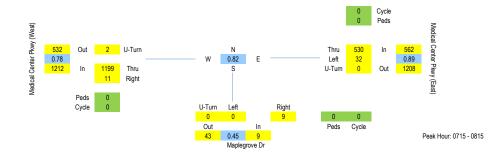
Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020) Classification: ALL

TIME	
0600 - 0615	
0615 - 0630	
0630 - 0645	
0645 - 0700	
Hourly Total	
0700 - 0715	
0715 - 0730	
0730 - 0745	
0745 - 0800	
Hourly Total	
0800 - 0815	
0815 - 0830	
0830 - 0845	
0845 - 0900	
Hourly Total	
Grand Total	
Approach (%)	
Total (%)	
P/Cycle	
Cars	
Truck	
	Ī
P/Cycle (%)	
Cars (%)	
Truck (%)	Ī

TIME
0715 - 0730
0730 - 0745
0745 - 0800
0800 - 0815
Grand Total
Approach (%)
Total (%)
PHF
FIII
P/Cycle
Cars
Truck
P/Cycle (%)
Cars (%)
Truck (%)



				tbound					North	bound					East	oound				
L		Me	dical Cent	ter Pkwy (East)				Maple	grove Dr				Med	lical Cente	er Pkwy (V	Vest)			
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int	Rolling
	4.1	4.2	4.3		4b	Total	4.4	4.5		4.6	4c	Total	4.7		4.8	4.9	4d	Total	Total	Hour
	0	4	59		0	63	0	0		8	0	8	0		81	0	0	81	152	852
	0	3	84		0	87	0	0		1	0	1	0		112	0	0	112	200	1009
	0	6	86		0	92	0	0		0	0	0	1		144	0	0	145	237	1208
	0	6	91		0	97	0	0		1	0	1	0		164	1	0	165	263	1414
	0	19	320		0	339	0	0		10	0	10	1		501	1	0	503	852	-
	0	1	106		0	107	0	0		2	0	2	1		197	2	0	200	309	1697
	0	0	129		0	129	0	0		2	0	2	0		265	3	0	268	399	1783
	0	6	113		0	119	0	0		1	0	1	1		321	1	0	323	443	1754
	0	12	144		0	156	0	0		1	0	1	- 1		382	6	0	389	546	1681
	0	19	492		0	511	0	0		6	0	6	3		1165	12	0	1180	1697	-
	0	14	144		0	158	0	0		5	0	5	0		231	1	0	232	395	1525
	1	6	131		0	138	0	0		1	0	1	4		224	3	0	231	370	
I	0	6	146		0	152	0	0		1	0	1	0		214	3	0	217	370	i
I	0	8	136		0	144	0	2		3	0	5	1		237	3	0	241	390	i
ı	1	34	557		0	592	0	2		10	0	12	5		906	10	0	921	1525	i
				_					_											_
Ī	1	72	1369		0	1442	0	2		26	0	28	9	ľ	2572	23	0	2604	4074	i
				="					_			-								
	0.07	4.99	94.94		0.00		0.00	7.14		92.86	0.00		0.35		98.77	0.88	0.00			
	0.02	1.77	33.60		0.00	35.40	0.00	0.05		0.64	0.00	0.69	0.22		63.13	0.56	0.00	63.92		
	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0		
	1	65	1334		-	1400	0	2		25	-	27	8		2536	21	-	2565		
	0	7	35		-	42	0	0		1	-	1	1		36	2	-	39		
ı	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00	Ì	0.00	0.00	-	0.00		
	100.00	90.28	97.44		-	97.09	0.00	100.00		96.15	-	96.43	88.89		98.60	91.30	-	98.50		
Į	0.00	9.72	2.56]	-	2.91	0.00	0.00		3.85	-	3.57	11.11		1.40	8.70	-	1.50		

		West	bound			Northbound						Eastbound						
	Med	dical Cent	er Pkwy (E	East)				Mapleg	grove Dr				Med	lical Cente	er Pkwy (V	Vest)		
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int
4.1	4.2	4.3		4b	Total	4.4	4.5		4.6	4c	Total	4.7		4.8	4.9	4d	Total	Total
0	0	129		0	129	0	0		2	0	2	0		265	3	0	268	399
0	6	113		0	119	0	0		1	0	1	1		321	1	0	323	443
0	12	144	1	0	156	0	0		1	0	1	1		382	6	0	389	546
0	14	144	1	0	158	0	0		5	0	5	0		231	1	0	232	395
0	32	530		0	562	0	0		9	0	9	2		1199	11	0	1212	1783
			_					_			_							
0.00	5.69	94.31	1	0.00		0.00	0.00		100.00	0.00		0.17		98.93	0.91	0.00		
0.00	1.79	29.73		0.00	31.52	0.00	0.00		0.50	0.00	0.50	0.11		67.25	0.62	0.00	67.98	
	89	9%					45	5%					78	3%				82%
0%	57%	92%				0%	0%		45%			50%		78%	46%			
0	0	0	1	-	0	0	0		0	-	0	0		0	0		0	0
0	30	520	1	-	550	0	0		8	-	8	2		1187	9		1198	1756
0	2	10		-	12	0	0		1	-	1	0		12	2		14	27
			_					_										
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00		0.00	0.00
0.00	93.75	98.11		-	97.86	0.00	0.00		88.89	-	88.89	100.00		99.00	81.82		98.84	98.49
0.00	6.25	1.89		-	2.14	0.00	0.00		11.11	-	11.11	0.00		1.00	18.18		1.16	1.51

Classified Turn Movement Count

Site 5 of 11 Robert Rose Dr (North) Medical Center Pkwy (East) Robert Rose Dr (South) Medical Center Pkwy (West)

Lat/Long 35.857916°, -86.436850°

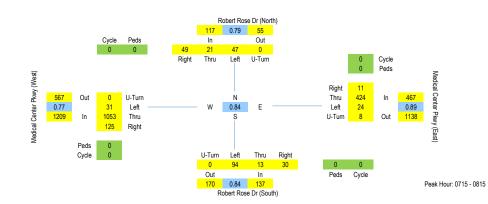
Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020)

Classification: ALL



				nbound						bound					North							ound				
			Robert Ros	se Dr (Nor					dical Cent						obert Ros	e Dr (Sout					dical Cente					
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	Int	Rolling
TIME	5.1	5.2	5.3	5.4	5a	Total	5.5	5.6	5.7	5.8	5b	Total	5.9	5.10	5.11	5.12	5c	Total	5.13	5.14	5.15	5.16	5d	Total	Total	Hour
0600 - 0615	0	5	3	17	0	25	3	0	37	0	0	40	0	10	0	4	0	14	0	2	79	13	0	94	173	935
0615 - 0630	0	2	4	19	0	25	2	2	55	4	0	63	0	15	2	5	0	22	0	4	91	11	0	106	216	1091
0630 - 0645	0	7	2	23	0	32	0	2	56	3	0	61	0	12	1	7	0	20	0	2	134	10	0	146	259	1319
0645 - 0700	0	2	5	16	0	23	0	2	68	2	0	72	0	13	2	7	0	22	0	5	141	24	0	170	287	1541
Hourly Total	0	16	14	75	0	105	5	6	216	9	0	236	0	50	5	23	0	78	0	13	445	58	0	516	935	-
0700 - 0715	0	7	5	14	0	26	1	2	75	4	0	82	0	20	2	7	0	29	0	3	164	25	0	192	329	1830
0715 - 0730	0	16	6	15	0	37	1	6	99	1	0	107	0	16	3	7	0	26	0	4	233	37	0	274	444	1930
0730 - 0745	0	7	9	11	0	27	3	5	89	3	0	100	0	27	4	10	0	41	0	8	277	28	0	313	481	1886
0745 - 0800	0	13	2	7	0	22	3	4	120	2	0	129	0	22	3	8	0	33	0	14	335	43	0	392	576	1797
Hourly Total	0	43	22	47	0	112	8	17	383	10	0	418	0	85	12	32	0	129	0	29	1009	133	0	1171	1830	-
0800 - 0815	0	11	4	16	0	31	1	9	116	5	0	131	0	29	3	5	0	37	0	5	208	17	0	230	429	1637
0815 - 0830	0	4	5	13	0	22	1	4	109	2	0	116	0	15	3	10	0	28	2	4	201	27	0	234	400	
0830 - 0845	0	5	2	8	0	15	1	3	126	1	0	131	0	22	4	9	0	35	0	3	183	25	0	211	392	
0845 - 0900	0	8	3	3	0	14	0	7	122	7	0	136	0	16	4	3	0	23	0	10	194	39	0	243	416	
Hourly Total	0	28	14	40	0	82	3	23	473	15	0	514	0	82	14	27	0	123	2	22	786	108	0	918	1637	
Grand Total	0	87	50	162	0	299	16	46	1072	34	0	1168	0	217	31	82	0	330	2	64	2240	299	0	2605	4402	
,																										
						_						_														
Approach (%)	0.00	29.10	16.72	54.18	0.00		1.37	3.94	91.78	2.91	0.00		0.00	65.76	9.39	24.85	0.00		0.08	2.46	85.99	11.48	0.00			
Total (%)	0.00	1.98	1.14	3.68	0.00	6.79	0.36	1.04	24.35	0.77	0.00	26.53	0.00	4.93	0.70	1.86	0.00	7.50	0.05	1.45	50.89	6.79	0.00	59.18		
					_						_															
P/Cycle	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0		
Cars	0	87	49	160	-	296	16	45	1035	33	-	1129	0	214	30	81	-	325	2	61	2211	294	-	2568		
Truck	0	0	1	2	-	3	0	1	37	1	-	39	0	3	1	1	-	5	0	3	29	5	-	37		
					_						_												-			
P/Cycle (%)	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00		
Cars (%)	0.00	100.00	98.00	98.77	-	99.00	100.00	97.83	96.55	97.06	-	96.66	0.00	98.62	96.77	98.78	-	98.48	100.00	95.31	98.71	98.33	-	98.58		
Truck (%)	0.00	0.00	2.00	1.23	-	1.00	0.00	2.17	3.45	2.94] -	3.34	0.00	1.38	3.23	1.22	-	1.52	0.00	4.69	1.29	1.67	-	1.42		

			South	bound					West	bound					North	bound					East	ound			1
		R	lobert Ros	e Dr (Nort	h)			Med	dical Cent	er Pkwy (E	ast)			R	obert Ros	e Dr (Sout	th)			Med	dical Cente	er Pkwy (V	Vest)		
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	Int
TIME	5.1	5.2	5.3	5.4	5a	Total	5.5	5.6	5.7	5.8	5b	Total	5.9	5.10	5.11	5.12	5c	Total	5.13	5.14	5.15	5.16	5d	Total	Total
0715 - 0730	0	16	6	15	0	37	1	6	99	1	0	107	0	16	3	7	0	26	0	4	233	37	0	274	444
0730 - 0745	0	7	9	11	0	27	3	5	89	3	0	100	0	27	4	10	0	41	0	8	277	28	0	313	481
0745 - 0800	0	13	2	7	0	22	3	4	120	2	0	129	0	22	3	8	0	33	0	14	335	43	0	392	576
0800 - 0815	0	11	4	16	0	31	1	9	116	5	0	131	0	29	3	5	0	37	0	5	208	17	0	230	429
Grand Total	0	47	21	49	0	117	8	24	424	11	0	467	0	94	13	30	0	137	0	31	1053	125	0	1209	1930
						-																			
Approach (%)	0.00	40.17	17.95	41.88	0.00		1.71	5.14	90.79	2.36	0.00		0.00	68.61	9.49	21.90	0.00		0.00	2.56	87.10	10.34	0.00	1	
Total (%)	0.00	2.44	1.09	2.54	0.00	6.06	0.41	1.24	21.97	0.57	0.00	24.20	0.00	4.87	0.67	1.55	0.00	7.10	0.00	1.61	54.56	6.48	0.00	62.64	
PHF		79	9%					89	9%					84	1%					77	7%				84%
FIII	0%	73%	58%	77%			67%	67%	88%	55%			0%	81%	81%	75%	1		0%	55%	79%	73%			
P/Cycle	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
Cars	0	47	20	48	-	115	8	24	412	11	-	455	0	94	13	30	-	137	0	30	1043	124	-	1197	1904
Truck	0	0	1	1	-	2	0	0	12	0	-	12	0	0	0	0	-	0	0	1	10	1	-	12	26
P/Cycle (%)	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00
Cars (%)	0.00	100.00	95.24	97.96	-	98.29	100.00	100.00	97.17	100.00	-	97.43	0.00	100.00	100.00	100.00	-	100.00	0.00	96.77	99.05	99.20	-	99.01	98.65
Truck (%)	0.00	0.00	4.76	2.04		1.71	0.00	0.00	2.83	0.00		2.57	0.00	0.00	0.00	0.00		0.00	0.00	3.23	0.95	0.80		0.99	1.35

Site 6 of 11

Wilkinson Pike (East) Greshampark Dr Wilkinson Pike (West)

Lat/Long 35.867615°, -86.446294°

Date Wednesday, February 12, 2020

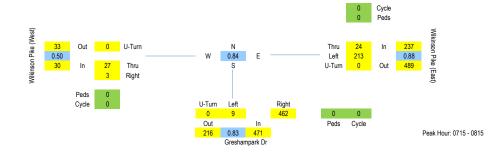
Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020) Classification: ALL

TIME	
0600 - 0615	
0615 - 0630	
0630 - 0645	
0645 - 0700	
Hourly Total	
0700 - 0715	
0715 - 0730	
0730 - 0745	
0745 - 0800	
Hourly Total	
0800 - 0815	
0815 - 0830	
0830 - 0845	П
0845 - 0900	
Hourly Total	
Grand Total	
Approach (%)	
Total (%)	
P/Cycle	П
Cars	
Truck	
P/Cycle (%)	
Cars (%)	
Truck (%)	

TIME
0715 - 0730
0730 - 0745
0745 - 0800
0800 - 0815
Grand Total
Approach (%)
Total (%)
PHF
FRIF
P/Cycle
Cars
Truck
P/Cycle (%)
Cars (%)
Truck (%)



		West	bound			Northbound								East	oound			l	
	1	Wilkinson	Pike (East	t)				Gresha	mpark Dr				١	Wilkinson	Pike (Wes	it)			
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App	Int	Rolling
6.1	6.2	6.3		6b	Total	6.4	6.5		6.6	6c	Total	6.7		6.8	6.9	6d	Total	Total	Hour
0	24	0		0	24	0	0	1	34	0	34	0		2	0	0	2	60	412
0	33	0		0	33	0	0	1	50	0	50	0		0	0	0	0	83	491
0	48	1		0	49	0	0	1	77	0	77	0		0	0	0	0	126	578
0	42	1		0	43	0	0	1	99	0	99	0		1	0	0	1	143	671
0	147	2		0	149	0	0		260	0	260	0		3	0	0	3	412	-
0	54	1		0	55	0	0	1	82	0	82	0		2	0	0	2	139	733
0	49	5		0	54	0	4		106	0	110	0		6	0	0	6	170	738
0	54	8		0	62	0	1	1	141	0	142	0		14	1	0	15	219	715
0	59	8		0	67	0	3	1	129	0	132	0		6	0	0	6	205	613
0	216	22		0	238	0	8	1	458	0	466	0		28	1	0	29	733	-
0	51	3		0	54	0	1	1	86	0	87	0		1	2	0	3	144	520
0	62	2		0	64	0	0	1	79	0	79	0		2	2	0	4	147	
0	46	2		0	48	0	0	1	68	0	68	0		0	1	0	1	117	1
0	54	3		0	57	0	0	1	52	0	52	0		1	2	0	3	112	1
0	213	10		0	223	0	1	1	285	0	286	0		4	7	0	11	520	1
			_					•											•
0	576	34		0	610	0	9	1	1003	0	1012	0	Ï	35	8	0	43	1665	1
			_					•											•
0.00	94.43	5.57		0.00	1	0.00	0.89	1	99.11	0.00		0.00	1	81.40	18.60	0.00			
0.00	34.59	2.04		0.00	36.64	0.00	0.54	1	60.24	0.00	60.78	0.00		2.10	0.48	0.00	2.58		
								_											
0	0	0		-	0	0	0	1	0	-	0	0	Ì	0	0	-	0		
0	569	32		-	601	0	9	1	996	-	1005	0		35	7	1 -	42		
0	7	2		-	9	0	0	1	7	-	7	0		0	1	-	1		
	•	•	•												•	4			
0.00	0.00	0.00	1	-	0.00	0.00	0.00	1	0.00		0.00	0.00	Ì	0.00	0.00	-	0.00		
0.00	98.78	94.12	1	-	98.52	0.00	100.00	1	99.30	-	99.31	0.00		100.00	87.50	-	97.67		
0.00	1.22	5.88	1	-	1.48	0.00	0.00	1	0.70	-	0.69	0.00		0.00	12.50	-	2.33		

		West	bound					North	bound					East	oound			
	1	Vilkinson	Pike (East	t)				Gresha	mpark Dr				١	Vilkinson I	Pike (Wes	t)		
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int
6.1	6.2	6.3		6b	Total	6.4	6.5		6.6	6c	Total	6.7		6.8	6.9	6d	Total	Total
0	49	5	1	0	54	0	4	1	106	0	110	0		6	0	0	6	170
0	54	8	1	0	62	0	1	1	141	0	142	0		14	1	0	15	219
0	59	8		0	67	0	3		129	0	132	0		6	0	0	6	205
0	51	3		0	54	0	1		86	0	87	0		1	2	0	3	144
0	213	24	1	0	237	0	9		462	0	471	0		27	3	0	30	738
													ii.					
0.00	89.87	10.13		0.00		0.00	1.91		98.09	0.00		0.00		90.00	10.00	0.00		
0.00	28.86	3.25		0.00	32.11	0.00	1.22		62.60	0.00	63.82	0.00		3.66	0.41	0.00	4.07	
	88	3%						3%					50)%				84%
0%	90%	75%	1			0%	56%	1	82%			0%		48%	38%			
0	0	0	1	-	0	0	0	1	0	-	0	0		0	0	-	0	0
0	209	23	1	-	232	0	9	1	458	-	467	0		27	3	-	30	729
0	4	1]	-	5	0	0		4	-	4	0		0	0	-	0	9
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	0.00
0.00	98.12	95.83	1	-	97.89	0.00	100.00		99.13	-	99.15	0.00		100.00	100.00	-	100.00	98.78
0.00	1.88	4.17	j	-	2.11	0.00	0.00	j	0.87	-	0.85	0.00		0.00	0.00	-	0.00	1.22

Site 7 of 11 W Park Dr Wilkinson Pike (East)

Wilkinson Pike (West)

Lat/Long 35.867391°, -86.443637°

Date Wednesday, February 12, 2020

Weather Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020) Classification: ALL

						WP	ark Dr				
					52	0.87	29				
		Cycle	Peds		In		Out				
		0	0	31		21	0				
				Right		Left	U-Turn		0	Cycle	
									0	Peds	
								Right	12		
235	Out	0	U-Turn			N		Thru	204	In	21
0.80		17	Left		W	0.84	Ε .				0.9
489	In	472	Thru			S		U-Turn	0	Out	493
	Peds	0									
	Cycle	0									

Peak Hour: 0715 - 0815

			South	bound					West	bound		
			W Pa	rk Dr				1	Wilkinson	Pike (East	:)	
	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	7.1	7.2		7.3	7a	Total	7.4		7.5	7.6	7b	Total
0600 - 0615	0	2		5	0	7	0		18	1	0	19
0615 - 0630	0	2		8	0	10	0		26	0	0	26
0630 - 0645	0	6		10	0	16	0		39	0	0	39
0645 - 0700	0	4		11	0	15	0		31	1	0	32
Hourly Total	0	14		34	0	48	0		114	2	0	116
0700 - 0715	0	3		9	0	12	0		46	2	0	48
0715 - 0730	0	5		7	0	12	0		47	2	0	49
0730 - 0745	0	5		6	0	11	0		55	5	0	60
0745 - 0800	0	5		10	0	15	0		58	2	0	60
Hourly Total	0	18		32	0	50	0		206	11	0	217
0800 - 0815	0	6		8	0	14	0		44	3	0	47
0815 - 0830	0	4		10	0	14	0		55	1	0	56
0830 - 0845	0	6		5	0	11	0		42	2	0	44
0845 - 0900	0	2		3	0	5	0		54	2	0	56
Hourly Total	0	18		26	0	44	0		195	8	0	203
Grand Total	0	50		92	0	142	0		515	21	0	536
Approach (%)	0.00	35.21		64.79	0.00		0.00		96.08	3.92	0.00	1
Total (%)	0.00	2.92		5.37	0.00	8.28	0.00		30.05	1.23	0.00	31.27
1001 (70)	0.00	2.02		0.01	0.00	0.20	0.00		00.00	1.20	0.00	01.21
P/Cycle	0	0		0	-	0	0		0	0	-	0
Cars	0	49		92	-	141	0		506	21	-	527
Truck	0	1		0	-	1	0		9	0	-	9
P/Cycle (%)	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	98.00		100.00	-	99.30	0.00		98.25	100.00	-	98.32
Truck (%)	0.00	2.00		0.00	-	0.70	0.00		1.75	0.00	-	1.68

Peak Rolling Hour Flow Rates	
Classification: ALL	

			South	bound					West	bound		
			W Pa	ark Dr				1	Vilkinson	Pike (East	t)	
	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App
TIME	7.1	7.2	l	7.3	7a	Total	7.4		7.5	7.6	7b	Total
0715 - 0730	0	5		7	0	12	0		47	2	0	49
0730 - 0745	0	5	l	6	0	11	0		55	5	0	60
0745 - 0800	0	5	Ī	10	0	15	0		58	2	0	60
0800 - 0815	0	6	Ī	8	0	14	0		44	3	0	47
			_									
Grand Total	0	21	Ī	31	0	52	0	Ĭ	204	12	0	216
Approach (%)	0.00	40.38	Ī	59.62	0.00	1	0.00	Ì	94.44	5.56	0.00	1
Total (%)	0.00	2.77		4.10	0.00	6.87	0.00		26.95	1.59	0.00	28.53
PHF		87	7%					90)%			
PHF	0%	88%		78%			0%		88%	60%		
P/Cycle	0	0		0	-	0	0		0	0	-	0
Cars	0	21	Ī	31	-	52	0		200	12	-	212
Truck	0	0	Ī	0	-	0	0		4	0	-	4
P/Cycle (%)	0.00	0.00	l	0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	100.00	l	100.00	-	100.00	0.00		98.04	100.00	-	98.15
Truck (%)	0.00	0.00	l	0.00		0.00	0.00		1.96	0.00	-	1.85

			oound				
	- \	Vilkinson I	Pike (Wes	t)			
U-Turn	Left	Thru		Peds	App	Int	Rolling
7.7	7.8	7.9		7d	Total	Total	Hour
0	0	34		0	34	60	425
0	2	47		0	49	85	510
0	0	77		0	77	132	601
0	3	98		0	101	148	693
0	5	256		0	261	425	٠
0	5	80		0	85	145	751
0	3	112		0	115	176	757
0	7	146		0	153	224	731
0	3	128		0	131	206	628
0	18	466		0	484	751	-
0	4	86		0	90	151	538
0	2	78		0	80	150	
0	2	64		0	66	121	
0	2	53		0	55	116	
0	10	281		0	291	538	
0	33	1003		0	1036	1714	
			_		_		
0.00	3.19	96.81		0.00			
0.00	1.93	58.52		0.00	60.44		
			_				
0	0	0		-	0		
0	32	997		-	1029		
0	1	6		-	7		
0.00	0.00	0.00		-	0.00		
0.00	96.97	99.40		-	99.32		
0.00	3.03	0.60		-	0.68		

		East	oound			i
	٧	Vilkinson I	Pike (Wes	t)		ł
U-Turn	Left	Thru		Peds	App	Int
7.7	7.8	7.9		7d	Total	Total
0	3	112		0	115	176
0	7	146		0	153	224
0	3	128		0	131	206
0	4	86		0	90	151
0	17	472		0	489	757
			=			
						ł
0.00	3.48	96.52		0.00		ł
0.00	2.25	62.35		0.00	64.60	
	80	1%				84%
0%	61%	81%				
0	0	0		,	0	0
0	17	468		-	485	749
0	0	4		-	4	8
						ł
0.00	0.00	0.00		-	0.00	0.00
0.00	100.00	99.15		-	99.18	98.94
0.00	0.00	0.85		-	0.82	1.06

Site 8 of 11

Wilkinson Pike (East) Willow Oak Trail Wilkinson Pike (West)

Lat/Long 35.866080°, -86.438822°

Date Wednesday, February 12, 2020

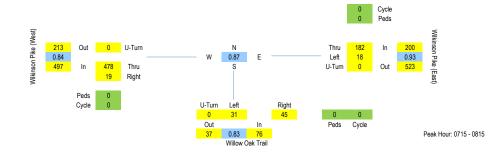
Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020) Classification: ALL

TIME
0600 - 0615
0615 - 0630
0630 - 0645
0645 - 0700
Hourly Total
0700 - 0715
0715 - 0730
0730 - 0745
0745 - 0800
Hourly Total
0800 - 0815
0815 - 0830
0830 - 0845
0845 - 0900
Hourly Total
Grand Total
Approach (%)
Total (%)
P/Cycle
Cars
Truck
P/Cycle (%)
Cars (%)
Truck (%)

TIME	
0715 - 0730	
0730 - 0745	۱
0745 - 0800	
0800 - 0815	
Grand Total	
	1
	ı
Approach (%)	
Total (%)	
PHF	
FIIF	۱
P/Cycle	1
Cars	1
Truck	1
	1
	ı
P/Cycle (%)	
Cars (%)	
Truck (%)	1



8.1 8.2 8.3 8b Total 8.4 8.5 8.6 8c Total 8.7 8.8 8.9 8d Total Total HI 0 1 13 0 14 0 6 3 0 9 0 34 2 0 36 59 4 0 2 28 0 30 0 11 3 0 14 0 78 7 0 48 78 4 0 2 20 0 22 0 11 10 0 21 0 99 3 0 102 145 6 0 2 20 0 22 0 11 10 0 21 0 99 3 0 102 145 6 0 2 29 0 31 0 14 10 0 24 0 81 2			West	bound					North	bound					East	oound				
8.1 8.2 8.3 8b Total 8.4 8.5 3.0 9 0 34 2 0 36 59 4 0 1 13 0 14 0 6 2 0 12 0 34 2 0 36 59 4 0 2 2.8 0 30 0 11 3 0 14 0 78 7 0 48 78 4 0 2 20 0 22 0 11 10 0 21 0 99 3 0 102 145 6 0 2 29 0 31 0 14 10 0 24 0 258 13 0 271 411 10 0 22 29 0 31 0 14 10 0 22 0 113 0 141 110 <		1	Wilkinson	Pike (Eas	t)				Willow	Oak Trail				1	Wilkinson	Pike (Wes	t)			
0 1 13 0 14 0 6 0 3 15 0 18 0 10 2 0 12 0 47 1 0 48 78 4 0 2 20 0 22 0 11 10 0 21 0 99 3 0 102 145 6 0 2 20 0 22 0 11 10 0 21 0 99 3 0 102 145 6 0 8 76 0 84 0 38 18 0 56 0 258 13 0 271 411 10 0 24 0 0 33 0 144 16 23 0 159 3 0 171 411 111 111 111 111 111 111 111 111 111	U-Tum	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App	Int	Rolling
0 3 15 0 18 0 10 2 0 12 0 47 1 0 48 78 4 0 2 20 0 11 10 0 21 0 78 7 0 85 129 5 0 2 20 111 10 0 21 0 99 3 0 102 145 6 0 8 76 0 84 0 38 18 0 56 0 258 13 0 271 411 10 0 24 0 81 2 0 83 138 7 0 2 29 0 31 0 14 0 23 0 109 7 0 116 185 7 0 2 49 0 51 0 9 14 0 23 0	8.1	8.2	8.3		8b	Total	8.4	8.5		8.6	8c	Total	8.7		8.8	8.9	8d	Total	Total	Hour
0 2 28 0 2 20 0 2 20 0 2 20 0 84 0 38 0 2 29 0 31 0 14 10 0 24 0 81 2 0 31 0 2 29 0 31 0 14 10 0 24 0 81 2 0 83 138 7 0 6 40 0 46 0 9 14 0 23 0 109 7 0 116 185 7 0 2 49 0 51 0 9 14 0 23 0 145 3 0 148 222 7 0 14 168 0 182 0 36 50 0 <t< td=""><td>0</td><td>1</td><td>13</td><td></td><td>0</td><td>14</td><td>0</td><td>6</td><td>1</td><td>3</td><td>0</td><td>9</td><td>0</td><td></td><td>34</td><td>2</td><td>0</td><td>36</td><td>59</td><td>411</td></t<>	0	1	13		0	14	0	6	1	3	0	9	0		34	2	0	36	59	411
0 2 20 0 8 76 0 2 29 0 84 0 38 0 2 29 0 6 40 0 2 29 0 46 0 9 14 0 23 0 2 49 0 51 0 9 14 0 23 0 14 168 0 0 14 168 0 0 14 168 0 0 14 168 0 0 142 0 16 0 0 14 168 0 139 20 0 14 168 0 139 20 0 5 51 0 56 0 3 0 5 51 0 56 0	0	3	15		0	18	0	10	1	2	0	12	0		47	1	0	48	78	490
0 8 76 0 2 29 0 31 0 14 10 0 24 0 81 2 0 83 138 7 0 6 40 0 0 46 0 9 14 0 23 0 19 7 0 116 185 7 0 4 50 0 14 168 0 54 0 4 0 182 0 83 138 7 18 0 23 0 19 7 0 116 185 7 145 3 0 146 222 7 145 3 0 148 222 7 145 3 0 148 222 7 145 3 0 148 222 7 145 3 0 148 222 7 145 3 0 148 222 7 146 0 139 49 0 9 15 0 182 0 36 0 6 43 0 49 0 9 15 0 14 0 89 15 0 14 0 89 16 0 470 16 0 486 754 0 2 3 7 0 2 3 7 0 4 53 0 5 51 0 56 0 3 0 57 0 5 8 0 11 0 7 10 0 4 6 75 10 0 86 0 76 139 0 2 3 7 0 4 53 0 57 0 5 8 0 11 0 7 76 4 0 79 129 0 17 184 0 201 0 23 0 1187 0 1018 43 0 1061 1715	0	2	28		0	30	0	11	1	3	0	14	0		78	7	0	85	129	597
0 2 29 0 6 40 0 6 40 0 2 49 0 2 49 0 4 50 0 4 50 0 14 168 0 14 168 0 14 168 0 14 168 0 14 168 0 182 0 36 50 0 86 0 40 182 0 36 50 0 86 0 40 7 0 16 40 7 0 16 40 7 0 16 40 7 0 16 40 7 0 16 50 14 0 89 50 14 0 89 6 <t< td=""><td>0</td><td>2</td><td>20</td><td></td><td>0</td><td>22</td><td>0</td><td>11</td><td>1</td><td>10</td><td>0</td><td>21</td><td>0</td><td></td><td>99</td><td>3</td><td>0</td><td>102</td><td>145</td><td>690</td></t<>	0	2	20		0	22	0	11	1	10	0	21	0		99	3	0	102	145	690
0 6 40 0 2 49 0 4 50 0 4 50 0 14 168 0 14 188 0 6 43 0 6 43 0 56 0 0 56 0 0 56 0 0 56 0 0 56 0 0 39 0 0 4 53 0 17 184 0 23 0 0 4 0 0 4 0 0 4 0 0 4 0 0 4 0 4 0 0 4 0 0 17 184 0 201 0 22 0 <td< td=""><td>0</td><td>8</td><td>76</td><td></td><td>0</td><td>84</td><td>0</td><td>38</td><td>1</td><td>18</td><td>0</td><td>56</td><td>0</td><td></td><td>258</td><td>13</td><td>0</td><td>271</td><td>411</td><td>-</td></td<>	0	8	76		0	84	0	38	1	18	0	56	0		258	13	0	271	411	-
0 2 49 0 4 50 0 4 50 0 14 168 0 14 168 0 6 43 0 6 43 0 49 0 9 5 0 14 0 89 5 0 94 4 0 7 0 70 2 37 0 4 53 0 0 4 53 0 4 53 0 4 53 0 5 0 17 184 0 23 0 17 184 0 201 0 22 0 45 0 39 428 0 467 0 97 90 187 0 0	0	2	29		0	31	0	14	1	10	0	24	0		81	2	0	83	138	754
0 4 50 0 14 168 0 14 168 0 6 43 0 5 51 0 55 14 0 2 37 0 2 37 0 4 53 0 17 184 0 23 0 23 0 39 428 0 467 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <td>0</td> <td>6</td> <td>40</td> <td></td> <td>0</td> <td>46</td> <td>0</td> <td>9</td> <td>1</td> <td>14</td> <td>0</td> <td>23</td> <td>0</td> <td></td> <td>109</td> <td>7</td> <td>0</td> <td>116</td> <td>185</td> <td>773</td>	0	6	40		0	46	0	9	1	14	0	23	0		109	7	0	116	185	773
0 14 168 0 182 0 36 50 0 86 0 470 16 0 486 754 0 90 0 16 0 486 754 0 89 5 0 94 157 5 0 11 0 0 71 5 0 94 157 5 0 11 0 71 5 0 74 0 77 5 0 71 5 0 76 133 0 75 4 0 79 129 55 0 11 0 75 4 0 79 129 55 0 0 55 0 0 55 0 0 55 0 0 55 0 11 0 290 14 0 304 550 125 290 14 0 304 550 125 290 14 0 304	0	2	49		0	51	0	9	1	14	0	23	0		145	3	0	148	222	727
0 6 43 0 5 51 0 5 6 0 3 0 2 37 0 39 0 6 5 0 11 0 75 4 0 79 125 0 17 184 0 201 0 23 0 5 0 11 0 75 4 0 79 129 0 39 428 0 467 0 97 90 0 187 0 1018 43 0 1061 1715 0 0 227 24.96 0 0 5.66 48.13 0.00 0.00 95.95 4.05 0.00 0 0 0 0 0 0 0 0 0 0 0 0 93.95 4.05 0.00 59.36 2.51 0.00 61.87	0	4	50		0	54	0	4	1	12	0	16	0		135	4	0	139	209	634
0 5 51 0 56 0 3 4 0 7 0 71 5 0 76 139 0 2 37 0 39 0 6 8 0 11 0 75 4 0 79 129 0 4 53 0 57 0 5 0 13 0 55 0 0 55 0 0 55 0 0 55 0 0 55 0 0 55 0 0 55 0 0 55 0 0 55 0 0 55 0 0 0 55 0 0 0 55 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0	14	168		0	182	0	36	1	50	0	86	0		470	16	0	486	754	-
0 2 37 0 39 0 6 5 0 11 0 75 4 0 79 129 0 4 53 0 57 0 5 0 13 0 55 0 0 55 125 22 0 445 0 290 14 0 304 550 0 39 428 0 467 0 97 90 0 187 0 1018 43 0 1061 1715 0 0 8.35 91.65 0.00 0.00 51.87 48.13 0.00 0.00 59.95 4.05 0.00 0.00 2.27 24.96 0.00 27.23 0.00 5.66 52.5 0.00 10.90 0.00 59.36 2.51 0.00 61.87	0	6	43		0	49	0	9	1	5	0	14	0		89	5	0	94	157	550
0 4 53 0 57 0 5 8 0 13 0 55 0 0 55 125 0 17 184 0 201 0 23 22 0 45 0 290 14 0 304 550 0 39 428 0 467 0 97 90 0 187 0 1018 43 0 1061 1715 0 0 8.35 91.65 0.00 0.00 51.87 48.13 0.00 0.00 59.95 4.05 0.00 0 0 2.27 24.96 0.00 27.23 0.00 5.66 5.25 0.00 10.90 0.00 59.36 2.51 0.00 61.87 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 <t< td=""><td>0</td><td>5</td><td>51</td><td></td><td>0</td><td>56</td><td>0</td><td>3</td><td>1</td><td>4</td><td>0</td><td>7</td><td>0</td><td></td><td>71</td><td>5</td><td>0</td><td>76</td><td>139</td><td></td></t<>	0	5	51		0	56	0	3	1	4	0	7	0		71	5	0	76	139	
0 17 184 0 201 0 23 22 0 45 0 290 14 0 304 550 0 39 428 0 467 0 97 90 0 187 0 1018 43 0 1061 1715 0.00 8.35 91.65 0.00 0.00 51.87 48.13 0.00 0.00 95.95 4.05 0.00 0.00 2.27 24.96 0.00 27.23 0.00 5.66 5.25 0.00 10.90 0.00 59.36 2.51 0.00 61.87 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0	2	37		0	39	0	6	1	5	0	11	0		75	4	0	79	129	1
0 39 428	0	4	53		0	57	0	5	1	8	0	13	0		55	0	0	55	125	1
0.00 8.35 91.65 0.00 0.00 51.87 48.13 0.00 0.00 95.95 4.05 0.00 51.87 0.00 2.27 24.96 0.00 27.23 0.00 5.66 5.25 0.00 10.90 0.00 59.36 2.51 0.00 61.87	0	17	184		0	201	0	23	1	22	0	45	0		290	14	0	304	550	1
0.00 8.35 91.65 0.00 0.00 51.87 48.13 0.00 0.00 95.95 4.05 0.00 51.87 0.00 2.27 24.96 0.00 27.23 0.00 5.66 5.25 0.00 10.90 0.00 59.36 2.51 0.00 61.87				_					•											•
0.00	0	39	428		0	467	0	97	1	90	0	187	0	Ì	1018	43	0	1061	1715	1
0.00				_					•											•
0.00																				
	0.00	8.35	91.65		0.00		0.00	51.87	1	48.13	0.00		0.00		95.95	4.05	0.00			
	0.00	2.27	24.96		0.00	27.23	0.00	5.66	1	5.25	0.00	10.90	0.00		59.36	2.51	0.00	61.87		
				_					_											
0 39 420 - 459 0 96 87 - 183 0 1012 42 - 1054	0	0	0		-	0	0	0	1	0	-	0	0		0	0	-	0		
	0	39	420		-	459	0	96	1	87	-	183	0		1012	42	-	1054		
0 0 8 - 8 0 1 3 - 4 0 6 1 - 7	0	0	8		-	8	0	1	1	3	-	4	0		6	1	-	7		
									_		_						_			
0.00 0.00 0.00 - 0.00 0.00 0.00 - 0.00 0.00 - 0.00 0.00 - 0.00	0.00	0.00	0.00		-	0.00	0.00	0.00	1	0.00	-	0.00	0.00		0.00	0.00	-	0.00		
0.00 100.00 98.13 - 98.29 0.00 98.97 96.67 - 97.86 0.00 99.41 97.67 - 99.34	0.00	100.00	98.13		-	98.29	0.00	98.97	1	96.67	-	97.86	0.00		99.41	97.67	-	99.34		
0.00 0.00 1.87 - 1.71 0.00 1.03 3.33 - 2.14 0.00 0.59 2.33 - 0.66	0.00	0.00	1.87		-	1.71	0.00	1.03	1	3.33	-	2.14	0.00		0.59	2.33	-	0.66		

		West	bound					North	bound					East	oound			
	1	Vilkinson	Pike (East	t)				Willow	Oak Trail				١	Vilkinson I	Pike (Wes	t)		
U-Tum	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int
8.1	8.2	8.3		8b	Total	8.4	8.5		8.6	8c	Total	8.7		8.8	8.9	8d	Total	Total
0	6	40		0	46	0	9		14	0	23	0		109	7	0	116	185
0	2	49		0	51	0	9		14	0	23	0		145	3	0	148	222
0	4	50		0	54	0	4		12	0	16	0		135	4	0	139	209
0	6	43		0	49	0	9		5	0	14	0		89	5	0	94	157
			=															
0	18	182		0	200	0	31		45	0	76	0		478	19	0	497	773
			•										,					
0.00	9.00	91.00		0.00		0.00	40.79		59.21	0.00		0.00		96.18	3.82	0.00		
0.00	2.33	23.54		0.00	25.87	0.00	4.01		5.82	0.00	9.83	0.00		61.84	2.46	0.00	64.29	
	93	3%					83	3%					84	1%				87%
0%	75%	91%		=		0%	86%		80%			0%		82%	68%			
0	0	0			0	0	0		0		0	0		0	0		0	0
0	18	178			196	0	31		44	-	75	0		474	19	-	493	764
0	0	4			4	0	0		1	-	1	0		4	0	-	4	9
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	0.00
0.00	100.00	97.80			98.00	0.00	100.00		97.78		98.68	0.00		99.16	100.00		99.20	98.84
0.00	0.00	2.20			2.00	0.00	0.00		2.22	-	1.32	0.00		0.84	0.00	-	0.80	1.16

Site 9 of 11 Local Access Willow Oak Trail Robert Rose Dr

Lat/Long 35.864176°, -86.440431°

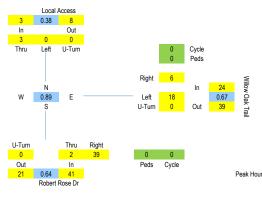
Date Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020)

Classification: ALL



Peak Hour: 0715 - 0815

Int	Rolling
Total	Hour
6	43
7	43
15	55
15	58
43	
6	58
19	68
18	61
15	53
58	-
16	51
12	
10	
13	
51	
	•
152	

			South	bound					West	bound			Northbound					
			Local	Access					Willow (Oak Trail					Robert	Robert Rose Dr		
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	9.1	9.2	9.3		9a	Total	9.4	9.5		9.6	9b	Total	9.7		9.8	9.9	9c	Total
0600 - 0615	0	0	1		0	1	0	1		0	0	1	0	1	0	4	0	4
0615 - 0630	0	0	0		0	0	0	3		1	0	4	0		0	3	0	3
0630 - 0645	0	0	0		0	0	0	8		0	0	8	0		0	7	0	7
0645 - 0700	0	0	0		0	0	0	1		1	0	2	0		1	12	0	13
Hourly Total	0	0	1		0	1	0	13		2	0	15	0		1	26	0	27
0700 - 0715	0	0	0		0	0	0	0		1	0	1	0		0	5	0	5
0715 - 0730	0	0	0		0	0	0	6		3	0	9	0		0	10	0	10
0730 - 0745	0	0	1		0	1	0	1		0	0	1	0		0	16	0	16
0745 - 0800	0	0	0		0	0	0	4		2	0	6	0		0	9	0	9
Hourly Total	0	0	1		0	1	0	11		6	0	17	0		0	40	0	40
0800 - 0815	0	0	2		0	2	0	7		1	0	8	0		2	4	0	6
0815 - 0830	0	0	0		0	0	0	4		4	0	8	0		1	3	0	4
0830 - 0845	0	1	1		0	2	0	2		2	0	4	0	1	0	4	0	4
0845 - 0900	0	0	2		0	2	0	2		1	0	3	0	1	1	7	0	8
Hourly Total	0	1	5		0	6	0	15		8	0	23	0		4	18	0	22
Grand Total	0	1	7		0	8	0	39		16	0	55	0		5	84	0	89
				='										_				
Approach (%)	0.00	12.50	87.50		0.00		0.00	70.91		29.09	0.00		0.00		5.62	94.38	0.00	
Total (%)	0.00	0.66	4.61		0.00	5.26	0.00	25.66		10.53	0.00	36.18	0.00		3.29	55.26	0.00	58.55
				•										-				
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0
Cars	0	1	7		-	8	0	39		15	-	54	0		3	83	-	86
Truck	0	0	0		-	0	0	0		1	-	1	0		2	1	-	3
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00	1	0.00	0.00	-	0.00
Cars (%)	0.00	100.00	100.00		-	100.00	0.00	100.00		93.75	-	98.18	0.00	1	60.00	98.81	-	96.63
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		6.25	-	1.82	0.00		40.00	1.19	-	3.37
				•														

Cycle Peds
0 0

Peak Rolling Hour Flow Rates

Classification: ALL

Ī			South	bound					West	bound					North	hound		Ī
			Local							Oak Trail					Robert			
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	Арр	U-Turn		Thru	Right	Peds	App
TIME	9.1	9.2	9.3		9a	Total	9.4	9.5		9.6	9b	Total	9.7		9.8	9.9	9c	Total
0715 - 0730	0	0	0		0	0	0	6		3	0	9	0		0	10	0	10
0730 - 0745	0	0	1		0	1	0	1		0	0	1	0		0	16	0	16
0745 - 0800	0	0	0		0	0	0	4		2	0	6	0		0	9	0	9
0800 - 0815	0	0	2		0	2	0	7		1	0	8	0		2	4	0	6
Grand Total	0	0	3		0	3	0	18		6	0	24	0		2	39	0	41
																		,
Approach (%)	0.00	0.00	100.00		0.00		0.00	75.00		25.00	0.00		0.00		4.88	95.12	0.00	
Total (%)	0.00	0.00	4.41		0.00	4.41	0.00	26.47		8.82	0.00	35.29	0.00		2.94	57.35	0.00	60.29
PHF		38	3%]			67	%					64	4%			
1111	0%	0%	38%				0%	64%		50%			0%		25%	61%		
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0
Cars	0	0	3		-	3	0	18		6	-	24	0		1	39	-	40
Truck	0	0	0		-	0	0	0		0	-	0	0		1	0	-	1
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	0.00	100.00		-	100.00	0.00	100.00		100.00	-	100.00	0.00		50.00	100.00	-	97.56
Truck (%)	0.00	0.00	0.00	l	-	0.00	0.00	0.00		0.00	-	0.00	0.00		50.00	0.00	-	2.44

Int	
Total	
19	
18	ĺ
15	ĺ
16	ĺ

68

89%
0
67
1

0.00
98.53
1.47

Classified Turn Movement Count

Site 10 of 11 Robert Rose Dr (North)

Grassington St Robert Rose Dr (South)

Lat/Long 35.863444°, -86.440064°

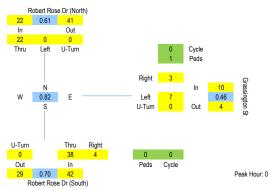
Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020)

Classification: ALL



Peak Hour: 0715 - 0815

			South	bound					West	bound					Northi	bound		
		R	obert Ros	e Dr (Nort	th)				Grassi	ngton St				R	obert Rose	e Dr (Sout	:h)	
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	10.1	10.2	10.3		10a	Total	10.4	10.5		10.6	10b	Total	10.7		10.8	10.9	10c	Total
0600 - 0615	0	0	2		0	2	0	2		1	0	3	0		3	0	0	3
0615 - 0630	0	0	3		0	3	0	3		1	1	5	0		2	1	0	3
0630 - 0645	0	0	7		1	8	0	6		1	1	8	0		6	0	0	6
0645 - 0700	0	0	1		0	1	0	1		0	0	1	0		13	1	0	14
Hourly Total	0	0	13		1	14	0	12		3	2	17	0		24	2	0	26
0700 - 0715	0	0	0		0	0	0	2		1	1	4	0		4	1	0	5
0715 - 0730	0	0	7		0	7	0	1		1	0	2	0		10	0	0	10
0730 - 0745	0	0	2		0	2	0	3		2	1	6	0		14	1	0	15
0745 - 0800	0	0	4		0	4	0	1		0	0	1	0		8	2	0	10
Hourly Total	0	0	13		0	13	0	7		4	2	13	0		36	4	0	40
0800 - 0815	0	0	9		0	9	0	2		0	0	2	0		6	1	0	7
0815 - 0830	0	0	4		0	4	0	2		0	0	2	0		4	0	0	4
0830 - 0845	0	0	3		0	3	0	0		1	0	1	0		3	1	0	4
0845 - 0900	0	1	3		0	4	0	2		2	0	4	0		6	1	0	7
Hourly Total	0	1	19		0	20	0	6		3	0	9	0		19	3	0	22
Grand Total	0	1	45		1	47	0	25		10	4	39	0		79	9	0	88
				=														
				_								_						_
Approach (%)	0.00	2.13	95.74		2.13		0.00	64.10		25.64	10.26		0.00		89.77	10.23	0.00	
Total (%)	0.00	0.57	25.86		0.57	27.01	0.00	14.37		5.75	2.30	22.41	0.00		45.40	5.17	0.00	50.57
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0
Cars	0	1	45		-	46	0	25		10	-	35	0		76	9	-	85
Truck	0	0	0		-	0	0	0		0	-	0	0		3	0	-	3
	I																	
														i				
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	100.00	100.00		-	100.00	0.00	100.00		100.00	-	100.00	0.00		96.20	100.00	-	96.59
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		3.80	0.00	-	3.41

Cycle Peds
0 0

18	51
10	
8	
15	
51	
174	
174	
174	

Int Rolling Total Hour

Peak Rolling Hour Flow Rates Classification: ALL

İ			0	bound					M/ 1	bound					North	d		
			obert Ros	e Dr (Nor					Grassi	ngton St				K	obert Ros	_		
	U-Turn	Left	Thru		Peds	App	U-Tum	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	10.1	10.2	10.3		10a	Total	10.4	10.5		10.6	10b	Total	10.7		10.8	10.9	10c	Total
0715 - 0730	0	0	7		0	7	0	1		1	0	2	0		10	0	0	10
0730 - 0745	0	0	2		0	2	0	3		2	1	6	0		14	1	0	15
0745 - 0800	0	0	4		0	4	0	1		0	0	1	0		8	2	0	10
0800 - 0815	0	0	9		0	9	0	2		0	0	2	0		6	1	0	7
Grand Total	0	0	22		0	22	0	7		3	1	11	0		38	4	0	42
Approach (%)	0.00	0.00	100.00		0.00		0.00	63.64		27.27	9.09		0.00		90.48	9.52	0.00	
Total (%)	0.00	0.00	29.33		0.00	29.33	0.00	9.33		4.00	1.33	14.67	0.00		50.67	5.33	0.00	56.00
PHF		6	1%					46	%					7	0%			
PHF	0%	0%	61%		_		0%	58%		38%			0%		68%	50%		
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0
Cars	0	0	22		-	22	0	7		3	-	10	0		38	4	-	42
Truck	0	0	0		-	0	0	0		0		0	0		0	0		0
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00		0.00
Cars (%)	0.00	0.00	100.00		-	100.00	0.00	100.00		100.00		100.00	0.00		100.00	100.00		100.00
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00

Int
Total
19
23
15
18

75

82%	
0	
74	
0	

0.00
98.67
0.00

Classified Turn Movement Count

Site 11 of 11 Robert Rose Dr (North)

Sculling St Robert Rose Dr (South)

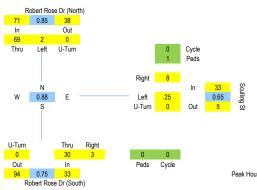
Lat/Long 35.860608°, -86.437829°

Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

0600 - 0900 (Weekday 3h Session) (12-02-2020) Classification: ALL



Peak Hour: 0715 - 0815

	Southbound Robert Rose Dr (North)						Westbound						Northbound					
		R	lobert Ros	e Dr (Nort	th)				Scul	ling St				R	lobert Ros	e Dr (Sout	h)	
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	11.1	11.2	11.3		11a	Total	11.4	11.5		11.6	11b	Total	11.7		11.8	11.9	11c	Total
0600 - 0615	0	0	18		0	18	0	5		0	0	5	0		1	0	0	1
0615 - 0630	0	0	15		0	15	0	2		0	1	3	0		5	3	0	8
0630 - 0645	0	0	21	1	0	21	0	7		0	5	12	0		3	0	0	3
0645 - 0700	0	0	14		0	14	0	8		3	0	11	0		6	3	0	9
Hourly Total	0	0	68		0	68	0	22		3	6	31	0		15	6	0	21
0700 - 0715	0	0	16		0	16	0	4		0	1	5	0		3	4	0	7
0715 - 0730	0	1	20		0	21	0	10		3	0	13	0		3	2	0	5
0730 - 0745	0	0	13		0	13	0	4		3	1	8	0		10	0	0	10
0745 - 0800	0	1	16		0	17	0	3		0	0	3	0		10	1	0	11
Hourly Total	0	2	65		0	67	0	21		6	2	29	0		26	7	0	33
0800 - 0815	0	0	20		0	20	0	8		2	0	10	0		7	0	0	7
0815 - 0830	0	1	13	1	0	14	0	4		1	0	5	0		1	2	0	3
0830 - 0845	0	0	11	1	0	11	0	5		0	0	5	0		4	0	0	4
0845 - 0900	0	0	10	1	0	10	0	2		0	0	2	0		8	2	0	10
Hourly Total	0	1	54		0	55	0	19		3	0	22	0		20	4	0	24
Grand Total	0	3	187	1	0	190	0	62		12	8	82	0		61	17	0	78
				_														
Approach (%)	0.00	1.58	98.42		0.00		0.00	75.61		14.63	9.76		0.00		78.21	21.79	0.00	
Total (%)	0.00	0.86	53.43		0.00	54.29	0.00	17.71		3.43	2.29	23.43	0.00		17.43	4.86	0.00	22.29
																	ii	
P/Cycle	0	0	0	l	-	0	0	0		0	-	0	0		0	0	-	0
Cars	0	2	185	1	-	187	0	62		12	-	74	0		58	16	-	74
Truck	0	1	2		-	3	0	0		0	-	0	0		3	1	-	4
																	ii	
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	66.67	98.93		-	98.42	0.00	100.00		100.00	-	100.00	0.00		95.08	94.12	-	94.87
Truck (%)	0.00	33.33	1.07	l	-	1.58	0.00	0.00		0.00	-	0.00	0.00		4.92	5.88	-	5.13

Cycle Peds
0 0

Peak Rolling Hour Flow Rates

Classification: ALL

ſ			South	bound					Wes	tbound					North	bound		
		R	obert Ros	e Dr (Nort	th)				Scul	ling St				R	obert Ros	e Dr (Sout	h)	
	U-Turn	Left	Thru		Peds	App	U-Tum	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	11.1	11.2	11.3		11a	Total	11.4	11.5		11.6	11b	Total	11.7		11.8	11.9	11c	Total
0715 - 0730	0	1	20		0	21	0	10		3	0	13	0		3	2	0	5
0730 - 0745	0	0	13		0	13	0	4		3	1	8	0		10	0	0	10
0745 - 0800	0	1	16		0	17	0	3		0	0	3	0		10	1	0	11
0800 - 0815	0	0	20		0	20	0	8		2	0	10	0		7	0	0	7
Grand Total	0	2	69		0	71	0	25		8	1	34	0		30	3	0	33
	•							-				:						-
Approach (%)	0.00	2.82	97.18		0.00		0.00	73.53		23.53	2.94	1	0.00		90.91	9.09	0.00	1
					0.00	E4.4E	0.00	18.12			0.72	24.64	0.00		21.74			00.04
Total (%)	0.00	1.45	50.00		0.00	51.45	0.00	18.12	0/	5.80	0.72	24.04	0.00	7	21.74	2.17	0.00	23.91
PHF	00/				l		00/		%	070/			00/	/:		000/		
DIO 1	0%	50%	86%			•	0%	63%		67%		•	0%		75%	38%		
P/Cycle	0	0	0		-	0	0	0		0		0	0		0	0	-	0
Cars	0	2	68		-	70	0	25		8	-	33	0		30	3	-	33
Truck	0	0	1		-	1	0	0		0	-	0	0		0	0	-	0
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	100.00	98.55		-	98.59	0.00	100.00		100.00	-	100.00	0.00		100.00	100.00	-	100.00
Truck (%)	0.00	0.00	1.45		-	1.41	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00

138

88%

Classified Turn Movement Count

Site 1 of 11

Medical Center Pkwy (North) Grashampark Dr (East) Medical Center Pkwy (South) Grashampark Dr (West)

Lat/Long

35.865444 °, -86.447212°

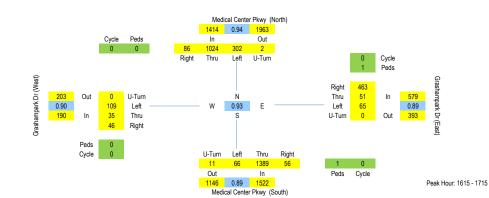
Date

Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL



Southbound Westbound Northbound Eastbound shampark Dr (East) hampark Dr (We U-Turn Right 1.12 Left Thru Right Peds App U-Turn Left Thru Right Peds App U-Turn Left Thru Peds App U-Turn Left Thru Right Peds App Int Rollina 1.16 1b Total 1.9 1.10 1c Total 1.13 1.15 Total Total Hour 1530 - 1545 764 3263 1545 - 1600 798 3447 1600 - 1615 823 3535 Hourly Total 1630 - 1645 1645 - 1700 885 3637 1715 - 1730 Hourly Total 1730 - 1745 1745 - 1800 1800 - 1815 1815 - 1830 Hourly Total 11 812 2825 177 **3827** 0 173 99 1056 31 184 3812 169 1 4197 0 258 84 135 1 478 Grand Total 1 1329 0.29 21.22 73.82 4.63 0.05 0.00 13.02 7.45 79.46 0.08 0.74 4.38 90.83 4.03 0.02 Approach (%) 0.00 53.97 17.57 28.24 0.21 0.32 1.87 38.78 1.72 0.01 42.69 0.00 2.62 0.85 1.37 0.01 4.86 Total (%) 0.11 8.26 28.74 1.80 0.02 38.93 0.00 1.76 1.01 10.74 0.01 13.52 P/Cycle 0 173 99 1049 11 803 2809 3788 168 Cars Truck 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 P/Cycle (%) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 100.00 98.89 99.43 99.44 99.32 0.00 100.00 100.00 99.34 99.47 100.00 99.46 99.37 99.41 99.38 0.00 98.45 100.00 100.00 99.16 Cars (%) Truck (%) 0.00 1.11 0.57 0.56 0.68 0.00 0.00 0.00 0.66 0.53 0.00 0.54 0.63 0.59 0.62 0.00 1.55 0.00 0.00 0.84

			South	nbound					West	bound					North	bound					East	oound			1
		Med	ical Cente	er Pkwy (N	lorth)			G	rashampa	rk Dr (Ea	st)			Medi	ical Cente	r Pkwy (S	outh)			G	rashampa	ırk Dr (We	st)		<u> </u>
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	Int
TIME	1.1	1.2	1.3	1.4	1a	Total	1.5	1.6	1.7	1.8	1b	Total	1.9	1.10	1.11	1.12	1c	Total	1.13	1.14	1.15	1.16	1d	Total	Total
1615 - 1630	0	80	231	16	0	327	0	17	18	128	0	163	6	20	303	13	1	343	0	26	9	14	0	49	882
1630 - 1645	1	71	283	22	0	377	0	17	11	105	0	133	2	12	366	17	0	397	0	25	6	8	0	39	946
1645 - 1700	0	81	233	21	0	335	0	15	11	122	0	148	1	12	334	6	0	353	0	25	8	16	0	49	885
1700 - 1715	1	70	277	27	0	375	0	16	11	108	1	136	2	22	386	20	0	430	0	33	12	8	0	53	994
Grand Total	2	302	1024	86	0	1414	0	65	51	463	1	580	11	66	1389	56	1	1523	0	109	35	46	0	190	3707
																									i
Approach (%)	0.14	21.36	72.42	6.08	0.00		0.00	11.21	8.79	79.83	0.17		0.72	4.33	91.20	3.68	0.07		0.00	57.37	18.42	24.21	0.00		1
Total (%)	0.05	8.15	27.62	2.32	0.00	38.14	0.00	1.75	1.38	12.49	0.03	15.65	0.30	1.78	37.47	1.51	0.03	41.08	0.00	2.94	0.94	1.24	0.00	5.13	1
PHF		94	1%					89	9%					89	9%					90)%				93%
Friir	50%	93%	90%	80%			0%	96%	71%	90%			46%	75%	90%	70%			0%	83%	73%	72%			1
P/Cycle	0	0	0	0		0	0	0	0	0		0	0	0	0	0	-	0	0	0	0	0		0	0
Cars	2	298	1019	85		1404	0	65	51	460		576	11	65	1381	55	-	1512	0	108	35	46		189	3681
Truck	0	4	5	1		10	0	0	0	3		3	0	1	8	1	-	10	0	1	0	0		1	24
P/Cycle (%)	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00
Cars (%)	100.00	98.68	99.51	98.84	-	99.29	0.00	100.00	100.00	99.35	-	99.48	100.00	98.48	99.42	98.21	-	99.34	0.00	99.08	100.00	100.00	-	99.47	99.30
Truck (%)	0.00	1.32	0.49	1.16	-	0.71	0.00	0.00	0.00	0.65	-	0.52	0.00	1.52	0.58	1.79	-	0.66	0.00	0.92	0.00	0.00	-	0.53	0.65

Classified Turn Movement Count

Site 2 of 11

Medical Center Pkwy (North)

Medical Center Pkwy (South)

Willowoak Trail

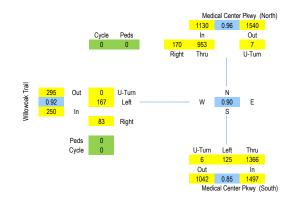
Lat/Long 35.862894°, -86.444927°

Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL



Peak Hour: 1630 - 1730

	U-Turn	Med				
	11 T		icai Centei	r Pkwy (N	lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	2.1		2.2	2.3	2a	Total
1530 - 1545	1		191	34	0	226
1545 - 1600	1		236	34	0	271
1600 - 1615	0		249	27	0	276
1615 - 1630	1		230	43	0	274
Hourly Total	3		906	138	0	1047
1630 - 1645	2		246	45	0	293
1645 - 1700	1		229	40	0	270
1700 - 1715	2		250	41	0	293
1715 - 1730	2		228	44	0	274
Hourly Total	7		953	170	0	1130
1730 - 1745	2		290	43	0	335
1745 - 1800	1		202	36	0	239
1800 - 1815	2		169	22	0	193
1815 - 1830	3		225	26	0	254
Hourly Total	8		886	127	0	1021
Grand Total	18		2745	435	0	3198
Approach (%)	0.56		85.83	13.60	0.00	
Total (%)	0.23		34.53	5.47	0.00	40.23
					-	
P/Cycle	0		0	0	-	0
Cars	18		2729	434	-	3181
Truck	0		16	1	-	17
P/Cycle (%)	0.00		0.00	0.00	-	0.00
Cars (%)	100.00		99.42	99.77	-	99.47
Truck (%)	0.00		0.58	0.23	-	0.53

		North	bound					East	oound				
	Med	ical Cente	Pkwy (S	outh)				Willow	oak Trail				
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	Int	Rolling
2.4	2.5	2.6		2c	Total	2.7	2.8		2.9	2d	Total	Total	Hour
0	25	298		0	323	0	50		25	0	75	624	2621
2	36	284		0	322	0	45		16	0	61	654	2737
0	23	307		0	330	0	41		23	0	64	670	2742
0	22	312		0	334	0	46		19	0	65	673	2871
2	106	1201		0	1309	0	182		83	0	265	2621	-
0	26	358		0	384	0	40		23	0	63	740	2877
0	29	292		0	321	0	54		14	0	68	659	2859
4	46	390		0	440	0	41		25	0	66	799	2825
2	24	326		0	352	0	32		21	0	53	679	2564
6	125	1366		0	1497	0	167		83	0	250	2877	-
0	25	309		0	334	0	35		18	0	53	722	2451
1	18	301		0	320	0	50		16	0	66	625	
0	14	259		0	273	0	54		18	0	72	538	1
3	14	256		0	273	0	23		16	0	39	566	1
4	71	1125		0	1200	0	162		68	0	230	2451	1
			,										
12	302	3692		0	4006	0	511		234	0	745	7949	
0.30	7.54	92.16		0.00	1	0.00	68.59		31.41	0.00	1		
0.15	3.80	46.45		0.00	50.40	0.00	6.43		2.94	0.00	9.37		
0	0	0			0	0	0		0	-	0		
12	298	3669		-	3979	0	508		233	-	741		
0	4	23		-	27	0	3		1	-	4		
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00		
100.00	98.68	99.38		-	99.33	0.00	99.41		99.57	-	99.46		
0.00	1.32	0.62		-	0.67	0.00	0.59		0.43	-	0.54		

0 0 Peds Cycle

	_					
			South	bound		
		Med	ical Cente	r Pkwy (N	lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	2.1		2.2	2.3	2a	Total
1630 - 1645	2		246	45	0	293
1645 - 1700	1		229	40	0	270
1700 - 1715	2		250	41	0	293
1715 - 1730	2		228	44	0	274
Grand Total	7		953	170	0	1130
Approach (%)	0.62		84.34	15.04	0.00	
Total (%)	0.24		33.12	5.91	0.00	39.28
PHF		96	6%			
PHF	88%		95%	94%		
P/Cycle	0		0	0	-	0
Cars	7		950	169	-	1126
Truck	0		3	1	-	4
P/Cycle (%)	0.00		0.00	0.00	-	0.00
Cars (%)	100.00		99.69	99.41	-	99.65
Truck (%)	0.00		0.31	0.59	-	0.35

			ound	Eastb					hound	North		
				Willowo				outh)		cal Center	Medi	
Int	App	Peds	Right		Left	U-Turn	App	Peds	, ,	Thru	Left	U-Tum
Total	Total	2d	2.9		2.8	2.7	Total	2c		2.6	2.5	2.4
740	63	0	23		40	0	384	0		358	26	0
659	68	0	14		54	0	321	0		292	29	0
799	66	0	25		41	0	440	0		390	46	4
679	53	0	21		32	0	352	0		326	24	2
2877	250	0	83		167	0	1497	0		1366	125	6
	1	0.00	33.20		66.80	0.00		0.00		91.25	8.35	0.40
	8.69	0.00	2.88		5.80	0.00	52.03	0.00		47.48	4.34	0.21
90%				2%	92					5%	85	
			83%		77%	0%				88%	68%	38%
0	0	-	0		0	0	0			0	0	0
2863	247	-	82		165	0	1490			1359	125	6
14	3	-	1		2	0	7	-		7	0	0
0.00	0.00	-	0.00		0.00	0.00	0.00			0.00	0.00	0.00
99.51	98.80	-	98.80		98.80	0.00	99.53			99.49	100.00	100.00
0.49	1.20	-	1.20		1.20	0.00	0.47			0.51	0.00	0.00

Site 3 of 11

Medical Center Pkwy (North)

Medical Center Pkwy (South)

Honeylocust Ln

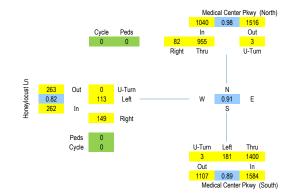
Lat/Long 35.860246°, -86.442494°

Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL



Peak Hour: 1630 - 1730

			South	bound		
		Med	cal Cente		lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	3.1		3.2	3.3	3a	Total
1530 - 1545	0		198	19	0	217
1545 - 1600	2		228	18	0	248
1600 - 1615	2		255	25	0	282
1615 - 1630	1		232	19	0	252
Hourly Total	5		913	81	0	999
1630 - 1645	0		239	18	0	257
1645 - 1700	1		229	28	0	258
1700 - 1715	0		246	20	0	266
1715 - 1730	2		241	16	0	259
Hourly Total	3		955	82	0	1040
1730 - 1745	2		290	17	0	309
1745 - 1800	0		203	18	0	221
1800 - 1815	2		156	9	0	167
1815 - 1830	0		217	6	0	223
Hourly Total	4		866	50	0	920
		,				
Grand Total	12		2734	213	0	2959
Approach (%)	0.41		92.40	7.20	0.00	i
Total (%)	0.15		34.44	2.68	0.00	37.27
P/Cycle	0		0	0	-	0
Cars	12		2720	211	-	2943
Truck	0		14	2	-	16
P/Cycle (%)	0.00		0.00	0.00	-	0.00
Cars (%)	100.00		99.49	99.06	-	99.46
Truck (%)	0.00		0.51	0.94	-	0.54

Peak Rolling Hour Flow Rates	
Classification: ALL	

			0 "			
	-			bound		
		Med	ical Cente	rPkwy (N	lorth)	
	U-Turn		Thru	Right	Peds	App
TIME	3.1		3.2	3.3	3a	Total
1630 - 1645	0		239	18	0	257
1645 - 1700	1		229	28	0	258
1700 - 1715	0		246	20	0	266
1715 - 1730	2		241	16	0	259
Grand Total	3		955	82	0	1040
Approach (%)	0.29		91.83	7.88	0.00	
Total (%)	0.10		33.09	2.84	0.00	36.04
PHF		98	3%			
PHF	38%		97%	73%		
P/Cycle	0		0	0	-	0
Cars	3		951	82	-	1036
Truck	0		4	0	-	4
P/Cycle (%)	0.00		0.00	0.00	-	0.00
Cars (%)	100.00		99.58	100.00	-	99.62
Truck (%)	0.00		0.42	0.00	-	0.38

		Month	bound			ı		Faatl	oound				
	Medi	cal Center		outh)					ocust Ln				
U-Turn	Left	Thru	1	Peds	Арр	U-Turn	Left		Right	Peds	Арр	Int	Rolling
3.4	3.5	3.6		3c	Total	3.7	3.8		3.9	3d	Total	Total	Hour
1	43	299		0	343	0	28		54	0	82	642	2711
1	55	288	1	0	344	0	33		44	0	77	669	2790
1	61	298	1	0	360	0	26		39	0	65	707	2800
1	62	298	1	0	361	0	40		40	0	80	693	2882
4	221	1183	1	0	1408	0	127		177	0	304	2711	-
1	47	359	1	0	407	0	24		33	0	57	721	2886
1	44	320	1	0	365	0	18		38	0	56	679	2876
0	44	399	1	0	443	0	37		43	0	80	789	2802
1	46	322	1	0	369	0	34		35	0	69	697	2510
3	181	1400	1	0	1584	0	113		149	0	262	2886	-
1	37	272	1	0	310	0	57		35	0	92	711	2342
1	44	268	1	0	313	0	37		34	0	71	605	
1	20	238	1	0	259	0	35		36	0	71	497	
0	20	237		0	257	0	24		25	0	49	529	
3	121	1015		0	1139	0	153		130	0	283	2342	
10	523	3598		0	4131	0	393		456	0	849	7939	
											-		
0.24	12.66	87.10	l	0.00		0.00	46.29		53.71	0.00			
0.13	6.59	45.32	j	0.00	52.03	0.00	4.95		5.74	0.00	10.69		
			1		_	_							
0	0	0	l	-	0	0	0		0	-	0		
10	522	3572	ł	-	4104	0	393		451	-	844		
0	1	26	l	-	27	0	0	l	5	-	5		
0.00	0.00	0.00	1		0.00	0.00	0.00	ı	0.00	Ì	0.00		
100.00	99.81	99.28	ł	-	99.35	0.00	100.00		98.90	-	99.41		
0.00	0.19	0.72	l	-	0.65	0.00	0.00		1.10	-	0.59		
0.00	0.13	0.12	ı	-	0.00	0.00	0.00		1.10	1	0.03		

0 0 Peds Cycle

			ound	Eastb					bound	North		
			cust Ln	Honeylo				outh)	r Pkwy (S	cal Center	Medi	
Int	App	Peds	Right		Left	U-Turn	App	Peds		Thru	Left	U-Tum
Total	Total	3d	3.9		3.8	3.7	Total	3c		3.6	3.5	3.4
721	57	0	33		24	0	407	0		359	47	1
679	56	0	38		18	0	365	0		320	44	1
789	80	0	43		37	0	443	0		399	44	0
697	69	0	35		34	0	369	0		322	46	1
									•			
2886	262	0	149		113	0	1584	0		1400	181	3
	1	0.00	56.87		43.13	0.00		0.00		88.38	11.43	0.19
	9.08	0.00	5.16		3.92	0.00	54.89	0.00		48.51	6.27	0.10
91%				%	82					9%	89	
•			87%		76%	0%				88%	96%	75%
0	0	-	0		0	0	0	-		0	0	0
2871	259	-	146		113	0	1576	-		1393	180	3
15	3	-	3		0	0	8	-		7	1	0
0.00	0.00	-	0.00		0.00	0.00	0.00	-		0.00	0.00	0.00
99.48	98.85	-	97.99		100.00	0.00	99.49	-		99.50	99.45	100.00
0.52	1.15	-	2.01		0.00	0.00	0.51	-		0.50	0.55	0.00

Classified Turn Movement Count

Site 4 of 11

Medical Center Pkwy (East) Maplegrove Dr Medical Center Pkwy (West)

Lat/Long 35.858245 °, -86.439332°

Date Wednesday, February 12, 2020

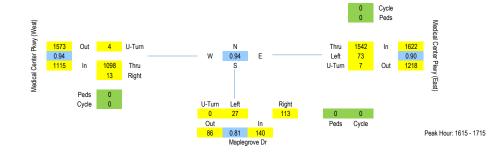
Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL

-	_
TIME	
1530 - 1545	
1545 - 1600	1
1600 - 1615	1
1615 - 1630	
Hourly Total	
1630 - 1645	
1645 - 1700	
1700 - 1715	Ī
1715 - 1730	
Hourly Total	
1730 - 1745	
1745 - 1800	1
1800 - 1815	1
1815 - 1830	1
Hourly Total	
Grand Total	
	1
Approach (%)	
Total (%)	
	ı
P/Cycle	
Cars	
Truck	J
	١
	J
P/Cycle (%)	
Cars (%)	
Truck (%)	ı

TIME]
1615 - 1630]
1630 - 1645	l
1645 - 1700]
1700 - 1715]
]
Grand Total	1
•	1
	I
Approach (%)	1
Total (%)]
PHF	1
1111	l
P/Cycle]
Cars	1
Truck	1
	1
	١
P/Cycle (%)	1
Cars (%)]
Truck (%)	1



			West	tbound					North	bound					East	oound				
		Me	dical Cent	ter Pkwy (East)				Mapleg	grove Dr				Med	lical Cente	er Pkwy (V	Vest)			
	U-Tum	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App	Int	Rolling
	4.1	4.2	4.3		4b	Total	4.4	4.5		4.6	4c	Total	4.7		4.8	4.9	4d	Total	Total	Hour
	1	22	344		0	367	0	5		21	0	26	1		261	4	0	266	659	2717
	1	12	323		0	336	0	2		31	0	33	1		274	7	0	282	651	2769
I	2	25	358		0	385	0	8		16	0	24	1		284	5	0	290	699	2809
	2	19	360		0	381	0	6		24	0	30	1		293	3	0	297	708	2877
	6	78	1385		0	1469	0	21		92	0	113	4		1112	19	0	1135	2717	-
ı	2	19	399		0	420	0	9		19	0	28	1		258	4	0	263	711	2850
ı	1	20	348		0	369	0	5		34	0	39	2		279	2	0	283	691	2841
	2	15	435		0	452	0	7		36	0	43	0		268	4	0	272	767	2765
I	1	20	359		0	380	0	5		19	0	24	0		273	4	0	277	681	2520
ı	6	74	1541		0	1621	0	26		108	0	134	3		1078	14	0	1095	2850	-
I	1	34	303		0	338	0	4		36	0	40	1		318	5	0	324	702	2383
I	3	19	315		0	337	0	8		31	0	39	2		230	7	0	239	615	
ı	1	12	248		0	261	0	12		16	0	28	1		228	4	0	233	522	1
ı	0	15	256		0	271	0	6		19	0	25	0		245	3	0	248	544	1
ı	5	80	1122		0	1207	0	30		102	0	132	4		1021	19	0	1044	2383	1
I																				_
Ī	17	232	4048		0	4297	0	77		302	0	379	11	Ĭ	3211	52	0	3274	7950	i
ſ																				
	0.40	5.40	94.21		0.00		0.00	20.32		79.68	0.00		0.34		98.08	1.59	0.00			
I	0.21	2.92	50.92		0.00	54.05	0.00	0.97		3.80	0.00	4.77	0.14		40.39	0.65	0.00	41.18		
ĺ				_'					_					•'						
	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0		
	17	227	4026		-	4270	0	72		300	-	372	11		3191	51	-	3253		
	0	5	22		-	27	0	5		2	-	7	0		20	1	-	21		
ĺ														•						
ı	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00	Ì	0.00	0.00	-	0.00		
ı	100.00	97.84	99.46		-	99.37	0.00	93.51		99.34	-	98.15	100.00		99.38	98.08	-	99.36		
Ī	0.00	2.16	0.54		-	0.63	0.00	6.49		0.66	-	1.85	0.00		0.62	1.92	-	0.64		

		West	bound					North	bound					East	oound			
	Med	dical Cent	er Pkwy (E	East)				Maple	grove Dr				Med	lical Cente	er Pkwy (V	/est)		
U-Tum	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int
4.1	4.2	4.3		4b	Total	4.4	4.5		4.6	4c	Total	4.7		4.8	4.9	4d	Total	Total
2	19	360		0	381	0	6		24	0	30	1		293	3	0	297	708
2	19	399		0	420	0	9		19	0	28	1		258	4	0	263	711
1	20	348		0	369	0	5		34	0	39	2		279	2	0	283	691
2	15	435		0	452	0	7		36	0	43	0		268	4	0	272	767
7	73	1542		0	1622	0	27		113	0	140	4		1098	13	0	1115	2877
			•															
					_			_			_							
0.43	4.50	95.07		0.00		0.00	19.29		80.71	0.00		0.36		98.48	1.17	0.00		
0.24	2.54	53.60		0.00	56.38	0.00	0.94		3.93	0.00	4.87	0.14		38.16	0.45	0.00	38.76	
	90)%		l			81	1%					94	l%				94%
88%	91%	89%				0%	75%		78%			50%		94%	81%			
0	0	0		-	0	0	0		0		0	0		0	0		0	0
7	72	1533		-	1612	0	26		112		138	4		1089	13		1106	2856
0	1	9		-	10	0	1		1	-	2	0		9	0	-	9	21
								_										
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00		0.00	0.00		0.00	0.00	-	0.00	0.00
100.00	98.63	99.42		-	99.38	0.00	96.30		99.12	-	98.57	100.00		99.18	100.00	-	99.19	99.27
0.00	1.37	0.58		-	0.62	0.00	3.70		0.88	-	1.43	0.00		0.82	0.00	-	0.81	0.73

Classified Turn Movement Count

Site 5 of 11 Robert Rose Dr (North) Medical Center Pkwy (East)

Robert Rose Dr (South) Medical Center Pkwy (West)

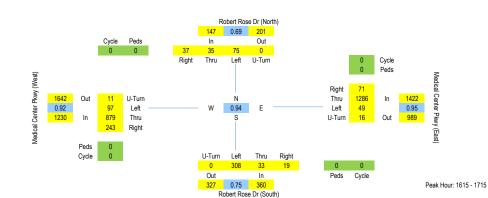
Lat/Long 35.857916°, -86.436850°

Wednesday, February 12, 2020

Weather Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020)

Classification: ALL



Southbound Westbound Northbound Eastbound al Center Pkwy (East) ert Rose Dr (South al Center Pkwy (West) U-Turn Right 5.16 Left Thru Right Peds App U-Turn Left Thru Right Peds App U-Turn Left Thru Right Peds App U-Turn Left Thru Peds App Int Rollina 5.12 5.8 5b 5c Total 5.13 Total Hour Total Total 1530 - 1545 737 2990 1545 - 1600 721 3018 1600 - 1615 Hourly Total 1630 - 1645 1645 - 1700 **158** 1715 - 1730 Hourly Total 1730 - 1745 1745 - 1800 1800 - 1815 1815 - 1830 Hourly Total 0 186 114 145 0 445 46 129 3266 191 0 3632 0 847 135 72 1 **1055** 28 261 2557 685 0 3531 Grand Total 0.00 41.80 25.62 32.58 0.00 0.00 80.28 12.80 6.82 0.09 Approach (%) 1.27 3.55 89.92 5.26 0.00 0.79 7.39 72.42 19.40 0.00 Total (%) 0.00 2.15 1.32 1.67 0.00 5.14 0.53 1.49 37.70 2.20 0.00 41.93 0.00 9.78 1.56 0.83 0.01 12.18 0.32 3.01 29.52 7.91 0.00 40.76 P/Cycle 46 127 3245 186 0 184 113 132 71 2545 676 Cars Truck 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 P/Cycle (%) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 98.92 99.12 100.00 99.33 100.00 98.45 99.36 97.38 99.23 99.29 97.78 98.61 99.05 99.62 99.53 98.69 99.38 Cars (%) Truck (%) 0.00 1.08 0.88 0.00 0.67 0.00 1.55 0.64 2.62 0.77 0.00 0.71 2.22 1.39 0.95 0.00 0.38 0.47 1.31 0.62

Southbound Robert Rose Dr (North)									West	bound			Northbound												
		R	lobert Ros	se Dr (Nort	h)			Med	dical Cente	er Pkwy (E	ast)			R	obert Ros	e Dr (Sout	:h)			Med	lical Cente	er Pkwy (V	Vest)		
	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	U-Turn	Left	Thru	Right	Peds	App	Int
TIME	5.1	5.2	5.3	5.4	5a	Total	5.5	5.6	5.7	5.8	5b	Total	5.9	5.10	5.11	5.12	5c	Total	5.13	5.14	5.15	5.16	5d	Total	Total
1615 - 1630	0	16	6	3	0	25	6	12	318	22	0	358	0	63	6	4	0	73	3	24	253	54	0	334	790
1630 - 1645	0	29	10	14	0	53	2	12	311	18	0	343	0	83	9	6	0	98	4	22	184	61	0	271	765
1645 - 1700	0	17	8	9	0	34	6	10	318	14	0	348	0	57	8	4	0	69	2	29	225	60	0	316	767
1700 - 1715	0	13	11	11	0	35	2	15	339	17	0	373	0	105	10	5	0	120	2	22	217	68	0	309	837
Grand Total	0	75	35	37	0	147	16	49	1286	71	0	1422	0	308	33	19	0	360	11	97	879	243	0	1230	3159
	Approach (%) 0.00 51.02 23.81 25.17 0.00												1												
Approach (%)	0.00	51.02	23.81	25.17	0.00		1.13	3.45	90.44	4.99	0.00		0.00	85.56	9.17	5.28	0.00		0.89	7.89	71.46	19.76	0.00		
Total (%)	0.00	2.37	1.11	1.17	0.00	4.65	0.51	1.55	40.71	2.25	0.00	45.01	0.00	9.75	1.04	0.60	0.00	11.40	0.35	3.07	27.83	7.69	0.00	38.94	
PHF		69	9%					95	5%					75	5%					92	2%				94%
FIII	0%	65%	80%	66%			67%	82%	95%	81%			0%	73%	83%	79%			69%	84%	87%	89%			
P/Cycle	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
Cars	0	75	34	37	-	146	16	48	1278	70	-	1412	0	305	32	19	-	356	11	96	874	238	-	1219	3133
Truck	0	0	1	0	-	1	0	1	8	1	-	10	0	3	1	0	-	4	0	1	5	5	-	11	26
P/Cycle (%)	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	-	0.00	0.00
Cars (%)	0.00	100.00	97.14	100.00	-	99.32	100.00	97.96	99.38	98.59	-	99.30	0.00	99.03	96.97	100.00	-	98.89	100.00	98.97	99.43	97.94	-	99.11	99.18
Truck (%)	0.00	0.00	2.86	0.00	-	0.68	0.00	2.04	0.62	1.41	-	0.70	0.00	0.97	3.03	0.00	-	1.11	0.00	1.03	0.57	2.06	-	0.89	0.82

Site 6 of 11

Wilkinson Pike (East) Greshampark Dr Wilkinson Pike (West)

Lat/Long 35.867615°, -86.446294°

Date Wednesday, February 12, 2020

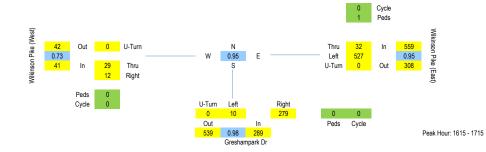
Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL

TIME
1530 - 1545
1545 - 1600
1600 - 1615
1615 - 1630
Hourly Total
1630 - 1645
1645 - 1700
1700 - 1715
1715 - 1730
Hourly Total
1730 - 1745
1745 - 1800
1800 - 1815
1815 - 1830
Hourly Total
Grand Total
Approach (%)
Total (%)
P/Cycle
Cars
Truck
P/Cycle (%)
Cars (%)
Truck (%)

TIME
1615 - 1630
1630 - 1645
1645 - 1700
1700 - 1715
Grand Total
Approach (%)
Total (%)
PHF
1111
P/Cycle
Cars
Truck
P/Cycle (%)
Cars (%)
Truck (%)



			West	tbound					North	bound					East	oound				
		١	Vilkinson	Pike (Eas	t)				Gresha	mpark Dr				١	Wilkinson	Pike (Wes	t)			
U-Tu	ım	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App	Int	Rolling
6.1		6.2	6.3		6b	Total	6.4	6.5		6.6	6c	Total	6.7		6.8	6.9	6d	Total	Total	Hour
0		98	3		0	101	0	0		64	0	64	0	1	2	3	0	5	170	757
0		94	5		0	99	0	0		62	0	62	0	1	4	1	0	5	166	809
0		110	3		0	113	0	1		69	0	70	0	ĺ	3	1	0	4	187	856
0		138	9		0	147	0	0		74	0	74	0		8	5	0	13	234	889
0		440	20		0	460	0	1		269	0	270	0		17	10	0	27	757	-
0		130	6		1	137	0	1		71	0	72	0		11	3	0	14	223	862
0		128	4		0	132	0	4		68	0	72	0		6	3	0	9	213	820
0		131	13		0	144	0	5		66	0	71	0		4	1	0	5	220	746
0		127	7		0	134	0	1		67	0	68	0	ĺ	2	3	0	5	207	603
0		516	30		1	547	0	11		272	0	283	0	i	23	10	0	33	863	-
0		95	3		0	98	0	2		76	0	78	0	ĺ	2	2	0	4	180	503
0		64	3		0	67	0	4		63	0	67	0	ĺ	2	3	0	5	139	
0		49	1	1	0	50	0	0		24	0	24	0	1	3	0	0	3	77	1
0		43	5	1	0	48	0	11		44	0	55	0	1	4	0	0	4	107	1
0		251	12	1	0	263	0	17		207	0	224	0	1	11	5	0	16	503	1
				_										_						
0		1207	62		1	1270	0	29		748	0	777	0	Ì	51	25	0	76	2123	
									='											
0.00	0 9	95.04	4.88	1	0.08	1	0.00	3.73		96.27	0.00		0.00	1	67.11	32.89	0.00			
0.00	0 5	56.85	2.92		0.05	59.82	0.00	1.37		35.23	0.00	36.60	0.00	ĺ	2.40	1.18	0.00	3.58		
				=					_											
0		0	0		-	0	0	0		0	-	0	0]	0	0	-	0		
0		1202	61		-	1263	0	29		744	-	773	0	ĺ	51	25	-	76		
0		5	1		-	6	0	0		4	-	4	0	ĺ	0	0	-	0		
0.00	0	0.00	0.00	1	-	0.00	0.00	0.00		0.00	-	0.00	0.00	1	0.00	0.00	-	0.00		
0.00	0 9	99.59	98.39	1	-	99.53	0.00	100.00		99.47	-	99.49	0.00	1	100.00	100.00	-	100.00		
0.00	0	0.41	1.61	1	-	0.47	0.00	0.00		0.53	-	0.51	0.00	1	0.00	0.00	-	0.00		
				_					-		-			-			•		I	

Westbound					Northbound						Eastbound							
Wilkinson Pike (East)				Greshampark Dr					Wilkinson Pike (West)									
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App	Int
6.1	6.2	6.3		6b	Total	6.4	6.5		6.6	6c	Total	6.7		6.8	6.9	6d	Total	Total
0	138	9	1	0	147	0	0		74	0	74	0		8	5	0	13	234
0	130	6	1	1	137	0	1		71	0	72	0		11	3	0	14	223
0	128	4	1	0	132	0	4		68	0	72	0		6	3	0	9	213
0	131	13	1	0	144	0	5		66	0	71	0		4	1	0	5	220
0	527	32		1	560	0	10		279	0	289	0		29	12	0	41	890
			_					_										
0.00	94.11	5.71		0.18		0.00	3.46		96.54	0.00		0.00		70.73	29.27	0.00		
0.00	59.21	3.60		0.11	62.92	0.00	1.12		31.35	0.00	32.47	0.00		3.26	1.35	0.00	4.61	
	95	5%					98	3%				73%					95%	
0%	95%	62%				0%	50%		94%			0%		66%	60%			
0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	0
0	525	32		-	557	0	10		278	-	288	0		29	12	-	41	886
0	2	0		-	2	0	0		1	-	1	0		0	0	-	0	3
			-															
0.00	0.00	0.00	1	-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	0.00
0.00	99.62	100.00	1	-	99.64	0.00	100.00		99.64	-	99.65	0.00		100.00	100.00	-	100.00	99.55
0.00	0.38	0.00]	-	0.36	0.00	0.00		0.36	-	0.35	0.00		0.00	0.00	-	0.00	0.34

Site 7 of 11 W Park Dr Wilkinson Pike (East)

Wilkinson Pike (West)

Lat/Long 35.867391°, -86.443637°

Date Wednesday, February 12, 2020

Weather Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL

W Park Dr

59 0.87 75
In Out

23 0

Left U-Turn Cycle Peds
0 0 Wilkinson Pike (West) Wilkinson Pike (East) 559 0.92 306 0 U-Turn 50 Left 256 Thru 548 0.95 279 0.94 E U-Tum 0 Out Peds Cycle

Peak Hour: 1615 - 1715

	Southbound						Westbound						
				ark Dr				Wilkinson Pike (East)					
	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App	
TIME	7.1	7.2		7.3	7a	Total	7.4		7.5	7.6	7b	Total	
1530 - 1545	0	4	Ī	7	0	11	0		98	3	0	101	
1545 - 1600	0	3	Ī	3	0	6	0		95	9	0	104	
1600 - 1615	0	4	Ī	4	0	8	0		109	6	0	115	
1615 - 1630	0	5	[11	0	16	0		138	6	0	144	
Hourly Total	0	16		25	0	41	0		440	24	0	464	
1630 - 1645	0	2		9	0	11	0		124	9	0	133	
1645 - 1700	0	9		6	0	15	0		131	3	0	134	
1700 - 1715	0	7	Ī	10	0	17	0		130	7	0	137	
1715 - 1730	0	3	[8	0	11	0		128	6	0	134	
Hourly Total	0	21		33	0	54	0		513	25	0	538	
1730 - 1745	0	3		4	0	7	0		94	6	0	100	
1745 - 1800	0	4		1	0	5	0		62	3	0	65	
1800 - 1815	0	2	Ī	4	0	6	0		46	3	0	49	
1815 - 1830	0	2	Ī	7	0	9	0		39	5	0	44	
Hourly Total	0	11	[16	0	27	0		241	17	0	258	
			-										
Grand Total	0	48		74	0	122	0		1194	66	0	1260	
Approach (%)	0.00	39.34	Ī	60.66	0.00	1	0.00	1	94.76	5.24	0.00		
Total (%)	0.00	2.20		3.40	0.00	5.60	0.00		54.82	3.03	0.00	57.85	
P/Cycle	0	0	l	0	-	0	0		0	0	-	0	
Cars	0	48		74	-	122	0		1188	66	-	1254	
Truck	0	0	l	0	-	0	0		6	0	-	6	
P/Cycle (%)	0.00	0.00	Ī	0.00	-	0.00	0.00	Ì	0.00	0.00	-	0.00	
Cars (%)	0.00	100.00	Ì	100.00	-	100.00	0.00		99.50	100.00	-	99.52	
Truck (%)	0.00	0.00	Ì	0.00	-	0.00	0.00		0.50	0.00	-	0.48	

Peak Rolling Hour Flow Rates	
Classification: ALI	

	Southbound						Westbound					
	W Park Dr						Wilkinson Pike (East)					
	U-Turn	Left		Right	Peds	App	U-Tum		Thru	Right	Peds	App
TIME	7.1	7.2	l	7.3	7a	Total	7.4		7.5	7.6	7b	Total
1615 - 1630	0	5		11	0	16	0		138	6	0	144
1630 - 1645	0	2		9	0	11	0		124	9	0	133
1645 - 1700	0	9		6	0	15	0		131	3	0	134
1700 - 1715	0	7	[10	0	17	0		130	7	0	137
								=				
Grand Total	0	23		36	0	59	0		523	25	0	548
Approach (%)	0.00	38.98	Ī	61.02	0.00		0.00		95.44	4.56	0.00	1
Total (%)	0.00	2.52		3.94	0.00	6.46	0.00		57.28	2.74	0.00	60.02
		87	7%				95%					
PHF	0%	64%		82%			0%		95%	69%		
P/Cycle	0	0	Ì	0	-	0	0		0	0	-	0
Cars	0	23		36	-	59	0		521	25	-	546
Truck	0	0		0	-	0	0		2	0	-	2
			_									
P/Cycle (%)	0.00	0.00	l	0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	100.00	l	100.00	-	100.00	0.00		99.62	100.00	-	99.64
Truck (%)	0.00	0.00	l	0.00	-	0.00	0.00		0.38	0.00	-	0.36

			oound				
	١	Vilkinson I	Pike (Wes	t)			
U-Turn	Left	Thru		Peds	App	Int	Rolling
7.7	7.8	7.9		7d	Total	Total	Hour
0	6	57		0	63	175	793
0	10	61		0	71	181	841
0	9	62		0	71	194	887
0	9	74		0	83	243	913
0	34	254		0	288	793	-
0	10	69		0	79	223	891
0	15	63		0	78	227	849
0	16	50		0	66	220	757
0	11	65		0	76	221	615
0	52	247		0	299	891	-
0	12	62		0	74	181	494
0	10	55		0	65	135	
0	7	16		0	23	78	
0	15	32		0	47	100	
0	44	165		0	209	494	
0	130	666		0	796	2178	
0.00	10.00	00.07	1	0.00	1		
0.00	16.33	83.67		0.00	00.55		
0.00	5.97	30.58		0.00	36.55		
0	0	0		-	0		
0	130	662		-	792		
0	0	4		-	4		
			1				
0.00	0.00	0.00		-	0.00		
0.00	100.00	99.40		-	99.50		
0.00	0.00	0.60		-	0.50		

	Wilkinson Pike (West)										
U-Turn	Left	Thru		Peds	App	Int					
7.7	7.8	7.9		7d	Total	Total					
0	9	74		0	83	243					
0	10	69		0	79	223					
0	15	63		0	78	227					
0	16	50		0	66	220					
0	50	256		0	306	913					
			=								
0.00	16.34	83.66		0.00							
0.00	5.48	28.04		0.00	33.52						
	92	.%				94%					
0%	78%	86%									
0	0	0		-	0	0					
0	50	255		-	305	910					
0	0	1		-	1	3					
0.00	0.00	0.00		-	0.00	0.00					
0.00	100.00	99.61		-	99.67	99.67					
0.00	0.00	0.39		-	0.33	0.33					

Murfreesboro, TN Classified Turn Movement Count

Site 8 of 11

Wilkinson Pike (East) Willow Oak Trail Wilkinson Pike (West)

Lat/Long 35.866080°, -86.438822°

Date Wednesday, February 12, 2020

Weather

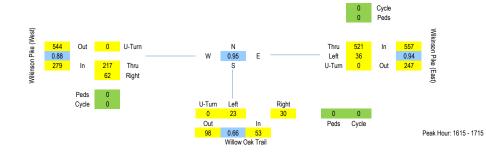
Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL

TIME	ı
1530 - 1545	ı
1545 - 1600	ı
1600 - 1615	ı
1615 - 1630	ı
Hourly Total	ı
1630 - 1645	ı
1645 - 1700	ı
1700 - 1715	ı
1715 - 1730	ı
Hourly Total	
1730 - 1745	
1745 - 1800	ı
1800 - 1815	ı
1815 - 1830	ı
Hourly Total	ı
	ı
Grand Total	ı
	ı
	ı
Approach (%)	ı
Total (%)	ı
	ı
	ı
P/Cycle	ı
Cars	ı
Truck	ı
	ı
P/Cycle (%)	ı
Cars (%)	
Truck (%)	ı

Peak Rolling Hour Flow Rates Classification: ALL

TIME
1615 - 1630
1630 - 1645
1645 - 1700
1700 - 1715
Grand Total
Approach (%)
Total (%)
PHF
110
P/Cycle
Cars
Truck
P/Cycle (%)
Cars (%)
Truck (%)



		West	bound					North	bound					East	oound			l	
	1	Wilkinson	Pike (East	t)				Willow	Oak Trail				1	Wilkinson	Pike (Wes	t)			
U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int	Rolling
8.1	8.2	8.3		8b	Total	8.4	8.5		8.6	8c	Total	8.7		8.8	8.9	8d	Total	Total	Hour
0	8	101		0	109	0	3		9	0	12	0		56	8	0	64	185	783
0	8	94		0	102	0	5		6	0	11	0		59	4	0	63	176	815
0	4	112		0	116	0	5	1	7	0	12	0		51	9	0	60	188	869
0	6	137		0	143	0	6		6	0	12	0		63	16	0	79	234	889
0	26	444		0	470	0	19		28	0	47	0		229	37	0	266	783	-
0	6	121		0	127	0	10		10	0	20	0		54	16	0	70	217	875
0	9	130		0	139	0	5		9	0	14	0		58	19	0	77	230	854
0	15	133		0	148	0	2	1	5	0	7	0		42	11	0	53	208	759
0	15	128		0	143	0	5	1	5	0	10	0		54	13	0	67	220	635
0	45	512		0	557	0	22	1	29	0	51	0		208	59	0	267	875	-
0	13	98		0	111	0	2	1	17	0	19	0		58	8	0	66	196	513
0	10	60		0	70	0	5	1	4	0	9	0		50	6	0	56	135	
0	7	43		0	50	0	4	1	8	0	12	0		14	8	0	22	84	
0	8	43		0	51	0	6	1	7	0	13	0		24	10	0	34	98	
0	38	244		0	282	0	17	1	36	0	53	0		146	32	0	178	513	
			-					•											•
0	109	1200		0	1309	0	58	1	93	0	151	0	Ï	583	128	0	711	2171	
0.00	8.33	91.67	1	0.00		0.00	38.41	1	61.59	0.00		0.00	Ì	82.00	18.00	0.00	1		
0.00	5.02	55.27		0.00	60.29	0.00	2.67	1	4.28	0.00	6.96	0.00		26.85	5.90	0.00	32.75		
		•													•	•	,		
0	0	0	1	-	0	0	0	1	0	-	0	0	Ì	0	0] -	0		
0	106	1194	1	-	1300	0	58	1	93	-	151	0		579	128	-	707		
0	3	6	1	-	9	0	0	1	0	-	0	0		4	0	-	4		
			1																
0.00	0.00	0.00	1	-	0.00	0.00	0.00	1	0.00	۱ .	0.00	0.00	ì	0.00	0.00	1 -	0.00		
0.00	97.25	99.50	1	-	99.31	0.00	100.00	1	100.00	-	100.00	0.00		99.31	100.00	-	99.44		
0.00	2.75	0.50	1	-	0.69	0.00	0.00	1	0.00	-	0.00	0.00		0.69	0.00	-	0.56		
																		ı	

		West	bound					North	bound									
	١	Vilkinson	Pike (East	t)				Willow	Oak Trail				V	Vilkinson I	Pike (Wes	t)		
U-Tum	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	Int
8.1	8.2	8.3		8b	Total	8.4	8.5		8.6	8c	Total	8.7		8.8	8.9	8d	Total	Total
0	6	137		0	143	0	6		6	0	12	0		63	16	0	79	234
0	6	121		0	127	0	10		10	0	20	0		54	16	0	70	217
0	9	130		0	139	0	5		9	0	14	0		58	19	0	77	230
0	15	133		0	148	0	2		5	0	7	0		42	11	0	53	208
0	36	521		0	557	0	23		30	0	53	0	ľ	217	62	0	279	889
0.00	6.46	93.54		0.00		0.00	43.40		56.60	0.00		0.00		77.78	22.22	0.00		
0.00	4.05	58.61		0.00	62.65	0.00	2.59		3.37	0.00	5.96	0.00		24.41	6.97	0.00	31.38	
	94	1%					66	6%					88	1%				95%
0%	60%	95%				0%	58%		75%			0%		86%	82%			,
0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	0
0	35	518		-	553	0	23		30	-	53	0		216	62	-	278	884
0	1	3		-	4	0	0		0	-	0	0		1	0	-	1	5
0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00	Ì	0.00	0.00	-	0.00	0.00
0.00	97.22	99.42		-	99.28	0.00	100.00		100.00	-	100.00	0.00		99.54	100.00	-	99.64	99.44
0.00	2.78	0.58		-	0.72	0.00	0.00		0.00	-	0.00	0.00		0.46	0.00	-	0.36	0.56

Murfreesboro, TN

Classified Turn Movement Count

Site 9 of 11 Local Access Willow Oak Trail Robert Rose Dr

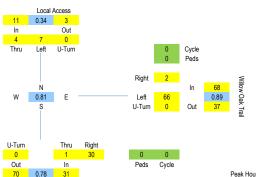
Lat/Long 35.864176°, -86.440431°

Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020) Classification: ALL



Out In 70 0.78 31 Robert Rose Dr Peak Hour: 1630 - 1730

			South	bound					West	bound			Northbound						
	Local Access								Willow	Oak Trail					Robert I	Rose Dr			
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	
TIME	9.1	9.2	9.3		9a	Total	9.4	9.5		9.6	9b	Total	9.7		9.8	9.9	9c	Total	
1530 - 1545	0	2	3		0	5	0	8		2	0	10	0		0	5	0	5	
1545 - 1600	0	2	1		0	3	0	5		1	0	6	0		1	6	0	7	
1600 - 1615	0	2	1		0	3	0	9		0	0	9	0		2	7	0	9	
1615 - 1630	0	0	0	1	0	0	0	12		0	0	12	0		0	10	0	10	
Hourly Total	0	6	5		0	11	0	34		3	0	37	0		3	28	0	31	
1630 - 1645	0	5	3		0	8	0	15		1	0	16	0		0	10	0	10	
1645 - 1700	0	1	0		0	1	0	16		1	0	17	0		0	8	0	8	
1700 - 1715	0	1	0		0	1	0	16		0	0	16	0		0	4	0	4	
1715 - 1730	0	0	1		0	1	0	19		0	0	19	0		1	8	0	9	
Hourly Total	0	7	4		0	11	0	66		2	0	68	0		1	30	0	31	
1730 - 1745	0	1	0	1	0	1	0	15		0	0	15	0		0	14	0	14	
1745 - 1800	0	0	1		0	1	0	7		1	0	8	0		0	8	0	8	
1800 - 1815	0	1	0		0	1	0	7		1	0	8	0		0	8	0	8	
1815 - 1830	0	0	0		0	0	0	9		0	0	9	0		0	10	0	10	
Hourly Total	0	2	1		0	3	0	38		2	0	40	0		0	40	0	40	
Grand Total	0	15	10		0	25	0	138		7	0	145	0		4	98	0	102	
Approach (%)	0.00	60.00	40.00	1	0.00	1	0.00	95.17		4.83	0.00		0.00		3.92	96.08	0.00		
Total (%)	0.00	5.51	3.68	1	0.00	9.19	0.00	50.74		2.57	0.00	53.31	0.00		1.47	36.03	0.00	37.50	
				•															
P/Cycle	0	0	0	l	-	0	0	0		0	-	0	0		0	0	-	0	
Cars	0	15	10		-	25	0	138		7	-	145	0		4	98	-	102	
Truck	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	
P/Cycle (%)	0.00	0.00	0.00]	-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	
Cars (%)	0.00	100.00	100.00		-	100.00	0.00	100.00		100.00	-	100.00	0.00		100.00	100.00	-	100.00	
Truck (%)	0.00	0.00	0.00	j	-	0.00	0.00	0.00		0.00	-	0.00	0.00	l	0.00	0.00	-	0.00	

Cycle Peds
0 0

Int Rolling Total Hour

Peak Rolling Hour Flow Rates

Classification: ALL

ſ			South	bound					West	tbound					North	bound		
Ì			Local	Access					Willow	Oak Trail					Robert	Rose Dr		
	U-Turn	Left	Thru		Peds	App	U-Tum	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App
TIME	9.1	9.2	9.3		9a	Total	9.4	9.5		9.6	9b	Total	9.7		9.8	9.9	9c	Total
1630 - 1645	0	5	3		0	8	0	15		1	0	16	0		0	10	0	10
1645 - 1700	0	1	0		0	1	0	16		1	0	17	0		0	8	0	8
1700 - 1715	0	1	0		0	1	0	16		0	0	16	0		0	4	0	4
1715 - 1730	0	0	1		0	1	0	19		0	0	19	0		1	8	0	9
Grand Total	0	7	4		0	11	0	66		2	0	68	0		1	30	0	31
										-		•				-		-
Approach (%)	0.00	63.64	36.36		0.00		0.00	97.06		2.94	0.00]	0.00		3.23	96.77	0.00]
Total (%)	0.00	6.36	3.64		0.00	10.00	0.00	60.00		1.82	0.00	61.82	0.00		0.91	27.27	0.00	28.18
PHF		34	1%					89	%					78	8%			
FNF	0%	35%	33%				0%	87%		50%			0%		25%	75%		
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0		0
Cars	0	7	4		-	11	0	66		2	-	68	0		1	30		31
Truck	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0
P/Cycle (%)	0.00	0.00	0.00	l	-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	0.00	100.00	100.00		-	100.00	0.00	100.00		100.00	-	100.00	0.00		100.00	100.00	-	100.00
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00

110

81%

Murfreesboro, TN

Classified Turn Movement Count

Site 10 of 11 Robert Rose Dr (North)

Grassington St Robert Rose Dr (South)

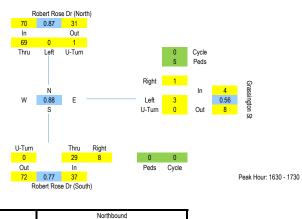
Lat/Long 35.863444°, -86.440064°

Wednesday, February 12, 2020

Weather Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020)

Classification: ALL



			South	bound					West	bound			Northbound						
		R	obert Ros	e Dr (Nort	h)				Grassi	ngton St				R	obert Ros	e Dr (Sout	:h)		
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	
TIME	10.1	10.2	10.3		10a	Total	10.4	10.5		10.6	10b	Total	10.7		10.8	10.9	10c	Total	
1530 - 1545	0	1	10		0	11	0	1		0	2	3	0		5	4	0	9	
1545 - 1600	0	0	5		0	5	0	0		1	0	1	0		6	0	0	6	
1600 - 1615	0	0	11		0	11	0	1		0	0	1	0		9	2	0	11	
1615 - 1630	0	0	12		0	12	0	0		0	0	0	0		10	0	0	10	
Hourly Total	0	1	38		0	39	0	2		1	2	5	0		30	6	0	36	
1630 - 1645	0	0	18		0	18	0	3		1	0	4	0		9	3	0	12	
1645 - 1700	1	0	15		2	18	0	0		0	3	3	0		7	1	0	8	
1700 - 1715	0	0	16		0	16	0	0		0	0	0	0		4	2	0	6	
1715 - 1730	0	0	20		1	21	0	0		0	2	2	0		9	2	0	11	
Hourly Total	1	0	69		3	73	0	3		1	5	9	0		29	8	0	37	
1730 - 1745	0	0	15		1	16	0	2		1	3	6	1		13	1	0	15	
1745 - 1800	0	0	8		0	8	0	3		0	0	3	0		8	2	0	10	
1800 - 1815	0	0	7		0	7	0	1		0	0	1	0		8	4	0	12	
1815 - 1830	0	0	10		0	10	0	0		0	0	0	0		11	2	0	13	
Hourly Total	0	0	40		1	41	0	6		1	3	10	1		40	9	0	50	
														ii.					
Grand Total	1	1	147		4	153	0	11		3	10	24	1		99	23	0	123	
														ii.					
Approach (%)	0.65	0.65	96.08		2.61		0.00	45.83		12.50	41.67		0.81		80.49	18.70	0.00		
Total (%)	0.33	0.33	49.00		1.33	51.00	0.00	3.67		1.00	3.33	8.00	0.33		33.00	7.67	0.00	41.00	
											i			iı					
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	
Cars	1	1	147		-	149	0	11		3	-	14	1		99	23	-	123	
Truck	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	
				ı							1			1			ı		
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	
Cars (%)	100.00	100.00	100.00		-	100.00	0.00	100.00		100.00	-	100.00	100.00		100.00	100.00	-	100.00	
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	

Cycle Peds 0 3

Peak Rolling Hour Flow Rates Classification: ALL

			Caudh	bound					Meet	bound			Northbound						
			obert Ros		ir)					naton St					lobert Rose		L-\		
	U-Turn	Left		e Dr (Nori		A	U-Tum	1.0	Glassi		Peds	A	U-Turn	, r				A	
TIME	10.1	Leπ 10.2	Thru 10.3		Peds 10a	App Total	10.4	Left 10.5		Right 10.6	10b	App Total	10.7		Thru 10.8	Right 10.9	Peds 10c	App Total	
										10.0		Total							
1630 - 1645	0	0	18		0	18	0	3		1	0	4	0		9	3	0	12	
1645 - 1700	1	0	15		2	18	0	0		0	3	3	0		7	1	0	8	
1700 - 1715	0	0	16		0	16	0	0		0	0	0	0		4	2	0	6	
1715 - 1730	0	0	20		1	21	0	0		0	2	2	0		9	2	0	11	
Grand Total	1	0	69		3	73	0	3		1	5	9	0		29	8	0	37	
Approach (%)	1.37	0.00	94.52		4.11		0.00	33.33		11.11	55.56	1	0.00		78.38	21.62	0.00		
Total (%)	0.84	0.00	57.98		2.52	61.34	0.00	2.52		0.84	4.20	7.56	0.00		24.37	6.72	0.00	31.09	
BUE		87	7%					56	%					7	7%				
PHF	25%	0%	86%				0%	25%		25%			0%		81%	67%			
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	
Cars	1	0	69		-	70	0	3		1	-	4	0		29	8	-	37	
Truck	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	
Cars (%)	100.00	0.00	100.00		-	100.00	0.00	100.00		100.00	-	100.00	0.00		100.00	100.00	-	100.00	
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00		0.00	0.00		0.00	0.00	-	0.00	

Int	Rolling
Total	Hour
23	78
12	91
23	103
22	102
80	•
34	111
29	110
22	107
34	105
119	•
37	97
21	
20	
23	
101	
101	ı

300

Int
Total
34
29
22
34

119

88%
0
111
0

Murfreesboro, TN

Classified Turn Movement Count

Site 11 of 11 Robert Rose Dr (North)

Sculling St Robert Rose Dr (South)

Lat/Long 35.860608°, -86.437829°

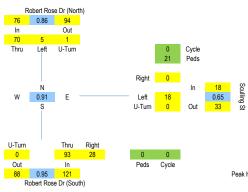
Date Wednesday, February 12, 2020

Weather

Light Rain Shower 47°F

1530 - 1830 (Weekday 3h Session) (12-02-2020)

Classification: ALL



Peak Hour: 1715 - 1815

			South	bound					West	bound			Northbound						
		R	obert Ros	e Dr (Nort	th)				Scul	ling St				R	lobert Ros	Dr (Sout	:h)		
	U-Turn	Left	Thru		Peds	App	U-Tum	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	App	
TIME	11.1	11.2	11.3		11a	Total	11.4	11.5		11.6	11b	Total	11.7		11.8	11.9	11c	Total	
1530 - 1545	0	2	18		0	20	0	3		0	1	4	0		18	2	0	20	
1545 - 1600	0	1	13		0	14	0	1		0	0	1	0		13	3	0	16	
1600 - 1615	0	3	16	1	0	19	0	3		0	0	3	0		25	1	0	26	
1615 - 1630	0	2	10		0	12	0	6		0	5	11	0		20	11	0	31	
Hourly Total	0	8	57		0	65	0	13		0	6	19	0		76	17	0	93	
1630 - 1645	0	3	20		0	23	0	5		0	0	5	0		21	3	0	24	
1645 - 1700	0	4	16		0	20	0	2		0	0	2	0		14	5	0	19	
1700 - 1715	0	5	17	1	0	22	0	3		0	4	7	0		21	4	0	25	
1715 - 1730	0	2	20		0	22	0	7		0	5	12	0		23	6	0	29	
Hourly Total	0	14	73		0	87	0	17		0	9	26	0		79	18	0	97	
1730 - 1745	1	1	19		0	21	0	2		0	13	15	0		23	6	0	29	
1745 - 1800	0	1	18		0	19	0	3		0	3	6	0		22	10	0	32	
1800 - 1815	0	1	13		0	14	0	6		0	0	6	0		25	6	0	31	
1815 - 1830	0	2	12	1	0	14	0	6		2	0	8	0		18	9	0	27	
Hourly Total	1	5	62		0	68	0	17		2	16	35	0		88	31	0	119	
				-															
Grand Total	1	27	192		0	220	0	47		2	31	80	0		243	66	0	309	
Approach (%)	0.45	12.27	87.27	1	0.00		0.00	58.75		2.50	38.75		0.00		78.64	21.36	0.00		
Total (%)	0.16	4.43	31.53	1	0.00	36.12	0.00	7.72		0.33	5.09	13.14	0.00		39.90	10.84	0.00	50.74	
P/Cycle	0	0	0		-	0	0	0		0	-	0	0		0	0	-	0	
Cars	1	26	192		-	219	0	47		2	-	49	0		240	66	-	306	
Truck	0	1	0		-	1	0	0		0	-	0	0		3	0	-	3	
																	-		
P/Cycle (%)	0.00	0.00	0.00	1	-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00	
Cars (%)	100.00	96.30	100.00]	-	99.55	0.00	100.00		100.00	-	100.00	0.00		98.77	100.00	-	99.03	
Truck (%)	0.00	3.70	0.00]	-	0.45	0.00	0.00		0.00	-	0.00	0.00		1.23	0.00	-	0.97	

Cycle Peds
0 0

Peak Rolling Hour Flow Rates Classification: ALL

			South	bound					West	bound					North	bound		
		R	obert Ros	e Dr (Nor	th)				Scul	ling St				R	obert Ros	e Dr (Sout	:h)	
	U-Turn	Left	Thru		Peds	App	U-Turn	Left		Right	Peds	App	U-Turn		Thru	Right	Peds	Арр
TIME	11.1	11.2	11.3		11a	Total	11.4	11.5		11.6	11b	Total	11.7		11.8	11.9	11c	Total
1715 - 1730	0	2	20		0	22	0	7		0	5	12	0		23	6	0	29
1730 - 1745	1	1	19		0	21	0	2		0	13	15	0		23	6	0	29
1745 - 1800	0	1	18		0	19	0	3		0	3	6	0		22	10	0	32
1800 - 1815	0	1	13		0	14	0	6		0	0	6	0		25	6	0	31
				-														
Grand Total	1	5	70		0	76	0	18		0	21	39	0		93	28	0	121
				=							=	•				-	=	-
Approach (%)	1.32	6.58	92.11	1	0.00		0.00	46.15		0.00	53.85	1	0.00		76.86	23.14	0.00	1
Total (%)	0.42	2.12	29.66		0.00	32.20	0.00	7.63		0.00	8.90	16.53	0.00		39.41	11.86	0.00	51.27
` '	0.42	86			0.00	02.20	0.00	7.00	%	0.00	0.50	10.00	0.00	Q.	5%	11.00	0.00	01.21
PHF	25%	63%	88%		1		0%	64%	70	0%			0%		93%	70%		
P/Cvcle	0	007/0	0070		_	0	0	0470		0		0	0		0	0		0
Cars	1	5	70		_	76	0	18		0		18	0		91	28		119
Truck	0	0	0			0	0	0		0		0	0		2	0		2
Truck		U	·	ı		0	-	Ū		Ū		U				·		
P/Cycle (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		0.00	0.00	-	0.00
Cars (%)	100.00	100.00	100.00		-	100.00	0.00	100.00		0.00	-	100.00	0.00		97.85	100.00	-	98.35
Truck (%)	0.00	0.00	0.00		-	0.00	0.00	0.00		0.00	-	0.00	0.00		2.15	0.00	-	1.65

Int	Rolling
Total	Hour
44	171
31	180
48	190
54	192
177	•
52	201
41	201
54	214
63	215
210	•
65	206
57	
51	
49	
222	

609

236

91%

APPENDIX B TRIP GENERATION WORKSHEETS

TRIP GENERA	TION SUM	IMARY - Prop	osed Clari	Park N	laster	Plan			
Trip Generation N	lanual, 10ti	h Edition, Instit	tute of Trans	portati	on Eng	gineers			
Land Use	ITE Code	Total Units	Daily Trips	A.M	. Peak I	lour	P.M	. Peak l	lour
Land Ose	TTE Code	Total Offics	Daily Trips	Enter	Exit	Total	Enter	Exit	Total
Single Family	210	36 homes	406	7	23	30	24	14	38
Townhomes (Low-Rise)	220 186 units		1,622	23	78	101	75	44	119
Apartments (Mid-Rise)	221	280 units	1,524	24	70	94	73	46	119
Hotel	310	240 Rooms	2,283	68	47	115	79	75	154
Bowling Alley	437	49,000 sf	-	38	2	40	35	19	54
General Office	710	100,000 sf	1,061	103	17	120	18	96	114
Medical Office	720	15,000 sf	489	32	9	41	15	38	53
Convenience Market w/Gas Pumps	853	4,700 sf	2,934	95	96	191	116	116	232
Convenience Market W/Gas Fumps	000	(Pass-By Trip %)	60%	63%	63%	63%	66%	66%	66%
Furniture Store	890	10,000 sf	98	2	1	3	3	3	6
Drive-in Bank	912	14,000 sf	1,277	77	56	133	143	143	286
Fast Casual	930	11,667 GSF	3,677	14	10	24	91	74	165
Quality Restaurant	931	11,667 GSF	978	5	4	9	54	37	91
High-Turnover (Sit-Down) Restaurant	932	11,666 GSF	1,309	64	52	116	71	43	114
Unadjusted Total Trip G	eneration		17,658	552	465	1,017	797	748	1,545
Internal Trip Reduct	ion %		15%		15%			20%	
Internal Trips		2,649	83	70	153	159	150	309	
Adjusted Total Trip Ge		15,009	469	395	864	638	598	1236	
	Pass-By Trips (from Convenience Market w/Gas Pumps) (Trip Generation x Pass-By Trip % x Internal Trip Reduction %)					102	61	61	122
Adjusted Total Primary Tri	Adjusted Total Primary Trip Generation					762	577	537	1,114

Single-Family Detached Housing - 36 Dwelling Units

Use ITE Land Use Code 210 (Single-Family Detached Housing) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

$$Ln(T) = 0.92 Ln(X) + 2.71$$

 $Ln(T) = 0.92 Ln(36) + 2.71$
 $T = 406$

A.M. Peak Hour of Adjacent Street Traffic

$$T = 0.71(X) + 4.80$$

 $T = 0.71(36) + 4.80$
 $T = 30$

Enter =
$$0.25(30) = 8$$

Exit = $0.75(30) = 22$

P.M. Peak Hour of Adjacent Street Traffic

$$Ln(T) = 0.96 Ln(X) + 0.20$$

 $Ln(T) = 0.96 Ln(36) + 0.20$
 $T = 38$

Enter =
$$0.63(38) = 24$$

Exit = $0.37(38) = 14$

Multifamily (Low-Rise) - 186 Dwelling Units

Use ITE Land Use Code 220 (Multifamily (Low-Rise)) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 7.56(X) - 40.86 T = 7.56(186) - 40.86T = 1365

A.M. Peak Hour of Adjacent Street Traffic

Ln(T) = 0.95 Ln(X) - 0.51 Ln(T) = 0.95 Ln(186) - 0.51 T = 86

> Enter = 0.23(86) = 20Exit = 0.77(86) = 66

P.M. Peak Hour of Adjacent Street Traffic

Ln(T) = 0.89 Ln(X) - 0.02 Ln(T) = 0.89 Ln(186) - 0.02T = 103

> Enter = 0.63(103) = 65Exit = 0.37(103) = 38

Multifamily (Mid-Rise) - 280 Dwelling Units

Use ITE Land Use Code 221 (Multifamily (Mid-Rise)) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 5.45(X) - 1.75 T = 5.45(280) - 1.75T = 1524

A.M. Peak Hour of Adjacent Street Traffic

Ln(T) = 0.98 Ln(X) - 0.98 Ln(T) = 0.98 Ln(280) - 0.98T = 94

> Enter = 0.26(94) = 24Exit = 0.74(94) = 70

P.M. Peak Hour of Adjacent Street Traffic

Ln(T) = 0.96 Ln(X) - 0.63 Ln(T) = 0.96 Ln(280) - 0.63T = 119

> Enter = 0.61(119) = 73Exit = 0.39(119) = 46

Hotel - 240 Rooms

Use ITE Land Use Code 310 (Hotel) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 11.29(X) - 426.97 T = 11.29(240) - 426.97T = 2283

A.M. Peak Hour of Adjacent Street Traffic

T = 0.50(X) - 5.34 T = 0.50(240) - 5.34 T = 115Enter = 0.59(115) = 68

Enter = 0.59(115) = 68Exit = 0.41(115) = 47

P.M. Peak Hour of Adjacent Street Traffic

T = 0.75(X) - 26.02 T = 0.75(240) - 26.02T = 154

> Enter = 0.51(154) = 79Exit = 0.49(154) = 75

Bowling Alley - 49,000 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 437 (Bowling Alley) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

No Data Provided

A.M. Peak Hour of Adjacent Street Traffic

T = 0.81(X) T = 0.81(49) T = 40Enter = 0.95(40) = 38 Exit = 0.05(40) = 2

P.M. Peak Hour of Adjacent Street Traffic

T = 1.01(X) + 4.92 T = 1.01(49) + 4.92 T = 54 Enter = 0.65(54) = 35 Exit = 0.35(54) = 19

General Office Building - 100,000 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 710 (General Office Building) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

$$Ln(T) = 0.97 Ln(X) + 2.50$$

 $Ln(T) = 0.97 Ln(100) + 2.50$
 $T = 1061$

A.M. Peak Hour

$$T = 0.94 (X) + 26.49$$

 $T = 0.94 (100) + 26.49$
 $T = 120$

Enter =
$$0.86(120) = 103$$

Exit = $0.14(120) = 17$

P.M. Peak Hour

$$Ln(T) = 0.95 Ln(X) + 0.36$$

 $Ln(T) = 0.95 Ln(100) + 0.36$
 $T = 114$

Enter =
$$0.16(114) = 18$$

Exit = $0.84(114) = 96$

Medical-Dental Office Building - 15,000 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 720 (Medical Office Building) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 38.42(X) - 87.62 T = 38.42(15) - 87.62T = 489

A.M. Peak Hour of Adjacent Street Traffic

Ln(T) = 0.89 Ln(X) + 1.31 Ln(T) = 0.89 Ln(15) + 1.31 T = 41

> Enter = 0.78(41) = 32Exit = 0.22(41) = 9

P.M. Peak Hour of Adjacent Street Traffic

T = 3.39(X) + 2.02 T = 3.39(15) + 2.02T = 53

> Enter = 0.28(53) = 15Exit = 0.72(53) = 38

TRIP GENERATION

Convenience Market with Gasoline Pumps - 4,700 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 853 (Convenience Market with Gas Pumps) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 624.20(X) T = 624.20(4.7)T = 2934

A.M. Peak Hour of Adjacent Street Traffic

T = 40.59(X) T = 40.59(4.7) T = 191 Enter = 0.50(191) = 95 Exit = 0.50(191) = 96

P.M. Peak Hour of Adjacent Street Traffic

T = 49.29(X) T = 49.29(4.7) T = 232

> Enter = 0.50(191) = 116 Exit = 0.50(191) = 116

TRIP GENERATION

Furniture Store - 10,000 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 890 (Furniture Store) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

$$T = 5.17(X) + 46.56$$

 $T = 5.17(10) + 46.56$
 $T = 98$

A.M. Peak Hour of Adjacent Street Traffic

$$T = 0.24(X) + 0.94$$

 $T = 0.24(10) + 0.94$
 $T = 3$

Enter =
$$0.71(3) = 2$$

Exit = $0.29(3) = 1$

P.M. Peak Hour of Adjacent Street Traffic

$$Ln(T) = 0.85 Ln(X) - 0.18$$

 $Ln(T) = 0.85 Ln(10) - 0.18$
 $T = 6$

Enter =
$$0.47(6) = 3$$

Exit = $0.53(6) = 3$

Drive-in Bank - 14,000 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 912 (Drive-in Bank) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 82.87(X) + 117.10 T = 82.87(14) + 117.10T = 1277

A.M. Peak Hour of Adjacent Street Traffic

T = 9.50(X) T = 9.50(14)T = 133

> Enter = 0.58(133) = 77Exit = 0.42(133) = 56

P.M. Peak Hour of Adjacent Street Traffic

T = 20.45(X) T = 20.45(14)T = 286

> Enter = 0.50(286) = 143 Exit = 0.50(286) = 143

Fast Casual Restaurant - 11,667 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 931 (Quality Restaurant) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 315.17(X)

T = 315.17(11.667)

T = 3677

A.M. Peak Hour of Adjacent Street Traffic

T = 2.07(X)

T = 2.07(11.667)

T = 24

Enter = 0.67(165) = 14

Exit = 0.33(165) = 10

P.M. Peak Hour of Adjacent Street Traffic

T = 14.13(X)

T = 14.13(11.667)

T = 165

Enter = 0.55(165) = 91

Exit = 0.45(165) = 74

Quality Restaurant - 11,667 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 931 (Quality Restaurant) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 83.84(X)T = 83.84(11.667)

T = 978

A.M. Peak Hour of Adjacent Street Traffic

T = 0.73(X) T = 0.73(11.667)T = 9

> Directional Distribution Not Available

P.M. Peak Hour of Adjacent Street Traffic

T = 7.80(X) T = 7.80(11.667)T = 91

> Enter = 0.67(91) = 54Exit = 0.33(91) = 37

High-Turnover (Sit-Down) Restaurant - 11,666 Sq. Feet Gross Floor Area (X = GSF/1000)

Use ITE Land Use Code 932 (High-Turnover (Sit-Down) Restaurant) and associated trip generation rates for 24-hour total trips and peak hour trips.

Average Daily Traffic

T = 112.18(X) T = 112.18(11.666)T = 1309

A.M. Peak Hour of Adjacent Street Traffic

T = 9.94(X) T = 9.94(11.666) T = 116Enter = 0.55(116) = 64 Exit = 0.45(116) = 52

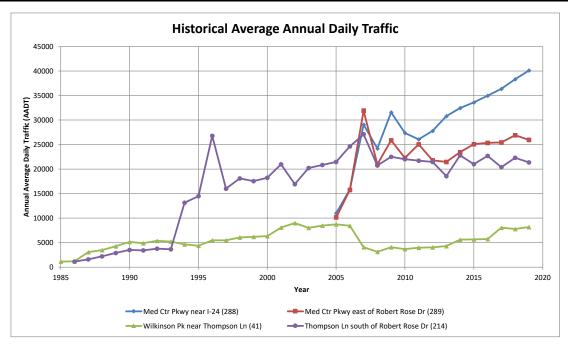
P.M. Peak Hour of Adjacent Street Traffic

T = 9.77(X) T = 9.77(11.666) T = 114

> Enter = 0.62(114) = 71Exit = 0.38(114) = 43

APPENDIX C TRAFFIC ASSIGNMENT WORKSHEETS

	HISTORI	CAL TRAFFIC COUNT DA	ATA	
Year	Med Ctr Pkwy near I-24 (288)	Med Ctr Pkwy east of Robert	Wilkinson Pk near Thompson	Thompson Ln south of
Teal	Med Ctr Pkwy near i-24 (288)	Rose Dr (289)	Ln (41)	Robert Rose Dr (214)
1985			1153	
1986			1200	1124
1987			3058	1574
1988			3469	2215
1989			4272	2884
1990			5135	3495
1991			4888	3412
1992			5375	3770
1993			5200	3650
1994			4659	13126
1995			4348	14464
1996			5459	26788
1997			5457	16008
1998			6043	18096
1999			6197	17545
2000			6302	18213
2001			8069	20975
2002			8990	16933
2003			8011	20223
2004			8461	20830
2005	11022	10157	8715	21455
2006	15809	15731	8431	24639
2007	29059	31927	4064	27097
2008	24194	20958	3122	20740
2009	31564	25870	4056	22515
2010	27378	22277	3663	22011
2011	26072	25051	3968	21702
2012	27780	21789	4039	21448
2013	30803	21466	4293	18560
2014	32443	23455	5601	22789
2015	33598	25081	5657	20985
2016	34983	25331	5713	22687
2017	36380	25426	8056	20390
2018	38334	26902	7787	22306
2019	40120	25950	8170	21360



		Med Ctr Pkwy near I-24 (288)	Med Ctr Pkwy east of Robert Rose Dr (289)	Wilkinson Pk near Thompson Ln (41)	Thompson Ln south of Robert Rose Dr (214)
Analysis	Begin	2011	2012	2007	1994
Period	End	2019	2019	2019	2019
Futui	re Year	2030	2030	2030	2030
Forecasted [*]	Traffic Volume	58777	35209	12256	25197
Annual G	rowth Rate	3.53%	2.81% 3.76%		1.51%
Growt	h Factor	1.465	1.357	1.500	1.180

TRAFFIC VOLUME WORKSHEET SPECIFIC NON-SITE TRIP GENERATION & PROPOSED DEVELOPMENT TRIP GENERATION



PROPOSED SITE DEVELOPMENT TRIP GENERATION												
Dovelopment	Deily	A.N	/I. Peak H	our	P.M. Peak Hour							
Development	Daily	Enter	Exit	Total	Enter	Exit	Total					
Clari Park Primary Trips	13,513	418	344	762	577	537	1,114					
Clari Park Pass-By Trips	1,496	51	51	102	61	61	122					
TOTAL	15,009	469	395	864	638	598	1,236					

PROPOSED WILLOWOAK REASSIGNMENT											
Description	AADT	A.M. P	eak Hour	(9.3%)	P.M. Peak Hour (10.6%)						
Description	AADI	EB	WB	Total	EB	WB	Total				
Willowoak Reassignment	3,500	234	91	325	152	219	371				
TOTAL	3,500	234	91	325	152	219	371				

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT GRESHAMPARK DRIVE A.M. PEAK HOUR



Description		Northboun al Center Thru			Southboun al Center Thru			Eastbound eshampar Thru		Westbound Greshampark Dr Left Thru Right		
2020 EXISTING TRAFFIC VOLUMES	32	488	22	522	1211	22	18	19	57	33	19	165
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor Annual Background Growth Trips	3.0 1.23 7	3.0 1.23 112	3.0 1.23 5	3.0 1.23 120	3.0 1.23 278	3.0 1.23 5	3.0 1.23 4	3.0 1.23 4	3.0 1.23 13	3.0 1.23 8	3.0 1.23 4	3.0 1.23 38
2027 Background Traffic Volumes		600	27	642	1489	27	22	23	70	41	23	203
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	0	32 110	7 29	8 33	32 134	0	0	0	0	7 24	0	8 28
2027 Site Traffic Volumes	0	110	29	33	134	0	0	0	0	24	0	28
Willowoak Dr Extension Adjustment % Willowoak Dr Extension Adjustment Trips	-	69	4% -9	93% -218	218	_	-	3% -7		15% -14	9% -8	76% -69
2027 TOTAL TRAFFIC VOLUMES	39	779	47	457	1841	27	22	16	70	51	15	162

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT GRESHAMPARK DRIVE P.M. PEAK HOUR



	ı	Northboun	d	Southbound				Eastbound	d	Westbound		
Description	Medic	al Center	Pkwy	Medic	al Center	Pkwy	Gre	shampar	k Dr	Gre	shampar	k Dr
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	77	1389	56	304	1024	86	109	35	46	65	51	463
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Growth Factor Annual Background Growth Trips	1.23 18	1.23 319	1.23 13	1.23 70	1.23 235	1.23 20	1.23 25	1.23 8	1.23 11	1.23 15	1.23 12	1.23 106
2027 Background Traffic Volumes	95	1708	69	374	1259	106	134	43	57	80	63	569
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out		32	7	8	32					7		0
Clari Park Primary Trips % Out Trips	0	32 172	40	46	185	0	0	0	0	7 38	0	8 43
2027 Site Traffic Volumes	0	172	40	46	185	0	0	0	0	38	0	43
Willowoak Dr Extension Adjustment % Willowoak Dr Extension Adjustment Trips		175	14% -21	77% -117	117			9% -14		11% -24	9% -20	80% -175
2027 TOTAL TRAFFIC VOLUMES	95	2055	88	303	1561	106	134	29	57	94	43	437

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT WILLOWOAK TRAIL A.M. PEAK HOUR



A.M. PEAK HOUR	-		1(1.1		Southbound				F	_	Westbound		
Book and out and			Northboun		_		-		Eastboun		vvestbound		
Description			al Center			al Center			llowoak 1		1.6	T 1	D'ala
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES		33	484			1267	24	20		9			
2020 EXISTING TRAIT IC VOLUMES		55	404			1207	24	20		9			
2027 BACKGROUND TRAFFIC VOLUMES													
Annual Background Growth													
Growth Rate (%/year)		3.0	3.0			3.0	3.0	3.0		3.0			
Growth Factor		1.23	1.23	1.00	1.00	1.23	1.23	1.23	1.00	1.23	1.00	1.00	1.00
Annual Background Growth	n Trips	8	111	0	0	291	6	5	0	2	0	0	0
2027 Background Traffic Vo	olumes	41	595	0	0	1558	30	25	0	11	0	0	0
2027 SITE TRAFFIC VOLUMES													
	% In		6	7	18	14		1	1				
Clari Park Primary Trips	% Out		14			6	1				7	1	18
	Trips	0	73	29	75	79	3	4	4	0	24	3	62
	% In		-45	45		-55	55						
Clari Park Pass-By Trips	% Out		-43	45		-33	55				55		45
, ,	Trips	0	-23	23	0	-28	28	0	0	0	28	0	23
									-			-	
2027 Site Traffic Vo	olumes	0	50	52	75	51	31	4	4	0	52	3	85
William of De Foto of a Advanced Trian			•		040				-		۱	•	00
Willowoak Dr Extension Adjustment Trips			-9	9	218	-14			7		14	8	69
2027 TOTAL TRAFFIC VOLUMES		41	636	61	293	1595	61	29	11	11	66	11	154
		• •	- 50	٠.	_30	. 200	٥.		• •			• •	,

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT WILLOWOAK TRAIL P.M. PEAK HOUR



P.M. PEAK HOUR													
		Northbour	d	Southbound				Eastboun		Westbound			
Description	Medic	cal Center	Pkwy	Medic	al Center	Pkwy	Wi	llowoak 1	Γrail				
·	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
2020 EXISTING TRAFFIC VOLUMES	131	1366			953	170	167		83				
2027 BACKGROUND TRAFFIC VOLUMES												,	
Annual Background Growth													
Growth Rate (%/year)	3.0	3.0			3.0	3.0	3.0		3.0				
Growth Factor	1.23	1.23	1.00	1.00	1.23	1.23	1.23	1.00	1.23	1.00	1.00	1.00	
Annual Background Growth Trips	30	314	0	0	219	39	38	0	19	0	0	0	
2027 Background Traffic Volumes	161	1680	0	0	1172	209	205	0	102	0	0	0	
2027 SITE TRAFFIC VOLUMES													
% In		6	7	18	14		1	1					
Clari Park Primary Trips % Out		14			6	1				7	1	18	
Trips	0	110	40	104	113	5	6	6	0	38	5	97	
0/1													
% In Clari Park Pass-By Trips % Out		-45	45		-55	55						45	
Clari Park Pass-By Trips % Out Trips	0	-27	27	0	-34	34	0	0	0	55 34	0	45 27	
2027 Site Traffic Volumes	0	83	67	104	79	39	6	6	0	72	5	124	
Willowoak Dr Extension Adjustment Trips		-21	21	117	-24			14		24	20	175	
2027 TOTAL TRAFFIC VOLUMES	161	1742	88	221	1227	248	211	20	102	96	25	299	

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT HONEYLOCUST LANE A.M. PEAK HOUR



Description		Northbound Medical Center Pkwy			Southbound Medical Center Pkwy			Eastboun eylocust		Westbound		
·	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	34	498			1208	49	10		5			
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0	3.0			3.0	3.0	3.0		3.0			
Growth Factor	1.23	1.23	1.00	1.00	1.23	1.23	1.23	1.00	1.23	1.00	1.00	1.00
Annual Background Growth Trips	8	114	0	0	278	11	2	0	1	0	0	0
2027 Background Traffic Volumes	42	612	0	0	1486	60	12	0	6	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In		12	8	14			1	1				
Clari Park Primary Trips % Out					12	1				8	1	14
Trips	0	50	33	59	41	3	4	4	0	28	3	48
2027 Site Traffic Volumes	0	50	33	59	41	3	4	4	0	28	3	48
2027 TOTAL TRAFFIC VOLUMES	42	662	33	59	1527	63	16	4	6	28	3	48

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT HONEYLOCUST LANE P.M. PEAK HOUR



Description		Northboun	-		Southboun			Eastboune eylocust		Westbound		
·	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	184	1400			955	82	113		149			
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0	3.0			3.0	3.0	3.0		3.0			
Growth Factor	1.23	1.23	1.00	1.00	1.23	1.23	1.23	1.00	1.23	1.00	1.00	1.00
Annual Background Growth Trips	42	322	0	0	220	19	26	0	34	0	0	0
2027 Background Traffic Volumes	226	1722	0	0	1175	101	139	0	183	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In		12	8	14			1	1				
Clari Park Primary Trips % Out					12	1				8	1	14
Trips	0	69	46	81	64	5	6	6	0	43	5	75
2027 Site Traffic Volumes	0	69	46	81	64	5	6	6	0	43	5	75
2027 TOTAL TRAFFIC VOLUMES	226	1791	46	81	1239	106	145	6	183	43	5	75

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT MAPLEGROVE DRIVE A.M. PEAK HOUR



Description		Northboun	-	S	Southbour	nd		Eastbound		Westbound Medical Center Pkwy		
•	Left .	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	0		9					1199	11	32	530	
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0		3.0					3.0	3.0	3.0	3.0	
Growth Factor	1.23	1.00	1.23	1.00	1.00	1.00	1.00	1.23	1.23	1.23	1.23	1.00
Annual Background Growth Trips	0	0	2	0	0	0	0	276	3	7	122	0
2027 Background Traffic Volumes	0	0	11	0	0	0	0	1475	14	39	652	0
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out	1							19	1		19	
Trips	4	0	0	0	0	0	0	65	3	0	79	0
2027 Site Traffic Volumes	4	0	0	0	0	0	0	65	3	0	79	0
2027 TOTAL TRAFFIC VOLUMES	4	0	11	0	0	0	0	1540	17	39	731	0

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT MAPLEGROVE DRIVE P.M. PEAK HOUR



Description	Мар	Northboun	Drive		Southbour		Medic	Eastbound al Center	Pkwy	Westbound Medical Center Pkwy		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	27		113					1098	13	80	1542	
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0		3.0					3.0	3.0	3.0	3.0	
Growth Factor	1.23	1.00	1.23	1.00	1.00	1.00	1.00	1.23	1.23	1.23	1.23	1.00
Annual Background Growth Trips	6	0	26	0	0	0	0	252	3	18	354	0
2027 Background Traffic Volumes	33	0	139	0	0	0	0	1350	16	98	1896	0
2027 SITE TRAFFIC VOLUMES												
% In	1										19	
Clari Park Primary Trips % Out								19	1			
Trips	6	0	0	0	0	0	0	102	5	0	110	0
2027 Site Traffic Volumes	6	0	0	0	0	0	0	102	5	0	110	0
2027 TOTAL TRAFFIC VOLUMES	39	0	139	0	0	0	0	1452	21	98	2006	0

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT ROBERT ROSE DRIVE A.M. PEAK HOUR



Description		Northboun bert Rose	Dr	Ro	Southbour bert Rose	Dr	Medic	Eastbound	Pkwy	Westbound Medical Center Pkwy		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	94	13	30	47	21	49	31	1053	125	32	424	11
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Growth Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Annual Background Growth Trips	22	3	7	11	5	11	7	242	29	7	97	3
2027 Background Traffic Volumes	116	16	37	58	26	60	38	1295	154	39	521	14
2027 SITE TRAFFIC VOLUMES												
% In	4	6									15	15
Clari Park Primary Trips % Out				15	6			15	4			
Trips	17	25	0	52	21	0	0	52	14	0	63	63
2027 Site Traffic Volumes	17	25	0	52	21	0	0	52	14	0	63	63
2027 TOTAL TRAFFIC VOLUMES	133	41	37	110	47	60	38	1347	168	39	584	77

TRAFFIC VOLUME WORKSHEET MEDICAL CENTER PARKWAY AT ROBERT ROSE DRIVE P.M. PEAK HOUR



Description		Northboun bert Rose	-		Southbour bert Rose			Eastbound		Westbound Medical Center Pkwy		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	308	33	19	75	35	37	108	879	243	65	1286	71
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Growth Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Annual Background Growth Trips	71	8	4	17	8	9	25	202	56	15	296	16
2027 Background Traffic Volumes	379	41	23	92	43	46	133	1081	299	80	1582	87
2027 SITE TRAFFIC VOLUMES												
% In	4	6									15	15
Clari Park Primary Trips % Out				15	6			15	4			
Trips	23	35	0	81	32	0	0	81	21	0	87	87
2027 Site Traffic Volumes	23	35	0	81	32	0	0	81	21	0	87	87
2027 TOTAL TRAFFIC VOLUMES	402	76	23	173	75	46	133	1162	320	80	1669	174

TRAFFIC VOLUME WORKSHEET WILKINSON PIKE AT GRESHAMPARK DRIVE A.M. PEAK HOUR



A.M. PEAK HOUR		1	Northboun	d	5	Southbour	ıd		Eastbound	d	Westbound		
De	scription	Gre	shampar	k Dr				w	ilkinson	Pk	w	ilkinson	Pk
	•	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC	CVOLUMES	9		462					27	3	213	24	
2027 BACKGROUND TR	AFFIC VOLUMES												
Annual Background													
Growth Rate (%/yea	ar)	3.0		3.0					3.0	3.0	3.0	3.0	
Growth Factor		1.23	1.00	1.23	1.00	1.00	1.00	1.00	1.23	1.23	1.23	1.23	1.00
	Annual Background Growth Trips	2	0	106	0	0	0	0	6	1	49	6	0
2	027 Background Traffic Volumes	11	0	568	0	0	0	0	33	4	262	30	0
2027 SITE TRAFFIC VOI	LUMES												
Clari Park	% In x Primary Trips % Out Trips	0	0	2 2 15	0	0	0	0	0	0	2 2 15	0	0
	2027 Site Traffic Volumes	0	0	15	0	0	0	0	0	0	15	0	0
Willowoak Dr Exten	sion Adjustment Trips			-234		_	_		_	_	-91		
2027 TOTAL TRAFFIC V	OLUMES	11	0	349	0	0	0	0	33	4	186	30	0

TRAFFIC VOLUME WORKSHEET WILKINSON PIKE AT GRESHAMPARK DRIVE P.M. PEAK HOUR



P.M. PEAK HOUR	1	Northbour		5	Southbour	ıd		Eastbound	-	Westbound		
Description	Gre	eshampar	k Dr				W	ilkinson	Pk	W	ilkinson	Pk
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	10		279					29	12	527	32	
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0		3.0					3.0	3.0	3.0	3.0	
Growth Factor	1.23	1.00	1.23	1.00	1.00	1.00	1.00	1.23	1.23	1.23	1.23	1.00
Annual Background Growth Trips	3 2	0	64	0	0	0	0	7	3	121	7	0
2027 Background Traffic Volume	12	0	343	0	0	0	0	36	15	648	39	0
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Ou Trips	0	0	2 2 22	0	0	0	0	0	0	2 2 22	0	0
2027 Site Traffic Volume	s 0	0	22	0	0	0	0	0	0	22	0	0
Willowoak Dr Extension Adjustment Trips			-152							-219		
2027 TOTAL TRAFFIC VOLUMES	12	0	213	0	0	0	0	36	15	451	39	0

TRAFFIC VOLUME WORKSHEET WILKINSON PIKE AT WEST PARK DRIVE / CLARI PARK ACCESS #2 A.M. PEAK HOUR



Description	Clari	Northboun Park Acc	ess #2	w	Southbour est Park	Dr	W	Eastboun i lkinson	Pk	Westbound Wilkinson Pk		
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES				21		31	17	472			204	12
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)				3.0		3.0	3.0	3.0			3.0	3.0
Growth Factor	1.00	1.00	1.00	1.23	1.00	1.23	1.23	1.23	1.00	1.00	1.23	1.23
Annual Background Growth Trips	0	0	0	5	0	7	4	109	0	0	47	3
2027 Background Traffic Volumes	0	0	0	26	0	38	21	581	0	0	251	15
2027 SITE TRAFFIC VOLUMES												
% In									2	1	2	
Clari Park Primary Trips % Out	2		1					2				
Trips	7	0	3	0	0	0	0	7	8	4	8	0
2027 Site Traffic Volumes	7	0	3	0	0	0	0	7	8	4	8	0
Willowoak Dr Extension Adjustment Trips								-234			-91	
2027 TOTAL TRAFFIC VOLUMES	7	0	3	26	0	38	21	354	8	4	168	15

TRAFFIC VOLUME WORKSHEET WILKINSON PIKE AT WEST PARK DRIVE / CLARI PARK ACCESS #2 P.M. PEAK HOUR



P.M. PEAK HOUR	hd		Southbour	nd		Eastboun		Westbound				
Description		Northboun			est Park			ilkinson			ilkinson	
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES				23		36	50	256			523	25
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)				3.0		3.0	3.0	3.0			3.0	3.0
Growth Factor	1.00	1.00	1.00	1.23	1.00	1.23	1.23	1.23	1.00	1.00	1.23	1.23
Annual Background Growth Trips	0	0	0	5	0	8	11	59	0	0	120	6
2027 Background Traffic Volumes	0	0	0	28	0	44	61	315	0	0	643	31
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out	2		1					2	2	1	2	
Trips	11	0	5	0	0	0	0	11	12	6	12	0
2027 Site Traffic Volumes	11	0	5	0	0	0	0	11	12	6	12	0
Willowoak Dr Extension Adjustment Trips								-152			-219	
2027 TOTAL TRAFFIC VOLUMES	11	0	5	28	0	44	61	174	12	6	436	31

TRAFFIC VOLUME WORKSHEET WILKINSON PIKE AT WILLOWOAK TRAIL A.M. PEAK HOUR



Description	Wil	Northboun I Iowoak T	rail		Southbour		w	Eastbound /ilkinson	Pk	w	Westboun Iilkinson	Pk
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	31		45					478	19	18	182	
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)	3.0		3.0					3.0	3.0	3.0	3.0	
Growth Factor	1.23	1.00	1.23	1.00	1.00	1.00	1.00	1.23	1.23	1.23	1.23	1.00
Annual Background Growth Trips	7	0	10	0	0	0	0	110	4	4	42	0
2027 Background Traffic Volumes	38	0	55	0	0	0	0	588	23	22	224	0
2027 SITE TRAFFIC VOLUMES												
% In										12	3	
Clari Park Primary Trips % Out			12					3				
Trips	0	0	41	0	0	0	0	10	0	50	13	0
2027 Site Traffic Volumes	0	0	41	0	0	0	0	10	0	50	13	0
Willowoak Dr Extension Adjustment % Willowoak Dr Extension Adjustment Trips	-26		220					94% -220	-15	65	71% -65	
2027 TOTAL TRAFFIC VOLUMES	12	0	316	0	0	0	0	378	8	137	172	0

TRAFFIC VOLUME WORKSHEET WILKINSON PIKE AT WILLOWOAK TRAIL P.M. PEAK HOUR



Description	Wil	lowoak T	rail		Southbour		W	Eastbound /ilkinson	Pk	w	Westboun	Pk
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES	23		30					217	62	36	521	
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor	3.0 1.23 5	1.00	3.0 1.23 7	1.00	1.00	1.00	1.00	3.0 1.23 50	3.0 1.23 14	3.0 1.23 8	3.0 1.23 120	1.00
Annual Background Growth Trips 2027 Background Traffic Volumes		0	37	0	0	0	0	267	76	44	641	0
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	0	0	12 64	0	0	0	0	3 16	0	12 69	3 17	0
2027 Site Traffic Volumes	0	0	64	0	0	0	0	16	0	69	17	0
Willowoak Dr Extension Adjustment % Willowoak Dr Extension Adjustment Trips	-20		105					69% -105	-47	199	91% -199	
2027 TOTAL TRAFFIC VOLUMES	8	0	206	0	0	0	0	178	29	312	459	0

TRAFFIC VOLUME WORKSHEET WILLOWOAK TRAIL AT ROBERT ROSE DRIVE A.M. PEAK HOUR



Description		Northboun bert Rose Thru	-		Southboun rivate Dri Thru			Eastbound Ilowoak T Thru			Westboun I lowoak T Thru	
2020 EXISTING TRAFFIC VOLUMES		2	39	0	3					18		6
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor Annual Background Growth Trips	1.00	3.0 1.23 0	3.0 1.23 9	3.0 1.23 0	3.0 1.23 1	1.00 0	1.00	1.00 0	1.00 0	3.0 1.23 4	1.00 0	3.0 1.23 1
2027 Background Traffic Volumes	0	2	48	0	4	0	0	0	0	22	0	7
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	16 1 70	0	2 7	0	0	0	0	10 34	1 16 59	2 8	10 42	0
2027 Site Traffic Volumes	70	0	7	0	0	0	0	34	59	8	42	0
Willowoak Dr Extension Adjustment % Willowoak Dr Extension Adjustment Trips	14		15% -14					225	9	4% -9	77	
2027 TOTAL TRAFFIC VOLUMES	84	2	41	0	4	0	0	259	68	21	119	7

TRAFFIC VOLUME WORKSHEET WILLOWOAK TRAIL AT ROBERT ROSE DRIVE P.M. PEAK HOUR



Description	Ro	Northboun	e Dr	P	Southboun	ve	Wi	Eastbound	rail	Wi	Vestboun	rail
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES		1	30	7	4					66		2
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)		3.0	3.0	3.0	3.0					3.0		3.0
Growth Factor	1.00	1.23	1.23	1.23	1.23	1.00	1.00	1.00	1.00	1.23	1.00	1.23
Annual Background Growth Trips	0	0	7	2	1	0	0	0	0	15	0	0
2027 Background Traffic Volumes	0	1	37	9	5	0	0	0	0	81	0	2
2027 SITE TRAFFIC VOLUMES												
% In	16								1	2	10	
Clari Park Primary Trips % Out	1		2					10	16			
Trips	98	0	11	0	0	0	0	54	92	12	58	0
2027 Site Traffic Volumes	98	0	11	0	0	0	0	54	92	12	58	0
Willowoak Dr Extension Adjustment % Willowoak Dr Extension Adjustment Trips	11		5% -11					120	32	21% -32	208	
2027 TOTAL TRAFFIC VOLUMES	109	1	37	9	5	0	0	174	124	61	266	2

TRAFFIC VOLUME WORKSHEET ROBERT ROSE DRIVE AT GRASSINGTON STREET/CLARI PARK ACCESS #7 A.M. PEAK HOUR



Description		Northboun	-	_	Southbour			Eastboune Park Acc		,	Westboun	d
·	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES		38	4	0	22							
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)		3.0	3.0	3.0	3.0							
Growth Factor	1.00	1.23	1.23	1.23	1.23	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Annual Background Growth Trips	0	9	1	0	5	0	0	0	0	0	0	0
2027 Background Traffic Volumes	0	47	5	0	27	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In	1	16			2	1						
Clari Park Primary Trips % Out		2			16		1		1			
Trips	4	74	0	0	63	4	3	0	3	0	0	0
2027 Site Traffic Volumes	4	74	0	0	63	4	3	0	3	0	0	0
2027 TOTAL TRAFFIC VOLUMES	4	121	5	0	90	4	3	0	3	0	0	0

TRAFFIC VOLUME WORKSHEET ROBERT ROSE DRIVE AT GRASSINGTON STREET/CLARI PARK ACCESS #7 P.M. PEAK HOUR



Description		Northboun		_	Southboun			Eastboun		\	Vestboun	d
•	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES		29	8	1	69							
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)		3.0	3.0	3.0	3.0							
Growth Factor	1.00	1.23	1.23	1.23	1.23	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Annual Background Growth Trips	0	7	2	0	16	0	0	0	0	0	0	0
2027 Background Traffic Volumes	0	36	10	1	85	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In	1	16			2	1						
Clari Park Primary Trips % Out		2			16		1		1			
Trips	6	103	0	0	97	6	5	0	5	0	0	0
2027 Site Traffic Volumes	6	103	0	0	97	6	5	0	5	0	0	0
2027 TOTAL TRAFFIC VOLUMES	6	139	10	1	182	6	5	0	5	0	0	0

TRAFFIC VOLUME WORKSHEET ROBERT ROSE DRIVE AT MARYLEBONE STREET/CLARI PARK ACCESS #8 A.M. PEAK HOUR



Description		Northboun bert Rose Thru	-	_	Southbour bert Rose Thru	-		Eastbound Park Acco Thru			Vestboun Irylebone Thru	
2020 EXISTING TRAFFIC VOLUMES		34	4	1	55	9			9	16		6
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor Annual Background Growth Trips	1.00	3.0 1.23 8	3.0 1.23 1	3.0 1.23 0	3.0 1.23 13	1.00	1.00	1.00	1.00	3.0 1.23 4	1.00	3.0 1.23 1
2027 Background Traffic Volumes	0	42	5	1	68	0	0	0	0	20	0	7
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	1	17 1 75	0	0	1 17 63	1	1 3	0	1 3	0	0	0
2027 Site Traffic Volumes	4	75	0	0	63	4	3	0	3	0	0	0
2027 TOTAL TRAFFIC VOLUMES	4	117	5	1	131	4	3	0	3	20	0	7

TRAFFIC VOLUME WORKSHEET ROBERT ROSE DRIVE AT MARYLEBONE STREET/CLARI PARK ACCESS #8 P.M. PEAK HOUR



Description	Ro	Northboun bert Rose	Dr	Ro	Southbour bert Rose	e Dr	Clari	Eastboun Park Acc	ess #8	Ma	Westboun Irylebone	St
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES		76	18	4	65					11		1
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)		3.0	3.0	3.0	3.0					3.0		3.0
Growth Factor	1.00	1.23	1.23	1.23	1.23	1.00	1.00	1.00	1.00	1.23	1.00	1.23
Annual Background Growth Trips	0	17	4	1	15	0	0	0	0	3	0	0
2027 Background Traffic Volumes	0	93	22	5	80	0	0	0	0	14	0	1
2027 SITE TRAFFIC VOLUMES												
% In	1	17			1	1						
Clari Park Primary Trips % Out		1			17		1		1			
Trips	6	103	0	0	97	6	5	0	5	0	0	0
2027 Site Traffic Volumes	6	103	0	0	97	6	5	0	5	0	0	0
2027 TOTAL TRAFFIC VOLUMES	6	196	22	5	177	6	5	0	5	14	0	1

TRAFFIC VOLUME WORKSHEET ROBERT ROSE DRIVE AT SCULLING STREET/CLARI PARK ACCESS #9 A.M. PEAK HOUR



Description		Northboun bert Rose Thru	-		Southbour bert Rose Thru			Eastboun Park Acc Thru			Westboun Sculling S Thru	
2020 EXISTING TRAFFIC VOLUMES		30	3	2	69					25		8
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)		3.0	3.0	3.0	3.0					3.0		3.0
Growth Factor	1.00	1.23	1.23	1.23	1.23	1.00	1.00	1.00	1.00	1.23	1.00	1.23
Annual Background Growth Trips	0	7	1	0	16	0	0	0	0	6	0	2
2027 Background Traffic Volumes	0	37	4	2	85	0	0	0	0	31	0	10
2027 SITE TRAFFIC VOLUMES												
% In	3	18				1						
Clari Park Primary Trips % Out					18		1		3			
Trips	13	75	0	0	62	4	3	0	10	0	0	0
2027 Site Traffic Volumes	13	75	0	0	62	4	3	0	10	0	0	0
2027 TOTAL TRAFFIC VOLUMES	13	112	4	2	147	4	3	0	10	31	0	10

TRAFFIC VOLUME WORKSHEET ROBERT ROSE DRIVE AT SCULLING STREET/CLARI PARK ACCESS #9 P.M. PEAK HOUR



Description		Northboun bert Rose Thru	-		Southboun bert Rose Thru	-		Eastbound Park Acco Thru			Westboun Sculling S Thru	
2020 EXISTING TRAFFIC VOLUMES		93	28	6	70		2011			18		0
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor Annual Background Growth Trips	1.00	3.0 1.23 21	3.0 1.23 6	3.0 1.23 1	3.0 1.23 16	1.00	1.00	1.00	1.00	3.0 1.23 4	1.00	3.0 1.23 0
2027 Background Traffic Volumes	0	114	34	7	86	0	0	0	0	22	0	0
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	3 17	18 104	0	0	18 97	1 6	1 5	0	3 16	0	0	0
2027 Site Traffic Volumes	17	104	0	0	97	6	5	0	16	0	0	0
2027 TOTAL TRAFFIC VOLUMES	17	218	34	7	183	6	5	0	16	22	0	0

TRAFFIC VOLUME WORKSHEET GRESHAMPARK DRIVE AT CLARI PARK ACCESS #1 A.M. PEAK HOUR



Description	· ·	Northboun shampar Thru	-	_	Southboun shampar Thru		Left	Eastbound Thru	d Right	· -	Vestboun Park Acce Thru	-
2020 EXISTING TRAFFIC VOLUMES	2011	471		20.1	216	g	2011		rugui	2011		- ug.u
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor Annual Background Growth Trips	1.00	3.0 1.23 108	1.00	1.00	3.0 1.23 50	1.00 0	1.00	1.00	1.00 0	1.00	1.00	1.00
2027 Background Traffic Volumes	0	579	0	0	266	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	0	2	13 54	2	2 7	0	0	0	0	13 45	0	2 7
2027 Site Traffic Volumes	0	8	54	8	7	0	0	0	0	45	0	7
Willowoak Dr Extension Adjustment Trips		-234			-91							
2027 TOTAL TRAFFIC VOLUMES	0	353	54	8	182	0	0	0	0	45	0	7

TRAFFIC VOLUME WORKSHEET GRESHAMPARK DRIVE AT CLARI PARK ACCESS #1 P.M. PEAK HOUR



Description		Northboun eshampar Thru	-	_	Southboun shampar Thru		Left	Eastbound Thru	d Right	· -	Vestboun Park Acce Thru	-
2020 EXISTING TRAFFIC VOLUMES		289	<u> </u>		539	<u> </u>			<u> </u>			<u> </u>
2027 BACKGROUND TRAFFIC VOLUMES Annual Background Growth												
Growth Rate (%/year) Growth Factor Annual Background Growth Trips	1.00	3.0 1.23 66	1.00	1.00	3.0 1.23 124	1.00	1.00	1.00	1.00	1.00	1.00	1.00
2027 Background Traffic Volumes	0	355	0	0	663	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In Clari Park Primary Trips % Out Trips	0	2 12	13 75	2 12	2 11	0	0	0	0	13 70	0	2 11
2027 Site Traffic Volumes	0	12	75	12	11	0	0	0	0	70	0	11
Willowoak Dr Extension Adjustment Trips		-152			-219							
2027 TOTAL TRAFFIC VOLUMES	0	215	75	12	455	0	0	0	0	70	0	11

TRAFFIC VOLUME WORKSHEET WILLOWOAK TRAIL AT CLARI PARK ACCESS #2/#3 A.M. PEAK HOUR



Description	Clari	Northboun Park Acc	ess #4	Clari	Southbour Park Acc	ess #3		Eastboun I lowoak 1	rail		Vestboun Iowoak T	rail
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES												
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth	İ											
Growth Rate (%/year)												
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Annual Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0
2027 Background Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In							1	2	23	23		2
Clari Park Primary Trips % Out	23		23	2		1					2	
Trips	79	0	79	7	0	3	4	8	96	96	7	8
2027 Site Traffic Volumes	79	0	79	7	0	3	4	8	96	96	7	8
Willowoak Dr Extension Adjustment Trips								234			91	
2027 TOTAL TRAFFIC VOLUMES	79	0	79	7	0	3	4	242	96	96	98	8

TRAFFIC VOLUME WORKSHEET WILLOWOAK TRAIL AT CLARI PARK ACCESS #2/#3 P.M. PEAK HOUR



Description		Northboun Park Acc Thru			Southboun Park Acco			Eastbound Iowoak T Thru			Vestboun Iowoak T Thru	
2020 EXISTING TRAFFIC VOLUMES												<u> </u>
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year) Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Growth Factor Annual Background Growth Trips		0	0	0	0	0	0	0	0	0	0	0
Annual Background Growth Trips	U	- 0	- 0	- 0	- 0	- 0	0	- 0	- 0	0	- 0	- 0
2027 Background Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In							1	2	23	23		2
Clari Park Primary Trips % Out	23		23	2		1					2	
Trips	124	0	124	11	0	5	6	12	133	133	11	12
2027 Site Traffic Volumes	124	0	124	11	0	5	6	12	133	133	11	12
Willowoak Dr Extension Adjustment Trips								152			219	
2027 TOTAL TRAFFIC VOLUMES	124	0	124	11	0	5	6	164	133	133	230	12

TRAFFIC VOLUME WORKSHEET HONEYLOCUST LN AT CLARI PARK ACCESS #4/#5 A.M. PEAK HOUR



Description	Clari	Northboun Park Acc	ess #6	Clari	Southboun	ess #5	Ho	Eastboun neylocus	t Ln	Clari F	Vestboun	ss #11
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES												
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)												
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Annual Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0
2027 Background Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In							7	9	7			
Clari Park Primary Trips % Out	7					7					9	
Trips	24	0	0	0	0	24	29	38	29	0	31	0
2027 Site Traffic Volumes	24	0	0	0	0	24	29	38	29	0	31	0
2027 TOTAL TRAFFIC VOLUMES	24	0	0	0	0	24	29	38	29	0	31	0

TRAFFIC VOLUME WORKSHEET HONEYLOCUST LN AT CLARI PARK ACCESS #4/#5 P.M. PEAK HOUR



Description		Northboun		_	outhbour			Eastboun neylocus		\	Vestboun	d
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC VOLUMES												
2027 BACKGROUND TRAFFIC VOLUMES												
Annual Background Growth												
Growth Rate (%/year)												
Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Annual Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0
2027 Background Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOLUMES												
% In							7	9	7			
Clari Park Primary Trips % Out	7					7					9	
Trips	38	0	0	0	0	38	40	52	40	0	48	0
2027 Site Traffic Volumes	38	0	0	0	0	38	40	52	40	0	48	0
2027 TOTAL TRAFFIC VOLUMES	38	0	0	0	0	38	40	52	40	0	48	0

TRAFFIC VOLUME WORKSHEET WILLOWOAK TRAIL AT CLARI PARK ACCESS #10 A.M. PEAK HOUR



Des	scription		Northboun	-	S	Southboun	ıd		Eastboun			Vestboun	
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020 EXISTING TRAFFIC	VOLUMES												
2027 BACKGROUND TRA	AFFIC VOLUMES												
Annual Background	Growth												
Growth Rate (%/yea	r)												
Growth Factor		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
A	Annual Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0
20	027 Background Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2027 SITE TRAFFIC VOL	UMES												
	% In								1	1	1	25	
Clari Park	Primary Trips % Out	1		1					25			1	
	Trips	3	0	3	0	0	0	0	90	4	4	108	0
	2027 Site Traffic Volumes	3	0	3	0	0	0	0	90	4	4	108	0
Willowoak Dr Extens	sion Adjustment Trips								234			91	
2027 TOTAL TRAFFIC VO	DLUMES	3	0	3	0	0	0	0	324	4	4	199	0

TRAFFIC VOLUME WORKSHEET WILLOWOAK TRAIL AT CLARI PARK ACCESS #10 P.M. PEAK HOUR



P.IVI. F	PEAK HOUR												
			Northboun		S	Southbour	nd		Eastbound			Vestboun	-
	Description	Clari P	ark Acce	ss #10				Wi	lowoak T	rail	Wi	Iowoak T	rail
		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
2020	EXISTING TRAFFIC VOLUMES												
2027	BACKGROUND TRAFFIC VOLUMES												
	Annual Background Growth												
	Growth Rate (%/year)												
	Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	Annual Background Growth Trips	0	0	0	0	0	0	0	0	0	0	0	0
	2027 Background Traffic Volumes	0	0	0	0	0	0	0	0	0	0	0	0
2027	SITE TRAFFIC VOLUMES												
	% In								1	1	1	25	
	Clari Park Primary Trips % Out	1		1					25			1	
	Trips	5	0	5	0	0	0	0	140	6	6	150	0
	2027 Site Traffic Volumes	5	0	5	0	0	0	0	140	6	6	150	0
	Willowoak Dr Extension Adjustment Trips								152			219	
2027	TOTAL TRAFFIC VOLUMES	5	0	5	0	0	0	0	292	6	6	369	0

APPENDIX D CAPACITY ANALYSIS WORKSHEETS EXISTING TRAFFIC

	•	→	•	·	←	•	•	<u>†</u>	<u></u>	<u> </u>		~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	†	7	ሻሻ	†	1	ሻ	^	7	*	^	7
Traffic Volume (veh/h)	18	19	57	33	19	165	32	488	22	522	1211	22
Future Volume (veh/h)	18	19	57	33	19	165	32	488	22	522	1211	22
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	20	21	64	37	21	-19	36	548	25	587	1361	25
Adj No. of Lanes	2	1	1	2	1	1	1	2	1	1	2	1
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	68	105	89	98	121	103	282	2246	1005	688	2351	1052
Arrive On Green	0.02	0.06	0.06	0.03	0.06	0.00	0.06	1.00	1.00	0.06	0.66	0.66
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	1774	3539	1583	1774	3539	1583
Grp Volume(v), veh/h	20	21	64	37	21	-19	36	548	25	587	1361	25
Grp Sat Flow(s), veh/h/ln	1721	1863	1583	1721	1863	1583	1774	1770	1583	1774	1770	1583
Q Serve(q_s), s	0.7	1.4	5.2	1.4	1.4	0.0	0.9	0.0	0.0	7.5	27.3	0.7
Cycle Q Clear(g_c), s	0.7	1.4	5.2	1.4	1.4	0.0	0.9	0.0	0.0	7.5	27.3	0.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	68	105	89	98	121	103	282	2246	1005	688	2351	1052
V/C Ratio(X)	0.29	0.20	0.72	0.38	0.17	-0.19	0.13	0.24	0.02	0.85	0.58	0.02
Avail Cap(c_a), veh/h	212	272	231	212	272	231	334	2246	1005	688	2351	1052
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	0.99	0.99	0.99	1.00	1.00	1.00
Uniform Delay (d), s/veh	62.8	58.6	60.3	62.0	57.5	0.0	9.6	0.0	0.0	13.6	11.9	7.4
Incr Delay (d2), s/veh	1.8	1.1	12.3	1.8	0.8	0.0	0.1	0.3	0.0	9.9	1.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	8.0	2.6	0.7	0.7	0.0	0.4	0.1	0.0	15.1	13.6	0.3
LnGrp Delay(d),s/veh	64.6	59.7	72.6	63.8	58.3	0.0	9.7	0.3	0.0	23.5	12.9	7.5
LnGrp LOS	Ε	Ε	Ε	Ε	Ε		Α	Α	Α	С	В	А
Approach Vol, veh/h		105			39			609			1973	
Approach Delay, s/veh		68.5			92.0			0.8			16.0	
Approach LOS		E			F			А			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.1	94.9	10.7	13.3	15.0	91.0	9.6	14.4				
Change Period (Y+Rc), s	7.5	8.5	7.0	6.0	7.5	8.5	7.0	6.0				
Max Green Setting (Gmax), s	7.5	66.5	8.0	19.0	7.5	66.5	8.0	19.0				
Max Q Clear Time (g_c+I1), s	2.9	29.3	3.4	7.2	9.5	2.0	2.7	3.4				
Green Ext Time (p_c), s	0.0	33.1	0.0	0.3	0.0	53.4	0.0	0.4				
Intersection Summary												
HCM 2010 Ctrl Delay			15.7									
HCM 2010 LOS			В									

	_				_	
ر	•	\rightarrow	4	†	↓	4
Mayamant	DI	- EDD	ND.	NDT	CDT	CDD
	BL_	EBR	NBL	NBT	SBT	SBR
	ነኘ	7	4,4	^	^	7
` ,	20	9	33	484	1267	24
` '	20	9	33	484	1267	24
Number	7	14	1	6	2	12
Initial Q (Qb), veh	0	0	0	0	0	0
, _ı ,	00	1.00	1.00			1.00
Parking Bus, Adj 1.	00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln 18	63	1863	1863	1863	1863	1863
	23	10	38	563	1473	28
Adj No. of Lanes	2	1	2	2	2	1
	86	0.86	0.86	0.86	0.86	0.86
Percent Heavy Veh, %	2	2	2	2	2	2
	92	42	99	2995	2662	1191
	03	0.03	0.06	1.00	1.00	1.00
Sat Flow, veh/h 34		1583	3442	3632	3632	1583
. , , .	23	10	38	563	1473	28
Grp Sat Flow(s), veh/h/ln17		1583	1721	1770	1770	1583
(5—):).9	0.8	1.4	0.0	0.0	0.0
7 10- 7).9	0.8	1.4	0.0	0.0	0.0
•	00	1.00	1.00			1.00
1 1 1 7 7 1	92	42	99	2995	2662	1191
V/C Ratio(X) 0.	25	0.24	0.38	0.19	0.55	0.02
Avail Cap(c_a), veh/h 3	44	158	304	2995	2662	1191
HCM Platoon Ratio 1.	00	1.00	2.00	2.00	2.00	2.00
Upstream Filter(I) 1.	00	1.00	0.99	0.99	0.82	0.82
Uniform Delay (d), s/veh 62		62.0	60.2	0.0	0.0	0.0
9	1.4	2.8	2.4	0.1	0.7	0.0
Initial Q Delay(d3),s/veh (0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/lr		0.0	0.0	0.0	0.0	0.0
			62.6	0.1	0.3	0.0
3 . ,	3.4	64.8				
LnGrp LOS	E	E	E	A	A	A
11	33			601	1501	
Approach Delay, s/veh 63				4.1	0.7	
Approach LOS	Ε			Α	Α	
Timer	1	2	3	4	5	6
Assigned Phs	1	2	J	4	J	6
3						
Phs Duration (G+Y+Rc), \$2				10.5		119.5
Change Period (Y+Rc), s &		9.5		7.0		9.5
Max Green Setting (Gmak)		80.5		13.0		100.5
Max Q Clear Time (g_c+l13	, .	2.0		2.9		2.0
Green Ext Time (p_c), s (0.0	28.3		0.0		29.7
Intersection Summary						
HCM 2010 Ctrl Delay			2.6			
HCM 2010 LOS			Α.			
HOW ZOTO LOS			А			

و	7	• 4	†	Ţ	4	
Mouamont		י אי	, NDT	CDT	CDD	
Movement EB		BR NB		SBT	SBR	
Lane Configurations		<u>ሾ</u> ኝ		† †	7	
Traffic Volume (veh/h) 1		5 3		1208	49	
Future Volume (veh/h) 1		5 3		1208	49	
Number		14	1 6	2	12	
` '.)		0 0	0	0	
Ped-Bike Adj(A_pbT) 1.0		.00 1.0			1.00	
Parking Bus, Adj 1.0		.00 1.0		1.00	1.00	
Adj Sat Flow, veh/h/ln 186		63 186		1863	1863	
Adj Flow Rate, veh/h 1	2	6 4	593	1438	58	
Adj No. of Lanes	2	1	2 2	2	1	
Peak Hour Factor 0.8	4 0.	84 0.8	4 0.84	0.84	0.84	
Percent Heavy Veh, %	2	2	2 2	2	2	
Cap, veh/h		29 10		2785	1246	
Arrive On Green 0.0		.02 0.0		0.79	0.79	
Sat Flow, veh/h 344		83 344		3632	1583	
Grp Volume(v), veh/h 1		6 4		1438	58	
Grp Sat Flow(s), veh/h/ln172		83 172		1770	1583	
Q Serve(g_s), s 0.		0.5 1/2		19.0	1.1	
					1.1	
Cycle Q Clear(g_c), s 0.		0.5 1.		19.0		
Prop In Lane 1.0		00 1.0		0705	1.00	
Lane Grp Cap(c), veh/h 6		29 10		2785	1246	
V/C Ratio(X) 0.1		21 0.4		0.52	0.05	
Avail Cap(c_a), veh/h 34		58 30		2785	1246	
HCM Platoon Ratio 1.0		.00 1.0		1.00	1.00	
Upstream Filter(I) 1.0		00 0.9		0.83	0.83	
Uniform Delay (d), s/veh 62	9 62	2.9 62.	0 1.1	5.0	3.1	
Incr Delay (d2), s/veh 1.	4 3	3.4 2.	5 0.1	0.6	0.1	
Initial Q Delay(d3),s/veh 0.) (0.0	0.0	0.0	0.0	
%ile BackOfQ(50%), veh/lr0		0.2 0.		9.4	0.5	
LnGrp Delay(d),s/veh 64.		5.3 64.		5.5	3.1	
	-		E A	A	A	
Approach Vol, veh/h 1			633	1496		
11 '			5.2	5.5		
Approach LOS	J <u>-</u>		3.2 A	3.3 A		
Approach LOS			А	А		
Timer	1	2	3 4	5	6	
Assigned Phs	1	2	4		6	
Phs Duration (G+Y+Rc), \$2	3 108	3.3	9.4		120.6	
Change Period (Y+Rc), s 8		5.0	7.0		6.0	
Max Green Setting (Gmax),		4.0	13.0		104.0	
Max Q Clear Time (g_c+l13)		1.0	2.5		5.1	
Green Ext Time (p_c), s 0.		5.5	0.0		29.7	
	ب کار	5.0	0.0		۷.۱	
Intersection Summary						
HCM 2010 Ctrl Delay		5.	9			
HCM 2010 LOS			4			

-					
→	>	6	←	•	-
	*	T		1	•
Movement EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations ††	7	1,4		ሻሻ	7
Traffic Volume (veh/h) 1199	11	32	530	0	9
Future Volume (veh/h) 1199	11	32	530	0	9
Number 2	12	1	6	7	14
Initial Q (Qb), veh 0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00
Parking Bus, Adj 1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln 1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h 1462	13	39	646	0	11
Adj No. of Lanes 2	1	2	2	2	1
Peak Hour Factor 0.82	0.82	0.82	0.82	0.82	0.82
	0.62	0.62	0.62		
<i>j</i> '				2	2
Cap, veh/h 2820	1262	100	3141	43	20
Arrive On Green 0.80	0.80	0.06	1.00	0.00	0.01
Sat Flow, veh/h 3632	1583	3442	3632	3442	1583
Grp Volume(v), veh/h 1462	13	39	646	0	11
Grp Sat Flow(s), veh/h/ln1770	1583	1721	1770	1721	1583
Q Serve(g_s), s 18.6	0.2	1.4	0.0	0.0	0.9
Cycle Q Clear(g_c), s 18.6	0.2	1.4	0.0	0.0	0.9
Prop In Lane	1.00	1.00		1.00	1.00
Lane Grp Cap(c), veh/h 2820	1262	100	3141	43	20
V/C Ratio(X) 0.52	0.01	0.39	0.21	0.00	0.55
Avail Cap(c_a), veh/h 2820	1262	318	3141	331	152
HCM Platoon Ratio 1.00	1.00	2.00	2.00	1.00	1.00
Upstream Filter(I) 0.86	0.86	0.98	0.98	0.00	1.00
Uniform Delay (d), s/veh 4.6	2.7	60.1	0.0	0.0	63.8
Incr Delay (d2), s/veh 0.6	0.0	2.4	0.1	0.0	21.6
Initial Q Delay(d3),s/veh 0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/lr9.2	0.1	0.7	0.1	0.0	0.5
LnGrp Delay(d),s/veh 5.2	2.7	62.5	0.1	0.0	85.4
LnGrp LOS A	Α	Ε	Α		F
Approach Vol, veh/h 1475			685	11	
Approach Delay, s/veh 5.1			3.7	85.4	
Approach LOS A			A	F	
			,,		
Timer 1	2	3	4	5	6
Assigned Phs 1	2		4		6
Phs Duration (G+Y+Rc), \$1.8	109.1		9.1		120.9
Change Period (Y+Rc), s 8.0	5.5		7.5		5.5
Max Green Setting (Gmalk), @	84.5		12.5		104.5
Max Q Clear Time (q_c+l13,4s			2.9		2.0
Green Ext Time (p_c), s 0.0	27.9		0.0		31.7
•	21.7		0.0		01.7
Intersection Summary					
HCM 2010 Ctrl Delay		5.1			
HCM 2010 LOS		Α			
		, ,			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	ነ	^	7	14	•	7	ሻሻ		7
Traffic Volume (veh/h)	31	1053	125	32	424	11	94	13	30	47	21	49
Future Volume (veh/h)	31	1053	125	32	424	11	94	13	30	47	21	49
Number	5	2	12	1	6	16	7	4	14	3	8	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	37	1254	149	38	505	13	112	15	36	56	25	58
Adj No. of Lanes	1	2	1	1	2	1	2	1	1	2	1	1
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	621	2294	1026	353	2281	1021	162	110	93	115	99	84
Arrive On Green	0.06	1.00	1.00	0.03	0.64	0.64	0.05	0.06	0.06	0.03	0.05	0.05
	1774	3539	1583	1774	3539	1583	3442	1863	1583	3442	1863	1583
Grp Volume(v), veh/h	37	1254	149	38	505	13	112	15	36	56	25	58
Grp Sat Flow(s), veh/h/lr		1770	1583	1774	1770	1583	1721	1863	1583	1721	1863	1583
Q Serve(g_s), s	0.9	0.0	0.0	0.9	7.7	0.4	4.2	1.0	2.8	2.1	1.7	4.7
Cycle Q Clear(g_c), s	0.9	0.0	0.0	0.9	7.7	0.4	4.2	1.0	2.8	2.1	1.7	4.7
Prop In Lane	1.00	0.0	1.00	1.00	,.,	1.00	1.00	1.0	1.00	1.00	1.,	1.00
Lane Grp Cap(c), veh/h		2294	1026	353	2281	1021	162	110	93	115	99	84
V/C Ratio(X)	0.06	0.55	0.15	0.11	0.22	0.01	0.69	0.14	0.39	0.49	0.25	0.69
Avail Cap(c_a), veh/h	666	2294	1026	404	2281	1021	225	201	171	199	201	171
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.86	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh		0.00	0.0	7.1	9.6	8.3	61.0	58.0	58.9	61.7	59.1	60.5
Incr Delay (d2), s/veh	0.0	0.8	0.0	0.1	0.2	0.0	5.2	0.6	2.6	3.2	1.3	9.7
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh		0.0	0.0	0.5	3.8	0.0	2.1	0.5	1.3	1.0	0.0	2.3
LnGrp Delay(d),s/veh	7.2	0.3	0.1	7.2	9.8	8.3	66.2	58.6	61.5	64.9	60.4	70.2
LnGrp LOS	Α.2	Α	Α	Α.2	Α.	Α	00.2 E	50.0 E	61.5 E	U4.7	E	70.2 E
Approach Vol, veh/h		1440			556			163			139	
		0.9			9.6			64.5			66.3	
Approach Delay, s/veh Approach LOS		0.9 A			9.0 A			04.5 E			00.3 E	
											L	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc)		93.3	11.8	13.7	11.7	92.8	12.6	12.9				
Change Period (Y+Rc),	s 7.5	9.0	7.5	6.0	8.0	9.0	6.5	6.0				
Max Green Setting (Gm		71.0	7.5	14.0	7.0	71.0	8.5	14.0				
Max Q Clear Time (g_c-	H112),9s	2.0	4.1	4.8	2.9	9.7	6.2	6.7				
Green Ext Time (p_c), s	0.0	21.8	0.0	0.3	0.0	21.3	0.1	0.2				
Intersection Summary												
HCM 2010 Ctrl Delay			11.5									
HCM 2010 LOS			В									
TIONI ZUTU LUJ			D									

Intersection						
Int Delay, s/veh	10.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations		LDK	WDL	्रस	NDL	NDK
Traffic Vol, veh/h	27	3	213	4 24	1	462
Future Vol, veh/h	27	3	213	24	9	462
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	- -	None
Storage Length	_	TVOTIC	_	NOTIC -	170	0
Veh in Median Storage,		_		0	0	-
Grade, %	π 0 0	-	-	0	0	-
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	32	4	254	29	11	550
IVIVIII I IOW	JZ	4	254	27	11	550
Major/Minor N	1ajor1		Major2		Vinor1	
Conflicting Flow All	0	0	36	0	571	34
Stage 1	-	-	-	-	34	-
Stage 2	-	-	-	-	537	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1575	-	482	1039
Stage 1	-	-	-	-	988	-
Stage 2	-	-	-	-	586	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1575	-	403	1039
Mov Cap-2 Maneuver	-	-	-	-	403	-
Stage 1	-	-	-	-	988	-
Stage 2	-	-	-	-	490	-
Ü						
Annroach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		6.9		12.3	
HCM LOS					В	
Minor Lane/Major Mvmt	t l	VBLn1 I	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		403	1039	-	_	1575
HCM Lane V/C Ratio		0.027		-		0.161
HCM Control Delay (s)		14.2	12.3	-	-	7.7
HCM Lane LOS		В	В	-	-	Α
HCM 95th %tile Q(veh)		0.1	3.2	-	-	0.6
2(1011)						5.5

Intersection						
Int Delay, s/veh	1.1					
		FDT	WDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	17	470	}	10	7	21
Traffic Vol, veh/h	17	472	204	12	21	31
Future Vol, veh/h	17	472	204	12	21	31
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	:,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	20	562	243	14	25	37
Major/Minor N	Major1	N	Major2		Minor2	
Conflicting Flow All	257	0	viajoiz	0	852	250
Stage 1	207	-	-	-	250	230
Stage 2	-	-	-	-	602	-
		-	-			
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-		3.318
Pot Cap-1 Maneuver	1308	-	-	-	330	789
Stage 1	-	-	-	-	792	-
Stage 2	-	-	-	-	547	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1308	-	-	-	323	789
Mov Cap-2 Maneuver	-	-	-	-	323	-
Stage 1	-	-	-	-	775	-
Stage 2	-	-	-	-	547	-
	ΕD		\M/R		SB	
Approach	EB		WB		SB	
Approach HCM Control Delay, s	EB 0.3		WB 0		13.2	
Approach						
Approach HCM Control Delay, s					13.2	
Approach HCM Control Delay, s HCM LOS	0.3	EBL		WBT	13.2	SBLn1
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	0.3		0	WBT	13.2 B	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	0.3	1308	0 EBT		13.2 B WBR :	499
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	0.3	1308 0.015	0 EBT -	-	13.2 B WBR :	499 0.124
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	0.3	1308 0.015 7.8	0 EBT - - 0	- - -	13.2 B WBR :	499 0.124 13.2
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	0.3	1308 0.015	0 EBT -	-	13.2 B WBR :	499 0.124

Intersection							
Int Delay, s/veh	1.6						
	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<u>₽</u>	LDI	WDL	्रस	NDL	NDK	
Traffic Vol, veh/h	478	19	18	182	1 31	45	
Future Vol, veh/h	478	19	18	182	31	45	
Conflicting Peds, #/hr	4/0	0	0	0	0	0	
	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	310p	None	
Storage Length	-	NOTIC -	-	None -	100	0	
Veh in Median Storage, #	# 0		-	0	0	-	
		-	-			-	
Grade, %	0		- 07	0	0		
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	549	22	21	209	36	52	
Major/Minor Ma	ajor1	1	Major2	1	Vinor1		
Conflicting Flow All	0	0	571	0	811	560	
Stage 1	-	-	-	-	560	-	
Stage 2	-	-	-	-	251	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	_	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	_		2.218	_		3.318	
Pot Cap-1 Maneuver	-	-	1002	-	349	528	
Stage 1	_		-	_	572	-	
Stage 2	_	_	_	_	791	_	
Platoon blocked, %	_			_	171		
Mov Cap-1 Maneuver	-		1002	-	341	528	
Mov Cap-2 Maneuver	-		1002	-	341	520	
	-	-	-		572	-	
Stage 1	-	-	-	-		-	
Stage 2	-	-	-	-	772	-	
Approach	EB		WB		NB		
HCM Control Delay, s	0		0.8		14.3		
HCM LOS					В		
Minor Lane/Major Mvmt	ı	NBLn1 ľ	VIRI n2	EBT	EBR	WBL	
Capacity (veh/h)		341	528	-	-	1002	
HCM Control Polov (a)		0.104		-		0.021	
HCM Long LOS		16.8	12.6	-	-	8.7	
HCM Lane LOS		С	В	-	-	A	
HCM 95th %tile Q(veh)		0.3	0.3	-	-	0.1	

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĥ		ሻ	ĵ,		ሻ	(Î			4	
Traffic Vol, veh/h	0	0	0	18	0	6	0	2	39	0	3	0
Future Vol, veh/h	0	0	0	18	0	6	0	2	39	0	3	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	90	-	-	100	-	-	-	-	-
Veh in Median Storage	:,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	20	0	7	0	2	44	0	3	0
Major/Minor N	Major1		N	Major2			Minor1		N	Minor2		
Conflicting Flow All	7	0	0	1	0	0	46	48	1	68	45	4
Stage 1	-	-	-	-	-	-	1	1	-	44	44	-
Stage 2	-	-	-	-	-	-	45	47	-	24	1	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1614	-	-	1622	-	-	955	844	1084	925	847	1080
Stage 1	-	-	-	-	-	-	1022	895	-	970	858	-
Stage 2	-	-	-	-	-	-	969	856	-	994	895	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1614	-	-	1622	-	-	944	834	1084	878	837	1080
Mov Cap-2 Maneuver	-	-	-	-	-	-	944	834	-	878	837	-
Stage 1	-	-	-	-	-	-	1022	895	-	970	848	-
Stage 2	-	-	-	-	-	-	953	846	-	951	895	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			5.4			8.5			9.3		
HCM LOS	U			J. 1			Α			7.3 A		
TIOWI LOO							Α.			71		
Minor Lane/Major Mvm	nt r	VBLn1 N		EBL	EBT	EBR	WBL	WBT	WBR S			
Capacity (veh/h)			1068	1614	-	-	1622	-	-	837		
HCM Lane V/C Ratio			0.043	-	-	-	0.012	-	-	0.004		
HCM Control Delay (s)		0	8.5	0	-	-	7.2	-	-	9.3		
HCM Lane LOS		Α	Α	Α	-	-	Α	-	-	Α		
HCM 95th %tile Q(veh)		-	0.1	0	-	-	0	-	-	0		

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	44	†	7	14.14	†	7	7	^	7	7	^	7
Traffic Volume (veh/h)	109	35	46	65	51	463	77	1389	56	304	1024	86
Future Volume (veh/h)	109	35	46	65	51	463	77	1389	56	304	1024	86
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	117	38	31	70	55	309	83	1494	37	327	1101	57
Adj No. of Lanes	2	1	1	2	1	1	1	2	1	1	2	1
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	162	227	193	109	199	169	323	2016	902	389	2196	983
Arrive On Green	0.05	0.12	0.12	0.03	0.11	0.11	0.06	1.00	1.00	80.0	0.62	0.62
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	1774	3539	1583	1774	3539	1583
Grp Volume(v), veh/h	117	38	31	70	55	309	83	1494	37	327	1101	57
Grp Sat Flow(s), veh/h/ln	1721	1863	1583	1721	1863	1583	1774	1770	1583	1774	1770	1583
Q Serve(g_s), s	5.0	2.7	2.6	3.0	4.1	16.0	3.0	0.0	0.0	11.3	25.7	2.1
Cycle Q Clear(g_c), s	5.0	2.7	2.6	3.0	4.1	16.0	3.0	0.0	0.0	11.3	25.7	2.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	162	227	193	109	199	169	323	2016	902	389	2196	983
V/C Ratio(X)	0.72	0.17	0.16	0.64	0.28	1.83	0.26	0.74	0.04	0.84	0.50	0.06
Avail Cap(c_a), veh/h	229	227	193	184	199	169	401	2016	902	389	2196	983
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.82	0.82	0.82	1.00	1.00	1.00
Uniform Delay (d), s/veh	70.5	59.0	59.0	71.8	61.7	67.0	13.3	0.0	0.0	10.6	15.7	11.2
Incr Delay (d2), s/veh	5.0	0.4	0.5	4.6	0.9	395.5	0.3	2.1	0.1	14.9	8.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.5	1.4	1.2	1.5	2.2	25.5	1.5	0.6	0.0	7.1	12.8	1.0
LnGrp Delay(d),s/veh	75.5	59.5	59.5	76.3	62.6	462.5	13.6	2.1	0.1	25.6	16.5	11.3
LnGrp LOS	Ε	Ε	Ε	Ε	Е	F	В	Α	Α	С	В	В
Approach Vol, veh/h		186			434			1614			1485	
Approach Delay, s/veh		69.6			349.5			2.6			18.3	
Approach LOS		Е			F			А			В	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.4	101.6	11.8	24.3	20.0	94.0	14.0	22.0				
Change Period (Y+Rc), s	7.5	8.5	7.0	6.0	7.5	8.5	7.0	6.0				
Max Green Setting (Gmax), s	11.5	83.5	8.0	18.0	12.5	82.5	10.0	16.0				
Max Q Clear Time (g_c+I1), s	5.0	27.7	5.0	4.7	13.3	2.0	7.0	18.0				
Green Ext Time (p_c), s	0.1	53.8	0.0	1.8	0.0	76.4	0.1	0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			52.7									
HCM 2010 LOS			D									

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	*		7	T	¥	*
Movement EB	. EBR	EBL	NBL	NBT	SBT	SBR
Lane Configurations 3	7	ns ሻ ሻ	1/4	^	^	7
			131	1366	953	170
, ,		•	131	1366	953	170
` ,		7	1	6	2	12
		0	0	0	0	0
` '			1.00			1.00
3 · -1 ·		1.00	1.00	1.00	1.00	1.00
			1863	1863	1863	1863
•			146	1518	1059	189
		2	2	2	2	107
,		0.90	0.90	0.90	0.90	0.90
,			2	2	2	2
		259	190	2883	2487	1113
		0.08	0.11	1.00	1.00	1.00
		3442	3442	3632	3632	1583
. ,			146	1518	1059	189
Grp Sat Flow(s), veh/h/ln172			1721	1770	1770	1583
10- /-		7.9	6.2	0.0	0.0	0.0
Cycle Q Clear(g_c), s 7.	8.6	c), s 7.9	6.2	0.0	0.0	0.0
Prop In Lane 1.0	1.00	1.00	1.00			1.00
Lane Grp Cap(c), veh/h 25	119	veh/h 259	190	2883	2487	1113
		0.72	0.77	0.53	0.43	0.17
	243	eh/h 528	264	2883	2487	1113
			2.00	2.00	2.00	2.00
		1.00	0.85	0.85	0.84	0.84
Uniform Delay (d), s/veh 67.8			65.8	0.0	0.0	0.0
			7.4	0.6	0.5	0.3
Initial Q Delay(d3),s/veh 0.			0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/lr3.			3.1	0.0	0.0	0.0
1 3 1 7			73.2	0.6	0.5	0.3
		E	<u>E</u>	A	A	A
				1664	1248	
[] [] [] [] [] [] [] [] [] []				7.0	0.4	
Approach LOS I		E		Α	Α	
Timer	2	1	3	4	5	6
		1		4		6
Phs Duration (G+Y+Rc), \$6.5				18.3		131.7
Change Period (Y+Rc), s 8.						
· ,		, .		7.0		9.5
Max Green Setting (Gmax),				23.0		110.5
Max Q Clear Time (g_c+l18,;		.0		10.6		2.0
Green Ext Time (p_c), s 0.	48.2)_c), s 0.1		0.7		52.8
Intersection Summary		nary				
HCM 2010 Ctrl Delay		elay	10.2			
HCM 2010 LOS		,	В			

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•	•	1	†	Ų.	4	
Movement EBI	EBR	NBL	NBT	SBT	SBR	
Lane Configurations 7		ካካ	† †	^	₹ 82	
Traffic Volume (veh/h) 11:		184	1400	955		
Future Volume (veh/h) 11:		184	1400	955	82	
Number		1	6	2	12	
Initial Q (Qb), veh		0	0	0	0	
Ped-Bike Adj(A_pbT) 1.00		1.00			1.00	
Parking Bus, Adj 1.00		1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln 1863	1863	1863	1863	1863	1863	
Adj Flow Rate, veh/h 124	164	202	1538	1049	90	
Adj No. of Lanes	1	2	2	2	1	
Peak Hour Factor 0.9	0.91	0.91	0.91	0.91	0.91	
Percent Heavy Veh, %		2	2	2	2	
Cap, veh/h 40		252	2813	2354	1053	
Arrive On Green 0.12		0.15	1.00	1.00	1.00	
Sat Flow, veh/h 3442		3442	3632	3632	1583	
		202	1538	1049	90	
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \						
Grp Sat Flow(s), veh/h/ln172		1721	1770	1770	1583	
Q Serve(g_s), s 4.5		8.5	0.0	0.0	0.0	
Cycle Q Clear(g_c), s 4.9		8.5	0.0	0.0	0.0	
Prop In Lane 1.00		1.00			1.00	
Lane Grp Cap(c), veh/h 40		252	2813	2354	1053	
V/C Ratio(X) 0.30	0.87	0.80	0.55	0.45	0.09	
Avail Cap(c_a), veh/h 528	243	493	2813	2354	1053	
HCM Platoon Ratio 1.00	1.00	2.00	2.00	2.00	2.00	
Upstream Filter(I) 1.00	1.00	0.83	0.83	0.90	0.90	
Uniform Delay (d), s/veh 60.		63.0	0.0	0.0	0.0	
Incr Delay (d2), s/veh 0.4		5.0	0.6	0.5	0.1	
Initial Q Delay(d3),s/veh 0.0		0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/lr2.4		4.2	0.3	0.0	0.0	
LnGrp Delay(d),s/veh 60.9		67.9	0.6	0.2	0.0	
LnGrp LOS E		<u>E</u>	A 7.40	A	A	
Approach Vol, veh/h 288			1740	1139		
Approach Delay, s/veh 76.			8.5	0.5		
Approach LOS E			А	Α		
Timer	2	3	4	5	6	
Assigned Phs			4	J	6	
Phs Duration (G+Y+Rc), \$9.5			24.8		125.2	
Change Period (Y+Rc), s 8.			7.0		6.0	
Max Green Setting (Gmax),			23.0		114.0	
Max Q Clear Time (g_c+ff1),!			17.3		2.0	
Green Ext Time (p_c), s 0.!	45.2		0.5		51.9	
Intersection Summary						
HCM 2010 Ctrl Delay		11.8				
HCM 2010 LOS		В				
1101VI 2010 LU3		Ď				

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	_	*	٧		١,	
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	^	7	ሻሻ	^	ሻሻ	- 7
	1098	13	80	1542	27	113
Future Volume (veh/h)	1098	13	80	1542	27	113
Number	2	12	1	6	7	14
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)		1.00	1.00		1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
	1863	1863	1863	1863	1863	1863
,	1168	14	85	1640	29	120
Adj No. of Lanes	2	1	2	2	2	1
	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2
3						142
	2595	1161	128	2915	309	
	0.73	0.73	0.07	1.00	0.09	0.09
	3632	1583	3442	3632	3442	1583
	1168	14	85	1640	29	120
Grp Sat Flow(s), veh/h/ln		1583	1721	1770	1721	1583
Q Serve(g_s), s	19.7	0.4	3.6	0.0	1.2	11.2
Cycle Q Clear(g_c), s	19.7	0.4	3.6	0.0	1.2	11.2
Prop In Lane		1.00	1.00		1.00	1.00
Lane Grp Cap(c), veh/h	2595	1161	128	2915	309	142
	0.45	0.01	0.66	0.56	0.09	0.85
	2595	1161	390	2915	516	237
	1.00	1.00	2.00	2.00	1.00	1.00
	0.89	0.89	0.69	0.69	1.00	1.00
1 1						
Uniform Delay (d), s/veh		5.4	68.5	0.0	62.7	67.3
Incr Delay (d2), s/veh	0.5	0.0	4.0	0.5	0.1	13.1
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh		0.2	1.8	0.2	0.6	5.4
LnGrp Delay(d),s/veh	8.5	5.4	72.5	0.5	62.8	80.4
LnGrp LOS	Α	Α	Е	Α	Е	F
Approach Vol, veh/h	1182			1725	149	
Approach Delay, s/veh	8.4			4.1	76.9	
Approach LOS	Α			A	70.7 E	
• •						
Timer	1	2	3	4	5	6
Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc),	\$3.6	115.5		20.9		129.1
Change Period (Y+Rc),		5.5		7.5		5.5
Max Green Setting (Gma		89.5		22.5		114.5
Max Q Clear Time (q_c+		21.7		13.2		2.0
Green Ext Time (p_c), s	•	45.3		0.3		60.3
	J. 1	. 5.0		3.0		33.0
Intersection Summary						
HCM 2010 Ctrl Delay			9.3			
HCM 2010 LOS			Α			
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		→	*	₹			7	ı	7	_	*	•
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	- 7	- ሽ		7	ሻሻ		7	77		7
Traffic Volume (veh/h)	108	879	243	65	1286	71	308	33	19	75	35	37
Future Volume (veh/h)	108	879	243	65	1286	71	308	33	19	75	35	37
Number	5	2	12	1	6	16	7	4	14	3	8	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
,	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	115	935	259	69	1368	76	328	35	20	80	37	39
Adj No. of Lanes	1	2	1	1	2	1	2	1	1	2	1	1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	254	2216	991	391	2178	974	376	200	170	121	74	63
Arrive On Green	0.08	1.00	1.00	0.03	0.62	0.62	0.11	0.11	0.11	0.04	0.04	0.04
Sat Flow, veh/h	1774	3539	1583	1774	3539	1583	3442	1863	1583	3442	1863	1583
Grp Volume(v), veh/h	115	935	259	69	1368	76	328	35	20	80	37	39
Grp Sat Flow(s), veh/h/ln		1770	1583	1774	1770	1583	1721	1863	1583	1721	1863	1583
Q Serve(g_s), s	3.7	0.0	0.0	2.1	36.3	2.9	14.1	2.6	1.7	3.4	2.9	3.6
Cycle Q Clear(g_c), s	3.7	0.0	0.0	2.1	36.3	2.9	14.1	2.6	1.7	3.4	2.9	3.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h		2216	991	391	2178	974	376	200	170	121	74	63
	0.45	0.42	0.26	0.18	0.63	0.08	0.87	0.18	0.12	0.66	0.50	0.62
Avail Cap(c_a), veh/h	363	2216	991	424	2178	974	424	236	201	172	112	95
	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	0.89	0.89	0.89	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh		0.0	0.0	9.7	18.1	11.7	65.8	60.9	60.5	71.5	70.5	70.9
Incr Delay (d2), s/veh	1.1	0.5	0.6	0.2	1.4	0.2	16.5	0.4	0.3	6.0	5.0	9.3
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh		0.2	0.2	1.1	18.1	1.3	7.6	1.3	0.8	1.7	1.6	1.8
LnGrp Delay(d),s/veh	16.4	0.5	0.6	9.9	19.5	11.8	82.3	61.3	60.8	77.5	75.6	80.2
LnGrp LOS	В	Α	Α	Α	В	В	62.5 F	E	E	77.5 E	75.0 E	F
Approach Vol, veh/h		1309	- / \	- / \	1513		<u>'</u>	383			156	<u>'</u>
Approach Delay, s/veh		1.9			18.6			79.3			77.7	
Approach LOS		1.9 A			10.0			79.3 E			77.7 E	
					D						L	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc),	\$2.2	102.9	12.8	22.1	13.8	101.3	22.9	12.0				
Change Period (Y+Rc), s	s 7.5	9.0	7.5	6.0	8.0	9.0	6.5	6.0				
Max Green Setting (Gma	ax) , 5	86.0	7.5	19.0	15.0	78.0	18.5	9.0				
Max Q Clear Time (g_c+		2.0	5.4	4.6	5.7	38.3	16.1	5.6				
Green Ext Time (p_c), s		40.3	0.0	0.4	0.2	26.8	0.3	0.1				
Intersection Summary												
HCM 2010 Ctrl Delay			21.8									
HCM 2010 LOS			C C									
110/01/2010 200			U									

Intersection							
Int Delay, s/veh	8.6						
		EDD	MDI	WDT	NDI	NDD	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	}	4.0	F	4	<u>ነ</u>	7	
Traffic Vol, veh/h	29	12	527	32	10	279	
Future Vol, veh/h	29	12	527	32	10	279	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	170	0	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	31	13	555	34	11	294	
	lajor1		Major2		Minor1		
Conflicting Flow All	0	0	44	0	1182	38	
Stage 1	-	-	-	-	38	-	
Stage 2	-	-	-	-	1144	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1564	-	210	1034	
Stage 1	_	-	-	-	984	-	
Stage 2	_	_	_	_	304	_	
Platoon blocked, %	_	_		_	001		
Mov Cap-1 Maneuver	_	_	1564	-	134	1034	
Mov Cap-1 Maneuver	_		1304	_	134	1034	
Stage 1	-	-	-	_	984	-	
· ·	-	-	-	-			
Stage 2	-	-	-	-	194	-	
Approach	EB		WB		NB		
HCM Control Delay, s	0		8.1		10.7		
HCM LOS					В		
Minor Lane/Major Mvmt	<u> </u>	NBLn11		EBT	EBR	WBL	
Capacity (veh/h)		134	1034	-	-	1564	
HCM Lane V/C Ratio		0.079	0.284	-	-	0.355	
HCM Control Delay (s)		34.1	9.9	-	-	8.6	
HCM Lane LOS		D	Α	-	-	Α	
HCM 95th %tile Q(veh)		0.3	1.2	-	-	1.6	

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EDL			WDK		SDK
Lane Configurations	EΩ	€	})E	72	24
Traffic Vol, veh/h	50	256	523	25	23	36
Future Vol, veh/h	50	256	523	25	23	36
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	53	272	556	27	24	38
Major/Minor	Major1	N	Majora	N	/linor?	
	Major1		Major2		Minor2	F70
Conflicting Flow All	583	0	-	0	948	570
Stage 1	-	-	-	-	570	-
Stage 2	-	-	-	-	378	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	991	-	-	-	289	521
Stage 1	-	-	-	-	566	-
Stage 2	-	-	-	-	693	-
Platoon blocked, %		_	-	_		
Mov Cap-1 Maneuver	991	-	_	_	271	521
Mov Cap-2 Maneuver	-	_	_	_	271	-
Stage 1	_	_		_	530	_
					693	
Ctano 2					073	-
Stage 2	-	-	-			
Stage 2	-	-	-			
Stage 2 Approach	EB	-	WB		SB	
	EB 1.4		WB 0		SB 16.2	
Approach		-				
Approach HCM Control Delay, s					16.2	
Approach HCM Control Delay, s HCM LOS	1.4	-	0	WOT	16.2 C	ODI 4
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm	1.4	EBL		WBT	16.2	
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	1.4	991	0	WBT_	16.2 C WBR :	383
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	1.4	991 0.054	0 EBT -		16.2 C WBR :	383 0.164
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	1.4	991 0.054 8.8	0 EBT - - 0	-	16.2 C WBR :	383 0.164 16.2
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS	1.4	991 0.054 8.8 A	0 EBT -	-	16.2 C WBR :	383 0.164 16.2 C
Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	1.4	991 0.054 8.8	0 EBT - - 0	- - -	16.2 C WBR :	383 0.164 16.2

Intersection						
Int Delay, s/veh	3.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ħ			4	ሻ	7
Traffic Vol, veh/h	217	62	36	521	98	53
Future Vol, veh/h	217	62	36	521	98	53
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-		- -	None
Storage Length	_	-	_	-	100	0
Veh in Median Storage,	# 0		_	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	228	65	38	548	103	56
Major/Minor M	lajor1	1	Major2	ľ	Minor1	
Conflicting Flow All	0	0	293	0	885	261
Stage 1	-	-	-	-	261	-
Stage 2	_	_	_	_	624	_
Critical Hdwy	_	_	4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	_	1.12	_	5.42	0.22
Critical Hdwy Stg 2		_	_	-	5.42	_
Follow-up Hdwy	-	_	2.218		3.518	
		-	1269		315	778
Pot Cap-1 Maneuver	-	-	1209	-		
Stage 1	-	-	-	-	783	-
Stage 2	-	-	-	-	534	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1269	-	301	778
Mov Cap-2 Maneuver	-	-	-	-	301	-
Stage 1	-	-	-	-	783	-
Stage 2	-	-	-	-	511	-
Annroach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		0.5		18.5	
HCM LOS					С	
Minor Lane/Major Mvmt	1	NBLn11	NBLn2	EBT	EBR	WBL
Capacity (veh/h)		301	778	_		1269
HCM Lane V/C Ratio		0.343		_	_	0.03
HCM Control Delay (s)		23.1	10	-	-	7.9
HCM Lane LOS		C	В	_	_	Α
HCM 95th %tile Q(veh)		1.5	0.2		_	0.1
HOW FOR FORME CELEBRATE		1.J	0.2			0.1

Intersection												
Int Delay, s/veh	7.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		ሻ	f)		ሻ	(î			4	
Traffic Vol, veh/h	0	0	0	66	0	2	0	1	30	7	4	0
Future Vol, veh/h	0	0	0	66	0	2	0	1	30	7	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	90	-	-	100	-	-	-	-	-
Veh in Median Storage	:,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	81	0	2	0	1	37	9	5	0
Major/Minor N	Major1		N	Major2		1	Minor1		N	Minor2		
Conflicting Flow All	2	0	0	1	0	0	167	165	1	183	164	1
Stage 1	-	-	-	-	-	-	1	1	-	163	163	-
Stage 2	-	-	-	-	-	-	166	164	-	20	1	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018		3.518	4.018	3.318
Pot Cap-1 Maneuver	1620	-	-	1622	-	-	797	728	1084	778	729	1084
Stage 1	-	-	-	-	-	-	1022	895	-	839	763	-
Stage 2	-	-	-	-	-	-	836	762	-	999	895	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1620	-	-	1622	-	-	763	692	1084	722	693	1084
Mov Cap-2 Maneuver	-	-	-	-	-	-	763	692	-	722	693	-
Stage 1	-	-	-	-	-	-	1022	895	-	839	725	-
Stage 2	-	-	-	-	-	-	789	724	-	964	895	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			7.1			8.5			10.2		
HCM LOS							Α			В		
Minor Lane/Major Mvm	ıt N	NBLn1 N	VBI n2	EBL	EBT	EBR	WBL	WBT	WBR S	SBI n1		
Capacity (veh/h)			1065	1620	-	-	1622	-	-	711		
HCM Lane V/C Ratio			0.036	-	_	_	0.05	_		0.019		
HCM Control Delay (s)		0	8.5	0	_	_	7.3	_	-	10.2		
HCM Lane LOS		A	Α	A	_	_	7.5 A	_	_	В		
HCM 95th %tile Q(veh)		-	0.1	0	-	-	0.2	-	-	0.1		
										J. 1		

APPENDIX E CAPACITY ANALYSIS WORKSHEETS BACKGROUND TRAFFIC

Lane Configurations 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 2 1 1 2 2 2 3 7 4 1 2 2 3 3 8 18 1 6 16 5 6 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 6 1 1 0 1 0	SBT SBR 1489 27 1489 27 1489 27 2 12 0 0 1.00 .00 1.00
Traffic Volume (veh/h) 22 23 70 41 23 203 39 600 27 642 1 Future Volume (veh/h) 22 23 70 41 23 203 39 600 27 642 1 Number 7 4 14 3 8 18 1 6 16 5 Initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1.00 1.00 1.00<	489 27 489 27 2 12 0 0 1.00
Future Volume (veh/h) 22 23 70 41 23 203 39 600 27 642 1 Number 7 4 14 3 8 18 1 6 16 5 Initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1.00 1.00 1.00 <t< td=""><td>189 27 2 12 0 0 1.00</td></t<>	189 27 2 12 0 0 1.00
Number 7 4 14 3 8 18 1 6 16 5 Initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 <td< td=""><td>2 12 0 0 1.00</td></td<>	2 12 0 0 1.00
Initial Q (Qb), veh 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 <t< td=""><td>0 0 1.00</td></t<>	0 0 1.00
Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	1.00
Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	
Adj Sat Flow, veh/h/ln 1863 1863 1863 1863 1863 1863 1863 1863	.00 1.00
,	
Adj Flow Rate, veh/h 25 26 79 46 26 24 44 674 30 721 1	363 1863
	573 30
Adj No. of Lanes 2 1 1 2 1 1 2 1 1	2 1
	.89 0.89
Percent Heavy Veh, % 2 2 2 2 2 2 2 2 2 2	2 2
	291 1025
	.65 0.65
	539 1583
	573 30
Grp Sat Flow(s), veh/h/ln 1721 1863 1583 1721 1863 1583 1774 1770 1583 1774 1	770 1583
	1.1 0.9
	1.1 0.9
Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00	1.00
Lane Grp Cap(c), veh/h 79 126 107 107 142 120 205 2195 982 617 2	291 1025
V/C Ratio(X) 0.32 0.21 0.74 0.43 0.18 0.20 0.22 0.31 0.03 1.17 (.73 0.03
Avail Cap(c_a), veh/h 212 272 231 212 272 231 253 2195 982 617 2	291 1025
	.00 1.00
	.00 1.00
, , ,	5.3 8.2
Incr Delay (d2), s/veh 1.7 1.0 11.2 2.0 0.7 1.0 0.4 0.4 0.1 92.5	2.1 0.1
Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0
	0.6 0.4
	7.4 8.3
LnGrp LOS E E E E B A A F	B A
Approach Vol, veh/h 130 96 748 2	124
Approach Delay, s/veh 66.9 60.4 1.2	5.9
Approach LOS E E A	D
Timer 1 2 3 4 5 6 7 8	
Assigned Phs 1 2 3 4 5 6 7 8	
Phs Duration (G+Y+Rc), s 11.5 92.7 11.1 14.8 15.0 89.1 10.0 15.9	
Change Period (Y+Rc), s 7.5 8.5 7.0 6.0 7.5 8.5 7.0 6.0	
Max Green Setting (Gmax), s 7.5 66.5 8.0 19.0 7.5 66.5 8.0 19.0	
Max Q Clear Time (g_c+l1), s 3.1 43.1 3.7 8.4 9.5 2.0 2.9 3.8	
Green Ext Time (p_c), s 0.0 22.8 0.0 0.5 0.0 60.2 0.0 0.6	
Intersection Summary	
HCM 2010 Ctrl Delay 37.2	
HCM 2010 LOS D	

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	*	-7	ı	*	•
Movement EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations 77	7	44	^	^	7
Traffic Volume (veh/h) 25	11	41	595	1558	30
Future Volume (veh/h) 25	11	41	595	1558	30
Number 7	14	1	6	2	12
Initial Q (Qb), veh 0	0	0	0	0	0
Ped-Bike Adj(A_pbT) 1.00	1.00	1.00			1.00
Parking Bus, Adj 1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln 1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h 29	13	48	692	1812	35
Adj No. of Lanes 2	1	2	2	2	1
Peak Hour Factor 0.86	0.86	0.86	0.86	0.86	0.86
Percent Heavy Veh, % 2	2	2	2	2	2
Cap, veh/h 103	48	109	2984	2640	1181
Arrive On Green 0.03	0.03	0.06	1.00	1.00	1.00
Sat Flow, veh/h 3442	1583	3442	3632	3632	1583
Grp Volume(v), veh/h 29	13	48	692	1812	35
Grp Sat Flow(s), veh/h/ln1721	1583	1721	1770	1770	1583
Q Serve(g_s), s 1.1	1.0	1.7	0.0	0.0	0.0
Cycle Q Clear(g_c), s 1.1	1.0	1.7	0.0	0.0	0.0
Prop In Lane 1.00	1.00	1.00			1.00
Lane Grp Cap(c), veh/h 103	48	109	2984	2640	1181
V/C Ratio(X) 0.28	0.27	0.44	0.23	0.69	0.03
Avail Cap(c_a), veh/h 344	158	304	2984	2640	1181
HCM Platoon Ratio 1.00	1.00	2.00	2.00	2.00	2.00
Upstream Filter(I) 1.00	1.00	0.98	0.98	0.65	0.65
Uniform Delay (d), s/veh 61.7	61.7	59.8	0.0	0.0	0.0
Incr Delay (d2), s/veh 1.5	3.1	2.7	0.2	1.0	0.0
Initial Q Delay(d3),s/veh 0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/lr0.5	0.5	0.9	0.0	0.4	0.0
LnGrp Delay(d),s/veh 63.1	64.7	62.5	0.1	1.0	0.0
	04.7 E	62.5 E			
		<u> </u>	740	A	A
Approach Vol, veh/h 42			740	1847	
Approach Delay, s/veh 63.6			4.2	1.0	
Approach LOS E			Α	Α	
Timer 1	2	3	4	5	6
Assigned Phs 1	2		4		6
Phs Duration (G+Y+Rc), \$2.6			10.9		119.1
Change Period (Y+Rc), s 8.5	9.5		7.0		9.5
Max Green Setting (Gmax), 5			13.0		100.5
Max Q Clear Time (g_c+l13), 78			3.1		2.0
Green Ext Time (p_c), s 0.0	43.0		0.0		47.7
Intersection Summary					
HCM 2010 Ctrl Delay		2.9			
HCM 2010 LOS		A			
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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻሻ	7	ሻሻ	^		7	
Traffic Volume (veh/h)	12	6	42	612	1486	60	
Future Volume (veh/h)	12	6	42	612	1486	60	
Number	7	14	1	6	2	12	
Initial Q (Qb), veh	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	
	1863	1863	1863	1863	1863	1863	
Adj Flow Rate, veh/h	14	7	50	729	1769	71	
Adj No. of Lanes	2	1	2	2	2	1	
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84	
Percent Heavy Veh, %	2	2	2	2	2	2	
Cap, veh/h	70	32	111	3113	2768	1238	
Arrive On Green	0.02	0.02	0.03	0.88	1.00	1.00	
	3442	1583	3442	3632	3632	1583	
	14	7				71	ĺ
Grp Volume(v), veh/h			50	729	1769		
Grp Sat Flow(s), veh/h/lr		1583	1721	1770	1770	1583	
Q Serve(g_s), s	0.5	0.6	1.9	4.1	0.0	0.0	
Cycle Q Clear(g_c), s	0.5	0.6	1.9	4.1	0.0	0.0	
Prop In Lane	1.00	1.00	1.00			1.00	
Lane Grp Cap(c), veh/h		32	111	3113	2768	1238	
V/C Ratio(X)	0.20	0.22	0.45	0.23	0.64	0.06	
Avail Cap(c_a), veh/h	344	158	304	3113	2768	1238	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.33	1.33	
Upstream Filter(I)	1.00	1.00	0.98	0.98	0.71	0.71	
Uniform Delay (d), s/veh	162.6	62.6	61.8	1.2	0.0	0.0	
Incr Delay (d2), s/veh	1.4	3.3	2.8	0.2	0.8	0.1	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh		0.3	0.9	2.0	0.3	0.0	
LnGrp Delay(d),s/veh	64.0	65.9	64.6	1.4	0.8	0.1	
LnGrp LOS	E	E	E	A	Α	A	
	21	<u> </u>	<u> </u>		1840		
Approach Vol, veh/h				779			
Approach Delay, s/veh				5.4	8.0		
Approach LOS	E			Α	Α		
Timer	1	2	3	4	5	6	
Assigned Phs	1	2		4		6	
Phs Duration (G+Y+Rc)				9.7		120.3	
Change Period (Y+Rc),		6.0		7.0		6.0	
Max Green Setting (Gm		84.0		13.0		104.0	
Max Q Clear Time (g_c-		2.0		2.6		6.1	
Green Ext Time (p_c), s		43.9		0.0		47.5	
	0.0	43.7		0.0		47.5	
Intersection Summary							
HCM 2010 Ctrl Delay			2.7				
HCM 2010 LOS			Α				
= = 0 0							

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Movement EB	EBR	BT_	R WBL	WBT	NBL	NBR
Lane Configurations 🛧					ሻሻ	7
Traffic Volume (veh/h) 147					0	11
Future Volume (veh/h) 147					0	11
, ,		2			7	14
		0			0	0
Ped-Bike Adj(A_pbT)	1.00	U			1.00	1.00
Parking Bus, Adj 1.0		ΛΛ			1.00	1.00
Adj Sat Flow, veh/h/ln 186					1863	1863
Adj Flow Rate, veh/h 179					0	13
,		2			2	1
Peak Hour Factor 0.8					0.82	0.82
J ,		2			2	2
Cap, veh/h 280	1255	04	5 109	3134	50	23
Arrive On Green 0.7	0.79	79	0.06	1.00	0.00	0.01
Sat Flow, veh/h 363	1583	32	3442	3632	3442	1583
Grp Volume(v), veh/h 179					0	13
Grp Sat Flow(s), veh/h/ln177				1770	1721	1583
Q Serve(g_s), s 27.				0.0	0.0	1.1
Cycle Q Clear(g_c), s 27.				0.0	0.0	1.1
Prop In Lane	1.00	. /			1.00	1.00
Lane Grp Cap(c), veh/h 280		Λ4			50	23
						0.57
V/C Ratio(X) 0.6					0.00	
Avail Cap(c_a), veh/h 280					331	152
HCM Platoon Ratio 1.0					1.00	1.00
Upstream Filter(I) 0.7					0.00	1.00
Uniform Delay (d), s/veh 5.		5.7			0.0	63.7
Incr Delay (d2), s/veh 0.	9 0.0).9	2.7	0.2	0.0	20.4
Initial Q Delay(d3),s/veh 0.	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln3.	5 0.1	3.6	0.9	0.1	0.0	0.6
` ,		5.6			0.0	84.1
		Α				F
Approach Vol, veh/h 181			<u> </u>	843	13	•
• •				3.7	84.1	
11		5.5				
Approach LOS	1	Α		А	F	
Timer	1 2	1	2 3	4	5	6
Assigned Phs	1 2	1	2	4		6
Phs Duration (G+Y+Rc), \$2.	108.5	2.1 1	5	9.4		120.6
Change Period (Y+Rc), s 8.				7.5		5.5
Max Green Setting (Gmax),				12.5		104.5
Max Q Clear Time (g_c+l13),				3.1		2.0
Green Ext Time (p_c), s 0.				0.0		51.1
Green Ext Time (P_C), S U.	J 30.3	J.U)	0.0		1.10
Intersection Summary						
HCM 2010 Ctrl Delay			6.0			
HCM 2010 LOS			A			
110/11/2010 200						

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	*	^	7	14.14	•	7	1,1	•	7
Traffic Volume (veh/h)	38	1295	154	39	521	14	116	16	37	58	26	60
Future Volume (veh/h)	38	1295	154	39	521	14	116	16	37	58	26	60
Number	5	2	12	1	6	16	7	4	14	3	8	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	45	1542	183	46	620	17	138	19	44	69	31	71
Adj No. of Lanes	1	2	1	1	2	1	2	1	1	2	1	1
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	541	2228	997	286	2215	991	189	137	116	121	115	97
Arrive On Green	0.06	1.00	1.00	0.03	0.63	0.63	0.05	0.07	0.07	0.04	0.06	0.06
Sat Flow, veh/h	1774	3539	1583	1774	3539	1583	3442	1863	1583	3442	1863	1583
Grp Volume(v), veh/h	45	1542	183	46	620	17	138	19	44	69	31	71
Grp Sat Flow(s), veh/h/li		1770	1583	1774	1770	1583	1721	1863	1583	1721	1863	1583
Q Serve(g_s), s	1.2	0.0	0.0	1.2	10.3	0.5	5.1	1.2	3.4	2.6	2.1	5.7
Cycle Q Clear(g_c), s	1.2	0.0	0.0	1.2	10.3	0.5	5.1	1.2	3.4	2.6	2.1	5.7
Prop In Lane	1.00	0.0	1.00	1.00	10.5	1.00	1.00	1.2	1.00	1.00	۷.۱	1.00
Lane Grp Cap(c), veh/h		2228	997	286	2215	991	189	137	116	121	115	97
V/C Ratio(X)	0.08	0.69	0.18	0.16	0.28	0.02	0.73	0.14	0.38	0.57	0.27	0.73
Avail Cap(c_a), veh/h	581	2228	997	333	2215	991	225	201	171	199	201	171
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.76	0.76	0.76	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/vel		0.76	0.76	7.9	11.0	9.2	60.5	56.4	57.4	61.7	58.2	59.9
3	0.0	1.4	0.0	0.3	0.3	0.0	9.4	0.5	2.0	4.1	1.3	9.9
Incr Delay (d2), s/veh		0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Initial Q Delay(d3),s/veh		0.0	0.0	0.6	5.1	0.0	2.7	0.0	1.6	1.3	1.1	2.8
%ile BackOfQ(50%),vel		1.4	0.1	8.1		9.2			59.4	65.9	59.5	
LnGrp Delay(d),s/veh	8.1				11.3		69.9	56.8				69.9 E
LnGrp LOS	A	A	A	A	B	A	<u>E</u>	E 201	<u>E</u>	<u>E</u>	E 171	<u>E</u>
Approach Vol, veh/h		1770			683			201			171	
Approach Delay, s/veh		1.4			11.1			66.4			66.4	
Approach LOS		Α			В			Е			Е	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc)	-	90.8	12.1	15.5	12.0	90.4	13.6	14.0				
Change Period (Y+Rc),		9.0	7.5	6.0	8.0	9.0	6.5	6.0				
Max Green Setting (Gm		71.0	7.5	14.0	7.0	71.0	8.5	14.0				
Max Q Clear Time (q_c		2.0	4.6	5.4	3.2	12.3	7.1	7.7				
Green Ext Time (p_c), s	•	32.6	0.0	0.3	0.0	30.4	0.0	0.3				
	. 5.0	52.0	3.0	3.0	5.5	33.1	3.0	3.0				
Intersection Summary			10.0									
HCM 2010 Ctrl Delay			12.3									
HCM 2010 LOS			В									

0.1

HCM 95th %tile Q(veh)

5.1

Intersection						
Int Delay, s/veh	11.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		LDK	WDL			
Lane Configurations	}	1	242	4	<u>ነ</u>	آ 540
Traffic Vol. veh/h	33 33	4	262 262	30	11	568 568
Future Vol, veh/h		4			11	
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	-	-	170	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	5	312	36	13	676
Major/Minor M	lajor1	N	Major2		Minor1	
						42
Conflicting Flow All	0	0	44	0	702	42
Stage 1	-	-	-	-	42	-
Stage 2	-	-	-	-	660	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1564	-	404	1029
Stage 1	-	-	-	-	980	-
Stage 2	-	-	-	-	514	-
Platoon blocked, %	_	-		-		
Mov Cap-1 Maneuver	-	-	1564	-	322	1029
Mov Cap-2 Maneuver	_	_	-	_	322	-
Stage 1	_		_	_	980	_
Stage 2	-				409	-
Staye 2	-	-	-	-	407	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		7.1		14.9	
HCM LOS					В	
Minor Long/Major Muset		UDI1 M	VIDI ~2	EDT	EDD	WDI
Minor Lane/Major Mvmt	. ' '	VBLn1 I		EBT	EBR	WBL
Capacity (veh/h)			1029	-		1564
HCM Lane V/C Ratio		0.041		-	-	0.199
HCM Control Delay (s)		16.7		-	-	,.,
HCM Lane LOS		С	В	-	-	Α
LIONA OF IL COME OF THE						

0.7

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	₩ ^	אטוע	ÿ.	אומט
Traffic Vol, veh/h	21	581	251	15	26	38
Future Vol, veh/h	21	581	251	15	26	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		310p -	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage	- - # -	0	0	-	0	
Grade, %	- :	0	0	-	0	-
Peak Hour Factor	84	84	84	84	84	84
		2	2			
Heavy Vehicles, %	2			2	2	2
Mvmt Flow	25	692	299	18	31	45
Major/Minor 1	Wajor1	N	Najor2	N	Minor2	
Conflicting Flow All	317	0		0	1050	308
Stage 1	-	-	-	-	308	-
Stage 2	_	_	-	_	742	_
Critical Hdwy	4.12	-	-	_	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_		3.518	3 318
Pot Cap-1 Maneuver	1243	_	_	_	252	732
Stage 1	1243	_	_	_	745	- 132
Stage 2	_		-	_	471	-
Platoon blocked, %	-	-	-	-	4/1	-
	12/2	-	-		244	732
Mov Cap-1 Maneuver		-	-	-		
Mov Cap-2 Maneuver	-	-	-	-	244	-
Stage 1	-	-	-	-	720	-
Stage 2	-	-	-	-	471	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		16	
HCM LOS	0.0				С	
TIOW EOO						
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	
Capacity (veh/h)		1243	-	-	-	404
HCM Lane V/C Ratio		0.02	-	-	-	0.189
HCM Control Delay (s)		8	0	-	-	16
HCM Lane LOS		Α	Α	-	-	С
HCM 95th %tile Q(veh))	0.1	-	-	-	0.7

Intersection						
Int Delay, s/veh	1.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK	WDL			
Lane Configurations	}	າາ	าา	4	\	7
Traffic Vol, veh/h	588	23	22	224	38	55
Future Vol, veh/h	588	23	22	224	38	55
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	100	0
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	676	26	25	257	44	63
		_		-		
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	702	0	996	689
Stage 1	-	-	-	-	689	-
Stage 2	-	-	-	-	307	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	895	-	271	446
Stage 1	-	-	-	-	498	_
Stage 2	_	_	_	-	746	_
Platoon blocked, %	_			_	740	
	-		895		262	446
Mov Cap 2 Manager	-	-	073	-		
Mov Cap-2 Maneuver	-	-	-	-	262	-
Stage 1	-	-	-	-	498	-
Stage 2	-	-	-	-	721	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		17.3	
HCM LOS	U		0.0		17.3	
TIOWI LOG					U	
Minor Lane/Major Mvm	nt N	NBLn11	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		262	446	-	-	895
HCM Lane V/C Ratio		0.167		-	-	0.028
HCM Control Delay (s)		21.5	14.4	_	_	9.1
HCM Lane LOS		C	В	_	_	A
HCM 95th %tile Q(veh))	0.6	0.5	_		0.1
HOW FOUT WITH CIVELL)	0.0	0.3	-	-	U. I

HCM 95th %tile Q(veh)

0.2

Intersection												
Int Delay, s/veh	7.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		ኘ	\$		ሻ	1			4	
Traffic Vol, veh/h	0	0	0	22	0	7	0	2	48	0	4	0
Future Vol, veh/h	0	0	0	22	0	7	0	2	48	0	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	_	-	90	_	-	100	_	-	_	_	-
Veh in Median Storage		0	_	-	0	_	-	0	_	_	0	_
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	25	0	8	0	2	54	0	4	0
WWW. Tiow	Ū	Ū		20		Ū		_	01	J	•	Ū
Major/Minor I	Major1			Major2		N	Minor1		ı	Minor2		
		0			0		57	59			CC	1
Conflicting Flow All	8	0	0	1	0	0			1	83	55 E.4	4
Stage 1	-	-	-	-	-	-	1	1	-	54	54	-
Stage 2	-	-	-	110	-	-	56	58	-	29	1	/ 22
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	2 210	-	-	2 210	-	-	6.12	5.52	2 210	6.12	5.52	2 210
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1612	-	-	1622	-	-	940	832	1084	904	836	1080
Stage 1	-	-	-	-	-	-	1022	895	-	958	850	-
Stage 2	-	-	-	-	-	-	956	847	-	988	895	-
Platoon blocked, %	1/17	-	-	1422	-	-	025	020	1004	0.47	ດລວ	1000
Mov Cap-1 Maneuver	1612	-	-	1622	-	-	925	820	1084	847	823	1080
Mov Cap-2 Maneuver	-	-	-	-	-	-	925	820	-	847	823	-
Stage 1	-	-	-	-	-	-	1022	895	-	958	837	-
Stage 2	-	-	-	-	-	-	936	834	-	936	895	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			5.5			8.6			9.4		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1 N	VBLn2	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1		
Capacity (veh/h)			1070	1612	-		1622	-	-	823		
HCM Lane V/C Ratio			0.053	-	-		0.015	-	-	0.005		
HCM Control Delay (s)		0	8.6	0	-	-	7.3	-	-	9.4		
HCM Lane LOS		A	А	A	-	-	A	-	-	Α		
11014 0511 0711 07 13			0.0									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1,1	†	7	ሻሻ	†	7	Ĭ	^	7	7	^	7
Traffic Volume (veh/h)	134	43	57	80	63	569	95	1708	69	374	1259	106
Future Volume (veh/h)	134	43	57	80	63	569	95	1708	69	374	1259	106
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	144	46	38	86	68	258	102	1837	46	402	1354	71
Adj No. of Lanes	2	1	1	2	1	1	1	2	1	1	2	1
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	189	232	197	128	199	169	251	1988	890	229	2149	962
Arrive On Green	0.05	0.12	0.12	0.04	0.11	0.11	0.05	0.75	0.75	0.08	0.61	0.61
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	1774	3539	1583	1774	3539	1583
Grp Volume(v), veh/h	144	46	38	86	68	258	102	1837	46	402	1354	71
Grp Sat Flow(s), veh/h/ln	1721	1863	1583	1721	1863	1583	1774	1770	1583	1774	1770	1583
Q Serve(g_s), s	6.2	3.3	3.2	3.7	5.1	16.0	3.7	63.5	1.1	12.5	36.5	2.8
Cycle Q Clear(g_c), s	6.2	3.3	3.2	3.7	5.1	16.0	3.7	63.5	1.1	12.5	36.5	2.8
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	189	232	197	128	199	169	251	1988	890	229	2149	962
V/C Ratio(X)	0.76	0.20	0.19	0.67	0.34	1.53	0.41	0.92	0.05	1.76	0.63	0.07
Avail Cap(c_a), veh/h	229	232	197	184	199	169	320	1988	890	229	2149	962
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.69	0.69	0.69	1.00	1.00	1.00
Uniform Delay (d), s/veh	69.9	59.0	58.9	71.3	62.1	67.0	16.6	16.3	8.5	46.6	18.7	12.1
Incr Delay (d2), s/veh	10.5	0.5	0.6	4.5	1.2	265.1	0.5	6.4	0.1	357.1	1.4	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.2	1.7	1.4	1.8	2.7	19.4	1.8	32.2	0.5	32.2	18.1	1.2
LnGrp Delay(d),s/veh	80.4	59.5	59.5	75.8	63.3	332.1	17.1	22.7	8.5	403.8	20.1	12.3
LnGrp LOS	F	Ε	Ε	Е	Ε	F	В	С	Α	F	С	В
Approach Vol, veh/h		228			412			1985			1827	
Approach Delay, s/veh		72.7			234.2			22.1			104.2	
Approach LOS		Е			F			С			F	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.2	99.6	12.6	24.7	20.0	92.8	15.2	22.0				
Change Period (Y+Rc), s	7.5	8.5	7.0	6.0	7.5	8.5	7.0	6.0				
Max Green Setting (Gmax), s	11.5	83.5	8.0	18.0	12.5	82.5	10.0	16.0				
Max Q Clear Time (g_c+l1), s	5.7	38.5	5.7	5.3	14.5	65.5	8.2	18.0				
Green Ext Time (p_c), s	0.1	44.7	0.0	1.7	0.0	16.9	0.1	0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			78.0									
HCM 2010 LOS			Е									

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•	•	*	1	Ť	¥	4
Movement EBL	EBR	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7		ሻሻ	^	^	7
Traffic Volume (veh/h) 205	102		161	1680	1172	209
Future Volume (veh/h) 205	102		161	1680	1172	209
Number 7	14		1	6	2	12
Initial Q (Qb), veh 0	0		0	0	0	0
` ,				U	U	
Ped-Bike Adj(A_pbT) 1.00	1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj 1.00	1.00		1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln 1863	1863		1863	1863	1863	1863
Adj Flow Rate, veh/h 228	113		179	1867	1302	232
Adj No. of Lanes 2	1	1	2	2	2	1
Peak Hour Factor 0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, % 2	2	2	2	2	2	2
Cap, veh/h 307	141	141	222	2834	2406	1076
Arrive On Green 0.09	0.09		0.13	1.00	1.00	1.00
Sat Flow, veh/h 3442	1583		3442	3632	3632	1583
Grp Volume(v), veh/h 228	113		179	1867	1302	232
	1583		1721	1770	1770	1583
Grp Sat Flow(s), veh/h/ln1721 Q Serve(q_s), s 9.7	10.5		7.6	0.0		0.0
10- /				0.0	0.0	
Cycle Q Clear(g_c), s 9.7	10.5		7.6	0.0	0.0	0.0
Prop In Lane 1.00	1.00		1.00	0004	0.407	1.00
Lane Grp Cap(c), veh/h 307	141		222	2834	2406	1076
V/C Ratio(X) 0.74	0.80		0.81	0.66	0.54	0.22
Avail Cap(c_a), veh/h 528	243		264	2834	2406	1076
HCM Platoon Ratio 1.00	1.00	1.00	2.00	2.00	2.00	2.00
Upstream Filter(I) 1.00	1.00	1.00	0.73	0.73	0.74	0.74
Uniform Delay (d), s/veh 66.6	67.0	67.0	64.4	0.0	0.0	0.0
Incr Delay (d2), s/veh 3.6	10.0		10.9	0.9	0.7	0.3
Initial Q Delay(d3),s/veh 0.0	0.0		0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln4.8	5.0		3.9	0.4	0.2	0.1
LnGrp Delay(d),s/veh 70.2	77.0		75.3	0.4	0.2	0.1
LnGrp LOS E	77.0 E		75.5 E	Α	Α	0.5 A
	E					А
Approach Vol, veh/h 341				2046	1534	
Approach Delay, s/veh 72.4				7.4	0.6	
Approach LOS E				Α	Α	
Timer 1	2	2	3	4	5	6
Assigned Phs 1	2			4		6
Phs Duration (G+Y+Rc), \$8.2				20.4		129.6
Change Period (Y+Rc), s 8.5	9.5			7.0		9.5
Max Green Setting (Gmax), 5	90.5			23.0		110.5
Max Q Clear Time (g_c+l19,6				12.5		2.0
Green Ext Time (p_c), s 0.1	67.5	67.5		0.9		78.2
Intersection Summary						
HCM 2010 Ctrl Delay			10.4			
HCM 2010 LOS			В			
HOW ZOTO LOG			U			

			-7	T	↓	4	
	EBL	EBR	NBL	NBT	SBT	SBR	
	ሻሻ	T T	T T	↑ ↑	↑ ↑	JDIK *	
	139	183	226	1722	TT	101	
	139	183	226	1722	1175		
` '						101	
Number	7	14	1	6	2	12	
Initial Q (Qb), veh	0	0	0	0	0	0	
3 · — 1	1.00	1.00	1.00			1.00	
,	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln 18	863	1863	1863	1863	1863	1863	
Adj Flow Rate, veh/h 1	153	201	248	1892	1291	111	
Adj No. of Lanes	2	1	2	2	2	1	
•).91	0.91	0.91	0.91	0.91	0.91	
Percent Heavy Veh, %	2	2	2	2	2	2	
3	483	222	298	2736	2229	997	
).14	0.14	0.17	1.00	1.00	1.00	
	442	1583	3442	3632	3632	1583	
. , , .	153	201	248	1892	1291	111	
Grp Sat Flow(s), veh/h/ln17		1583	1721	1770	1770	1583	
10- /-	6.0	18.8	10.4	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	6.0	18.8	10.4	0.0	0.0	0.0	
Prop In Lane 1.	1.00	1.00	1.00			1.00	
Lane Grp Cap(c), veh/h 4	483	222	298	2736	2229	997	
	0.32	0.90	0.83	0.69	0.58	0.11	
` ,	528	243	493	2736	2229	997	
	1.00	1.00	2.00	2.00	2.00	2.00	
	1.00	1.00	0.71	0.71	0.81	0.81	
		63.5	61.0	0.71	0.0	0.0	
Uniform Delay (d), s/veh 5							
J ()	0.4	32.1	4.4	1.0	0.9	0.2	
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/lr		10.2	5.1	0.4	0.3	0.1	
1 317	58.4	95.6	65.4	1.0	0.9	0.2	
LnGrp LOS	Ε	F	Е	Α	Α	Α	
Approach Vol, veh/h 3	354			2140	1402		
• •	79.5			8.5	0.8		
Approach LOS	E			A	A		
	_						
Timer	1	2	3	4	5	6	
Assigned Phs	1	2		4		6	
Phs Duration (G+Y+Rc), 3		100.5		28.0		122.0	
Change Period (Y+Rc), s	8.5	6.0		7.0		6.0	
Max Green Setting (Gmax)		84.0		23.0		114.0	
		2.0		20.8		2.0	
		62.7		0.3		78.4	
Max Q Clear Time (g_c+III	U.J			3.0			
Max Q Clear Time (g_c+fft Green Ext Time (p_c), s	0.5						
Max Q Clear Time (g_c+lft Green Ext Time (p_c), s Intersection Summary	0.5						
Max Q Clear Time (g_c+fft Green Ext Time (p_c), s	0.5		12.2 B				

	_		_	←	_	_
→	*	* 1	₹	•	1	1
Movement EBT	EBR		VBL	WBT	NBL	NBR
Lane Configurations * † †	7	7	J.J.	^	14.54	7
Traffic Volume (veh/h) 1350	16		98	1896	33	139
Future Volume (veh/h) 1350	16	16	98	1896	33	139
Number 2	12	12	1	6	7	14
Initial Q (Qb), veh 0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00		1.00	1.00
Parking Bus, Adj 1.00	1.00		1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln 1863	1863		863	1863	1863	1863
	17		104	2017	35	148
•						
Adj No. of Lanes 2	1		2	2	2	1
Peak Hour Factor 0.94	0.94).94	0.94	0.94	0.94
Percent Heavy Veh, % 2	2		2	2	2	2
Cap, veh/h 2511	1123	123 1	149	2853	369	170
Arrive On Green 0.71	0.71	0.71 0	0.09	1.00	0.11	0.11
Sat Flow, veh/h 3632	1583	583 34	442	3632	3442	1583
Grp Volume(v), veh/h 1436	17	17 1	104	2017	35	148
Grp Sat Flow(s), veh/h/ln1770	1583		721	1770	1721	1583
Q Serve(g_s), s 29.8	0.5		4.4	0.0	1.4	13.8
Cycle Q Clear(q_c), s 29.8	0.5		4.4	0.0	1.4	13.8
Prop In Lane	1.00		1.00	0.0	1.00	1.00
				30E3		
Lane Grp Cap(c), veh/h 2511	1123		149	2853	369	170
V/C Ratio(X) 0.57	0.02).70	0.71	0.09	0.87
Avail Cap(c_a), veh/h 2511	1123		390	2853	516	237
HCM Platoon Ratio 1.00	1.00		2.00	2.00	1.00	1.00
Upstream Filter(I) 0.81	0.81).41	0.41	1.00	1.00
Uniform Delay (d), s/veh 10.7	6.4	6.4 6	57.5	0.0	60.4	65.9
Incr Delay (d2), s/veh 0.8	0.0		2.4	0.6	0.1	21.4
Initial Q Delay(d3),s/veh 0.0	0.0		0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln4.6	0.2		2.1	0.2	0.7	7.1
LnGrp Delay(d),s/veh 11.4	6.4		70.0	0.6	60.5	87.4
1 3 1 7						
LnGrp LOS B	A	А	E	A 2121	E	<u> </u>
Approach Vol, veh/h 1453				2121	183	
Approach Delay, s/veh 11.4				4.0	82.2	
Approach LOS B				Α	F	
Timer 1	2	2	3	4	5	6
Assigned Phs 1	2			4		6
						126.4
Phs Duration (G+Y+Rc), \$4.5	111.9			23.6		
Change Period (Y+Rc), s 8.0	5.5			7.5		5.5
Max Green Setting (Gmax), &	89.5			22.5		114.5
Max Q Clear Time (g_c+l16,4s	31.8			15.8		2.0
Green Ext Time (p_c), s 0.2	50.2	0.2		0.3		86.9
Intersection Summary						
HCM 2010 Ctrl Delay		1	10.7			
HCM 2010 LOS		1	В			
HOW ZUTU LUS			D			

	<u> </u>		_	_	←	•	•	†	*	_	1	1
			T DD	▼ MD!	MOT)	I NOT	/ NDD	CDI	▼	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	7		^	7	ሻሻ		7	44	↑	7
Traffic Volume (veh/h)	133	1081	299	80	1582	87	379	41	23	92	43	46
Future Volume (veh/h)	133	1081	299	80	1582	87	379	41	23	92	43	46
Number	5	2	12	1	6	16	7	4	14	3	8	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
,	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	141	1150	318	85	1683	93	403	44	24	98	46	49
Adj No. of Lanes	1	2	1	1	2	1	2	1	1	2	1	1
	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	189	2146	960	317	2081	931	424	224	191	141	83	71
	0.09	1.00	1.00	0.03	0.59	0.59	0.12	0.12	0.12	0.04	0.04	0.04
·	1774	3539	1583	1774	3539	1583	3442	1863	1583	3442	1863	1583
Grp Volume(v), veh/h	141	1150	318	85	1683	93	403	44	24	98	46	49
Grp Sat Flow(s), veh/h/ln		1770	1583	1774	1770	1583	1721	1863	1583	1721	1863	1583
Q Serve(g_s), s	4.9	0.0	0.0	2.9	56.0	3.9	17.4	3.2	2.0	4.2	3.6	4.6
Cycle Q Clear(g_c), s	4.9	0.0	0.0	2.9	56.0	3.9	17.4	3.2	2.0	4.2	3.6	4.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	189	2146	960	317	2081	931	424	224	191	141	83	71
V/C Ratio(X)	0.75	0.54	0.33	0.27	0.81	0.10	0.95	0.20	0.13	0.70	0.55	0.69
Avail Cap(c_a), veh/h	283	2146	960	348	2081	931	424	236	201	172	112	95
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.81	0.81	0.81	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh		0.0	0.0	11.4	24.3	13.5	65.3	59.4	58.9	71.0	70.2	70.6
Incr Delay (d2), s/veh	4.7	8.0	0.7	0.4	3.5	0.2	31.0	0.4	0.3	8.9	5.6	12.6
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh		0.2	0.2	1.4	28.1	1.7	10.1	1.7	0.9	2.2	2.0	2.3
LnGrp Delay(d),s/veh	33.1	8.0	0.7	11.8	27.8	13.7	96.3	59.8	59.2	79.9	75.8	83.2
LnGrp LOS	С	Α	Α	В	С	В	F	Ε	Е	Ε	Ε	F
Approach Vol, veh/h		1609			1861			471			193	
Approach Delay, s/veh		3.6			26.4			91.0			79.8	
Approach LOS		Α			С			F			Ε	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc),		99.9	13.6	24.1	15.1	97.2	25.0	12.7				
Change Period (Y+Rc),		9.0	7.5	6.0	8.0	9.0	6.5	6.0				
Max Green Setting (Gma		86.0	7.5	19.0	15.0	78.0	18.5	9.0				
Max Q Clear Time (q_c+		2.0	6.2	5.2	6.9	58.0	19.4	6.6				
Green Ext Time (p_c), s	, .	58.3	0.0	0.5	0.2	18.0	0.0	0.1				
	0.0	00.0	3.0	3.0	J. <u>Z</u>	. 5.0	3.0	J. 1				
Intersection Summary			27.4									
HCM 2010 Ctrl Delay			27.4									
HCM 2010 LOS			С									

Intersection						
Int Delay, s/veh	9.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EBK	WBL			
Lane Configurations Traffic Vol, veh/h	}	10	648	4	ነ	242
· ·	36 36	15 15	648	39 39	12	343
Future Vol, veh/h		0			12	343
Conflicting Peds, #/hr	0		0	0	O Cton	O Cton
J	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	170	None
Storage Length		-	-	-	170	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	38	16	682	41	13	361
Major/Minor M	ajor1		Major2		Vinor1	
Conflicting Flow All	0	0	54	0	1451	46
Stage 1	-	-	-	-	46	-
Stage 2	_	_	_	_	1405	_
Critical Hdwy	_	_	4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	_	7.12	_	5.42	0.22
Critical Hdwy Stg 2	_		_	_	5.42	_
Follow-up Hdwy	_	_	2.218			3.318
Pot Cap-1 Maneuver	-	_	1551	-	144	1023
Stage 1	_	_	1001	_	976	1023
Stage 2	-		-	-	227	_
Platoon blocked, %	-	-	-	-	221	-
		-	1551		70	1023
Mov Cap-1 Maneuver	-	-	1001	-	79	
Mov Cap-2 Maneuver	-	-	-	-	79	-
Stage 1	-	-	-	-	976	-
Stage 2	-	-	-	-	125	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		8.6		12	
HCM LOS			0.0		В	
Minor Long/Major Mymt	N	JDI 51 N	מ וחוי	ГОТ	EDD	WDI
Minor Lane/Major Mvmt	<u> </u>	VBLn1 I		EBT	EBR	WBL
Capacity (veh/h)			1023	-		1551
HCM Lane V/C Ratio			0.353	-	-	0.44
HCM Control Delay (s)		59.1	10.4	-	-	9.1
HCM Lane LOS		F	В	-	-	A
HCM 95th %tile Q(veh)		0.5	1.6	-	-	2.3

Intersection						
Int Delay, s/veh	1.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	LDIN	VVDL	જા	NDL 7	T T
Traffic Vol, veh/h	267	76	44	641	28	37
Future Vol, veh/h	267	76	44	641	28	37
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-			None	- -	None
Storage Length	_	-	_	-	100	0
Veh in Median Storage,	# 0	_	_	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	281	80	46	675	29	39
IVIVIIIL I IOW	201	00	40	0/3	21	37
Major/Minor N	1ajor1	1	Major2		Minor1	
Conflicting Flow All	0	0	361	0	1088	321
Stage 1	-	-	-	-	321	-
Stage 2	-	-	-	-	767	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1198	-	239	720
Stage 1	-	-	-	-	735	-
Stage 2	-	-	-	-	458	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1198	-	224	720
Mov Cap-2 Maneuver	-	-	-	-	224	-
Stage 1	-	-	-	-	735	-
Stage 2	_	-		-	430	_
otago 2						
			14/5		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.5		16	
HCM LOS					С	
Minor Lane/Major Mvmt	1	NBLn11	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		224	720			1198
HCM Lane V/C Ratio		0.132		_		0.039
HCM Control Delay (s)		23.5	10.3	_	_	8.1
HCM Lane LOS		23.3 C	В	_	_	Α
HCM 95th %tile Q(veh)		0.4	0.2		_	0.1
1101VI 73111 70111E Q(VEII)		0.4	0.2	-	-	U. I

2027 Background Traffic - PM Peak Ho	ur
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Intersection												
Int Delay, s/veh	7.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ĵ.		*	ĵ.		*	f)			4	
Traffic Vol, veh/h	0	0	0	81	0	2	0	1	37	9	5	0
Future Vol, veh/h	0	0	0	81	0	2	0	1	37	9	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	_	-	90	_	-	100	-	-	_	-	-
Veh in Median Storage		0	_	-	0	_	-	0	_	_	0	_
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	100	0	2	0	1	46	11	6	0
Major/Minor N	Major1		ı	Major2		1	Minor1		1	Minor2		
Conflicting Flow All	2	0	0	1	0	0	205	203	1	226	202	1
Stage 1	-	-	-	_	-	-	1	1	-	201	201	-
Stage 2	_	_	_	_	_	_	204	202	_	25	1	_
Critical Hdwy	4.12	_		4.12	_		7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	7.12	_	_	7.12	_	_	6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2	_	_	_	_	_	_	6.12	5.52	_	6.12	5.52	_
Follow-up Hdwy	2.218	_	_	2.218	_	_	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1620	_	_	1622	_	_	753	693	1084	729	694	1084
Stage 1	- 1020	_	_	-	_	_	1022	895	-	801	735	- 100 7
Stage 2	_	-	-	-	-	-	798	734	-	993	895	-
Platoon blocked, %		_	_		_	_	, , , ,	, 0 1		,,,	370	
Mov Cap-1 Maneuver	1620	-	-	1622	-	-	712	650	1084	664	651	1084
Mov Cap-2 Maneuver	-	_	_		_	_	712	650	-	664	651	-
Stage 1	-	-	-	-	-	-	1022	895	-	801	689	-
Stage 2	_	_	_	_	_	_	742	688	_	950	895	_
2.a.go 2								300		.00	3,0	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			7.2			8.5			10.6		
HCM LOS	U			1.2			6.5 A			10.0 B		
TIOWI LOS							А			ט		
Minor Lane/Major Mvm	nt N	NBLn1 i	VRI n2	EBL	EBT	EBR	WBL	WBT	WBR:	SBI n1		
Capacity (veh/h)	1		1065	1620	LD1	LDIN.	1622	**D1	· ·	659		
HCM Lane V/C Ratio			0.044	1020	-	-	0.062	-		0.026		
HCM Control Delay (s)		0	8.5	0	-		7.4	-	-			
HCM Lane LOS		A	Α	A	-	-	7.4 A	-	-	В		
HCM 95th %tile Q(veh))	- A	0.1	0	-	-	0.2	-	-	0.1		
110W 73W 70W Q(VCH)			0.1	U	-		0.2			0.1		

APPENDIX F CAPACITY ANALYSIS WORKSHEETS TOTAL TRAFFIC

	۶	→	•	√	←	•	•	†	/	>		✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	14.54	↑	7	ሻሻ	•	7	ሻ	^	7	7	^	7
Traffic Volume (veh/h)	22	16	70	51	15	162	39	779	47	457	1841	27
Future Volume (veh/h)	22	16	70	51	15	162	39	779	47	457	1841	27
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	25	18	79	57	17	-22	44	875	53	513	2069	30
Adj No. of Lanes	2	1	1	2	1	1	1	2	1	1	2	1
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	79	123	104	115	142	121	139	2194	982	455	2290	1024
Arrive On Green	0.02	0.07	0.07	0.03	0.08	0.00	0.03	0.62	0.62	0.06	0.65	0.65
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	1774	3539	1583	1774	3539	1583
Grp Volume(v), veh/h	25	18	79	57	17	-22	44	875	53	513	2069	30
Grp Sat Flow(s), veh/h/ln	1721	1863	1583	1721	1863	1583	1774	1770	1583	1774	1770	1583
Q Serve(g_s), s	0.9	1.2	6.4	2.1	1.1	0.0	1.2	16.2	1.7	7.5	64.6	0.9
Cycle Q Clear(g_c), s	0.9	1.2	6.4	2.1	1.1	0.0	1.2	16.2	1.7	7.5	64.6	0.9
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	79	123	104	115	142	121	139	2194	982	455	2290	1024
V/C Ratio(X)	0.32	0.15	0.76	0.49	0.12	-0.18	0.32	0.40	0.05	1.13	0.90	0.03
Avail Cap(c_a), veh/h	212	272	231	212	272	231	187	2194	982	455	2290	1024
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	0.93	0.93	0.93	1.00	1.00	1.00
Uniform Delay (d), s/veh	62.5	57.3	59.7	61.7	56.0	0.0	26.7	12.5	9.7	25.0	19.5	8.3
Incr Delay (d2), s/veh	1.7	0.7	12.7	2.4	0.4	0.0	0.9	0.5	0.1	81.9	6.4	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	0.6	3.2	1.0	0.6	0.0	1.0	8.1	0.8	23.0	33.4	0.4
LnGrp Delay(d),s/veh	64.2	57.9	72.4	64.1	56.4	0.0	27.6	13.0	9.8	106.9	25.9	8.3
LnGrp LOS	Ε	Ε	Ε	Ε	Ε		С	В	Α	F	С	А
Approach Vol, veh/h		122			52			972			2612	
Approach Delay, s/veh		68.6			88.8			13.5			41.6	
Approach LOS		Е			F			В			D	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.5	92.6	11.4	14.5	15.0	89.1	10.0	15.9				
Change Period (Y+Rc), s	7.5	8.5	7.0	6.0	7.5	8.5	7.0	6.0				
Max Green Setting (Gmax), s	7.5	66.5	8.0	19.0	7.5	66.5	8.0	19.0				
Max Q Clear Time (g_c+I1), s	3.2	66.6	4.1	8.4	9.5	18.2	2.9	3.1				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.3	0.0	47.7	0.0	0.4				
Intersection Summary												
HCM 2010 Ctrl Delay			35.9									
HCM 2010 LOS			D									
			_									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	14	↑	7	ሻሻ		7	ሻሻ	^	7	ሻሻ	^	7
Traffic Volume (veh/h)	29	11	11	66	11	154	41	636	61	321	1595	33
Future Volume (veh/h)	29	11	11	66	11	154	41	636	61	321	1595	33
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	34	12	8	72	12	104	48	740	66	349	1855	38
Adj No. of Lanes	2	1	1	2	1	1	2	2	1	2	2	1
Peak Hour Factor	0.86	0.92	0.86	0.92	0.92	0.92	0.86	0.86	0.92	0.92	0.86	0.86
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	114	148	126	114	148	126	99	1997	893	379	2284	1022
Arrive On Green	0.03	0.08	0.08	0.03	0.08	0.08	0.06	1.00	1.00	0.11	0.65	0.65
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	3442	3539	1583	3442	3539	1583
Grp Volume(v), veh/h	34	12	8	72	12	104	48	740	66	349	1855	38
Grp Sat Flow(s),veh/h/ln	1721	1863	1583	1721	1863	1583	1721	1770	1583	1721	1770	1583
Q Serve(g_s), s	1.4	0.9	0.7	3.1	0.9	9.7	2.0	0.0	0.0	15.1	58.6	1.3
Cycle Q Clear(g_c), s	1.4	0.9	0.7	3.1	0.9	9.7	2.0	0.0	0.0	15.1	58.6	1.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	114	148	126	114	148	126	99	1997	893	379	2284	1022
V/C Ratio(X)	0.30	0.08	0.06	0.63	0.08	0.83	0.48	0.37	0.07	0.92	0.81	0.04
Avail Cap(c_a), veh/h	298	161	137	413	224	190	264	1997	893	379	2284	1022
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.39	0.39	0.39
Uniform Delay (d), s/veh	70.8	64.0	63.9	71.6	64.0	68.0	69.6	0.0	0.0	66.1	19.8	9.7
Incr Delay (d2), s/veh	1.4	0.2	0.2	5.7	0.2	16.5	3.5	0.5	0.2	13.9	1.3	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	0.5	0.3	1.6	0.5	4.8	1.0	0.1	0.0	7.9	28.8	0.6
LnGrp Delay(d),s/veh	72.2	64.2	64.1	77.3	64.2	84.5	73.1	0.5	0.2	80.0	21.2	9.7
LnGrp LOS	Е	Е	Е	Е	Е	F	Е	Α	Α	F	С	Α
Approach Vol, veh/h		54			188			854			2242	
Approach Delay, s/veh		69.3			80.4			4.6			30.1	
Approach LOS		E			F			Α			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.8	106.3	12.0	18.9	25.0	94.1	12.0	18.9				
Change Period (Y+Rc), s	8.5	9.5	7.0	7.0	8.5	9.5	7.0	7.0				
Max Green Setting (Gmax), s	11.5	75.5	18.0	13.0	16.5	70.5	13.0	18.0				
Max Q Clear Time (g_c+l1), s	4.0	60.6	5.1	2.9	17.1	2.0	3.4	11.7				
Green Ext Time (p_c), s	0.0	13.1	0.1	0.3	0.0	43.1	0.0	0.2				
	3.0		J. 1	5.0	3.0	. 5	3.0	J.L				
Intersection Summary			27.4									
HCM 2010 Ctrl Delay			27.1									
HCM 2010 LOS			С									

MovementEBLLane ConfigurationsTraffic Volume (veh/h)16Future Volume (veh/h)16Number7Initial Q (Qb), veh0Ped-Bike Adj(A_pbT)1.00Parking Bus, Adj1.00Adj Sat Flow, veh/h/ln1863Adj Flow Rate, veh/h19Adj No. of Lanes2Peak Hour Factor0.84Percent Heavy Veh, %2Cap, veh/h104Arrive On Green0.03Sat Flow, veh/h3442Grp Volume(v), veh/h19Grp Sat Flow(s),veh/h/ln1721Q Serve(g_s), s0.8Cycle Q Clear(g_c), s0.8Prop In Lane1.00Lane Grp Cap(c), veh/h104V/C Ratio(X)0.18Avail Cap(c_a), veh/h298HCM Platoon Ratio1.00Upstream Filter(I)1.00Uniform Delay (d2), s/veh0.8Initial Q Delay(d3),s/veh0.0%ile BackOfQ(50%),veh/ln0.4LnGrp Delay(d),s/veh71.8LnGrp LOSEApproach Vol, veh/hApproach Delay, s/vehApproach LOS	EBT 4 4 4 0 1.00 1863 4 1 0.92 2 72 0.04 1863 4 1863 0.3 0.3	EBR 6 6 6 14 0 1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00 61	WBL 28 28 3 0 1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3 1.00	WBT 3 3 8 0 1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2 0.2	WBR 48 48 48 0 1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	NBL 42 42 1 0 1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	NBT 662 662 66 0 1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	NBR 33 33 36 16 0 1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583 1.0	SBL 59 59 59 0 1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721 2.7	\$BT 1527 1527 1527 2 0 1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	SBR 63 63 12 0 1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583 75 1583
Traffic Volume (veh/h) 16 Future Volume (veh/h) 16 Number 7 Initial Q (Qb), veh 0 Ped-Bike Adj(A_pbT) 1.00 Parking Bus, Adj 1.00 Adj Sat Flow, veh/h/ln 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s), veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.8 Initial Q Delay(d3),s/veh 0.4	4 4 0 1.00 1863 4 1 0.92 2 72 0.04 1863 0.3 0.3	6 6 6 14 0 1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	28 28 3 0 1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	3 8 0 1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	48 48 18 0 1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	42 42 1 0 1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	662 662 6 0 1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	33 33 16 0 1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583	59 59 5 0 1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721	1527 1527 2 0 1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	63 63 12 0 1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583 75
Traffic Volume (veh/h) 16 Future Volume (veh/h) 16 Number 7 Initial Q (Qb), veh 0 Ped-Bike Adj(A_pbT) 1.00 Parking Bus, Adj 1.00 Adj Sat Flow, veh/h/ln 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s), veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.8 Initial Q Delay(d3),s/veh 0.4	4 4 0 1.00 1863 4 1 0.92 2 72 0.04 1863 0.3 0.3	6 14 0 1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4	28 28 3 0 1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	3 8 0 1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	48 18 0 1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	42 42 1 0 1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	662 662 6 0 1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	33 16 0 1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583	59 59 5 0 1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721	1527 1527 2 0 1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	63 12 0 1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583
Number 7 Initial Q (Qb), veh 0 Ped-Bike Adj(A_pbT) 1.00 Parking Bus, Adj 1.00 Adj Sat Flow, veh/h/In 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 19 Grp Volume(v), veh/h 19 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/In 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.8 Initial Q Delay(d3),s/veh 0.4 LnGrp Delay(d),s/veh 71.8 <td>1.00 1863 4 1 0.92 2 72 0.04 1863 4 1863 0.3</td> <td>14 0 1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4</td> <td>3 0 1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3</td> <td>8 0 1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2</td> <td>18 0 1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0</td> <td>1 0 1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1</td> <td>1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2</td> <td>16 0 1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583</td> <td>5 0 1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721</td> <td>2 0 1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770</td> <td>12 0 1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583</td>	1.00 1863 4 1 0.92 2 72 0.04 1863 4 1863 0.3	14 0 1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4	3 0 1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	8 0 1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	18 0 1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	1 0 1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	16 0 1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583	5 0 1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721	2 0 1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	12 0 1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583
Initial Q (Qb), veh 0 Ped-Bike Adj(A_pbT) 1.00 Parking Bus, Adj 1.00 Adj Sat Flow, veh/h/ln 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 0.8 Initial Q Delay(d3),s/veh 0.9 licg BackOfQ(50%),veh/ln 0.4 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1.00 1863 4 1 0.92 2 72 0.04 1863 0.3 0.3	0 1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4	0 1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	0 1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	0 1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	0 1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	0 1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583	0 1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721	0 1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	0 1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583
Ped-Bike Adj(A_pbT) 1.00 Parking Bus, Adj 1.00 Adj Sat Flow, veh/h/ln 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 19 Grp Volume(v), veh/h 19 Grp Sat Flow(s), veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay (d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h <td>1.00 1863 4 1 0.92 2 72 0.04 1863 0.3 0.3</td> <td>1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00</td> <td>1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3</td> <td>1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2</td> <td>1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0</td> <td>1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1</td> <td>1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2</td> <td>1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583</td> <td>1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721</td> <td>1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770</td> <td>1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583</td>	1.00 1863 4 1 0.92 2 72 0.04 1863 0.3 0.3	1.00 1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	1.00 1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	1.00 1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	1.00 1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	1.00 1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	1.00 1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	1.00 1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583	1.00 1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721	1.00 1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	1.00 1.00 1863 75 1 0.84 2 1138 1.00 1583
Parking Bus, Adj 1.00 Adj Sat Flow, veh/h/ln 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 19 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1863 4 1 0.92 2 72 0.04 1863 4 1863 0.3 0.3	1.00 1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	1.00 1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	1.00 1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	1.00 1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	1.00 1863 36 1 0.92 2 1135 0.72 1583 36 1583	1.00 1863 64 2 0.92 2 107 0.06 3442 64 1721	1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	1.00 1863 75 1 0.84 2 1138 1.00 1583
Adj Sat Flow, veh/h/ln 1863 Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1863 4 1 0.92 2 72 0.04 1863 4 1863 0.3 0.3	1863 4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	1863 30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	1863 3 1 0.92 2 60 0.03 1863 3 1863 0.2	1863 32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	1863 50 2 0.84 2 100 0.03 3442 50 1721 2.1	1863 788 2 0.84 2 2537 0.72 3539 788 1770 12.2	1863 36 1 0.92 2 1135 0.72 1583 36 1583	1863 64 2 0.92 2 107 0.06 3442 64 1721	1863 1818 2 0.84 2 2543 1.00 3539 1818 1770	1863 75 1 0.84 2 1138 1.00 1583 75
Adj Flow Rate, veh/h 19 Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/In 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh/h 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	4 1 0.92 2 72 0.04 1863 4 1863 0.3 0.3	4 1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	30 2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	3 1 0.92 2 60 0.03 1863 3 1863 0.2	32 1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	50 2 0.84 2 100 0.03 3442 50 1721 2.1	788 2 0.84 2 2537 0.72 3539 788 1770 12.2	36 1 0.92 2 1135 0.72 1583 36 1583	64 2 0.92 2 107 0.06 3442 64 1721	1818 2 0.84 2 2543 1.00 3539 1818 1770	75 1 0.84 2 1138 1.00 1583 75
Adj No. of Lanes 2 Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1 0.92 2 72 0.04 1863 4 1863 0.3 0.3	1 0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	2 0.92 2 82 0.02 3442 30 1721 1.3 1.3	1 0.92 2 60 0.03 1863 3 1863 0.2	1 0.92 2 51 0.03 1583 32 1583 3.0 3.0	2 0.84 2 100 0.03 3442 50 1721 2.1	2 0.84 2 2537 0.72 3539 788 1770 12.2	1 0.92 2 1135 0.72 1583 36 1583	2 0.92 2 107 0.06 3442 64 1721	2 0.84 2 2543 1.00 3539 1818 1770	1 0.84 2 1138 1.00 1583 75
Peak Hour Factor 0.84 Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.92 2 72 0.04 1863 4 1863 0.3 0.3	0.84 2 61 0.04 1583 4 1583 0.4 0.4 1.00	0.92 2 82 0.02 3442 30 1721 1.3 1.3	0.92 2 60 0.03 1863 3 1863 0.2	0.92 2 51 0.03 1583 32 1583 3.0 3.0	0.84 2 100 0.03 3442 50 1721 2.1	0.84 2 2537 0.72 3539 788 1770 12.2	0.92 2 1135 0.72 1583 36 1583	0.92 2 107 0.06 3442 64 1721	0.84 2 2543 1.00 3539 1818 1770	0.84 2 1138 1.00 1583 75
Percent Heavy Veh, % 2 Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	2 72 0.04 1863 4 1863 0.3 0.3	2 61 0.04 1583 4 1583 0.4 0.4 1.00	2 82 0.02 3442 30 1721 1.3 1.3	2 60 0.03 1863 3 1863 0.2	2 51 0.03 1583 32 1583 3.0 3.0	2 100 0.03 3442 50 1721 2.1	2 2537 0.72 3539 788 1770 12.2	2 1135 0.72 1583 36 1583	2 107 0.06 3442 64 1721	2 2543 1.00 3539 1818 1770	1138 1.00 1583 75
Cap, veh/h 104 Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	72 0.04 1863 4 1863 0.3 0.3	61 0.04 1583 4 1583 0.4 0.4 1.00	82 0.02 3442 30 1721 1.3 1.3	60 0.03 1863 3 1863 0.2	51 0.03 1583 32 1583 3.0 3.0	100 0.03 3442 50 1721 2.1	2537 0.72 3539 788 1770 12.2	1135 0.72 1583 36 1583	107 0.06 3442 64 1721	2543 1.00 3539 1818 1770	1138 1.00 1583 75
Arrive On Green 0.03 Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/In 0.4 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.04 1863 4 1863 0.3 0.3	0.04 1583 4 1583 0.4 0.4 1.00	0.02 3442 30 1721 1.3 1.3	0.03 1863 3 1863 0.2	0.03 1583 32 1583 3.0 3.0	0.03 3442 50 1721 2.1	0.72 3539 788 1770 12.2	0.72 1583 36 1583	0.06 3442 64 1721	1.00 3539 1818 1770	1.00 1583 75
Sat Flow, veh/h 3442 Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1863 4 1863 0.3 0.3	1583 4 1583 0.4 0.4 1.00	3442 30 1721 1.3 1.3 1.00	1863 3 1863 0.2	1583 32 1583 3.0 3.0	3442 50 1721 2.1	3539 788 1770 12.2	1583 36 1583	3442 64 1721	3539 1818 1770	1583 75
Grp Volume(v), veh/h 19 Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	4 1863 0.3 0.3	4 1583 0.4 0.4 1.00	30 1721 1.3 1.3 1.00	3 1863 0.2	32 1583 3.0 3.0	50 1721 2.1	788 1770 12.2	36 1583	64 1721	1818 1770	75
Grp Sat Flow(s),veh/h/ln 1721 Q Serve(g_s), s 0.8 Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1863 0.3 0.3	1583 0.4 0.4 1.00	1721 1.3 1.3 1.00	1863 0.2	1583 3.0 3.0	1721 2.1	1770 12.2	1583	1721	1770	
Q Serve(g_s), s	0.3 0.3 72	0.4 0.4 1.00	1.3 1.3 1.00	0.2	3.0 3.0	2.1	12.2				1583
Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.3 72	0.4 1.00	1.3 1.00		3.0			1.0	2.7	0.0	1000
Cycle Q Clear(g_c), s 0.8 Prop In Lane 1.00 Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	72	1.00	1.00	0.2		2 1	40.0		۷.۱	0.0	0.0
Lane Grp Cap(c), veh/h 104 V/C Ratio(X) 0.18 Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh					4.00	۷. ۱	12.2	1.0	2.7	0.0	0.0
V/C Ratio(X) Avail Cap(c_a), veh/h HCM Platoon Ratio Upstream Filter(I) Uniform Delay (d), s/veh Incr Delay (d2), s/veh Nile BackOfQ(50%),veh/ln LnGrp Delay(d),s/veh Approach Vol, veh/h Approach Delay, s/veh		61	00		1.00	1.00		1.00	1.00		1.00
Avail Cap(c_a), veh/h 298 HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/In 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.00		82	60	51	100	2537	1135	107	2543	1138
HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/In 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.06	0.07	0.37	0.05	0.63	0.50	0.31	0.03	0.60	0.71	0.07
HCM Platoon Ratio 1.00 Upstream Filter(I) 1.00 Uniform Delay (d), s/veh 70.9 Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/In 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	161	137	298	161	137	264	2537	1135	264	2543	1138
Uniform Delay (d), s/veh Incr Delay (d2), s/veh Initial Q Delay(d3),s/veh %ile BackOfQ(50%),veh/ln LnGrp Delay(d),s/veh T1.8 LnGrp LOS Approach Vol, veh/h Approach Delay, s/veh	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Incr Delay (d2), s/veh 0.8 Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	1.00	1.00	1.00	1.00	1.00	0.97	0.97	0.97	0.57	0.57	0.57
Initial Q Delay(d3),s/veh 0.0 %ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	69.5	69.5	72.1	70.4	71.7	71.7	7.7	6.2	69.4	0.0	0.0
%ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.3	0.4	2.7	0.3	12.3	3.7	0.3	0.1	3.0	1.0	0.1
%ile BackOfQ(50%),veh/ln 0.4 LnGrp Delay(d),s/veh 71.8 LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp LOS E Approach Vol, veh/h Approach Delay, s/veh	0.2	0.2	0.6	0.1	1.5	1.1	6.0	0.4	1.3	0.4	0.0
Approach Vol, veh/h Approach Delay, s/veh	69.8	70.0	74.8	70.7	84.0	75.4	8.1	6.2	72.5	1.0	0.1
Approach Delay, s/veh	Е	Е	Е	Е	F	Е	Α	Α	Е	Α	Α
Approach Delay, s/veh	27			65			874			1957	
	71.2			79.2			11.8			3.3	
• •	Е			Е			В			Α	
Timer 1	2	3	4	5	6	7	8				
Assigned Phs 1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s 12.9	113.8	10.6	12.8	13.2	113.5	11.5	11.8				
Change Period (Y+Rc), s 8.5	6.0	7.0	7.0	8.5	6.0	7.0	7.0				
Max Green Setting (Gmax), s 11.5	84.0	13.0	13.0	11.5	84.0	13.0	13.0				
Max Q Clear Time (g_c+l1), s 4.1	2.0	3.3	2.4	4.7	14.2	2.8	5.0				
Green Ext Time (p_c), s 0.0		0.0	0.1	0.1	43.7	0.0	0.0				
Intersection Summary	47.9										
HCM 2010 Ctrl Delay	47.9										
HCM 2010 LOS	47.9	8.2									

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→	*	*	•	•	7	
Movement EB1	EBR		NBL	WBT	NBL	NBR
Lane Configurations 🙌	7	7	14	^	1,1	7
Traffic Volume (veh/h) 1540			39	731	4	11
Future Volume (veh/h) 1540	17	17	39	731	4	11
Number 2			1	6	7	14
Initial Q (Qb), veh			0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00		1.00	1.00
Parking Bus, Adj 1.00			1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln 1863			863	1863	1863	1863
Adj Flow Rate, veh/h 1878			48	891	5	13
•						
Adj No. of Lanes 2			2	2	2	1
Peak Hour Factor 0.82			0.82	0.82	0.82	0.82
Percent Heavy Veh, % 2			2	2	2	2
Cap, veh/h 2790			109	3120	63	29
Arrive On Green 0.79	0.79	0.79	0.06	1.00	0.02	0.02
Sat Flow, veh/h 3632	1583	1583 3	3442	3632	3442	1583
Grp Volume(v), veh/h 1878	21	21	48	891	5	13
Grp Sat Flow(s), veh/h/ln1770			721	1770	1721	1583
Q Serve(g_s), s 31.1	0.4		1.7	0.0	0.2	1.1
Cycle Q Clear(g_c), s 31.1	0.4		1.7	0.0	0.2	1.1
Prop In Lane	1.00		1.00	0.0	1.00	1.00
Lane Grp Cap(c), veh/h 2790			109	3120	63	29
			0.44		0.08	0.45
V/C Ratio(X) 0.67	0.02			0.29		
Avail Cap(c_a), veh/h 2790			318	3120	331	152
HCM Platoon Ratio 1.00			2.00	2.00	1.00	1.00
Upstream Filter(I) 0.68			0.93	0.93	1.00	1.00
Uniform Delay (d), s/veh 6.2			59.8	0.0	62.7	63.2
Incr Delay (d2), s/veh 0.9	0.0	0.0	2.6	0.2	0.5	10.4
Initial Q Delay(d3),s/veh 0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/lir5.2			0.9	0.1	0.1	0.5
LnGrp Delay(d),s/veh 7.1	3.0		62.4	0.2	63.3	73.5
LnGrp LOS A	Α		E	Α	E	70.0 E
Approach Vol, veh/h 1899		, ,		939	18	
Approach LOS				3.4	70.7	
Approach LOS A				Α	Е	
Timer 1	2	2	3	4	5	6
Assigned Phs 1	2	2		4		6
Phs Duration (G+Y+Rc), \$2.1				9.9		120.1
Change Period (Y+Rc), s 8.0				7.5		5.5
Max Green Setting (Gmak), 6				12.5		104.5
Max Q Clear Time (g_c+l13,7				3.1		2.0
Green Ext Time (p_c), s 0.0	37.7	31.1		0.0		58.3
Intersection Summary						
HCM 2010 Ctrl Delay			6.2			
HCM 2010 LOS			Α			
HOW ZUTU LUG			А			

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Marriage			†	₩.	MOT	- WDD)	I NOT	/ NDD	CD!	▼	-
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	- 7	- ሽ	^	7	ሻሻ		7	ሻሻ		7
Traffic Volume (veh/h)	38	1347	168	39	584	77	133	41	37	110	47	60
Future Volume (veh/h)	38	1347	168	39	584	77	133	41	37	110	47	60
Number	5	2	12	1	6	16	7	4	14	3	8	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	45	1604	200	46	695	92	158	49	44	131	56	71
Adj No. of Lanes	1	2	1	1	2	1	2	1	1	2	1	1
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	470	2203	986	271	2190	980	209	117	100	181	117	99
Arrive On Green	0.06	1.00	1.00	0.03	0.62	0.62	0.06	0.06	0.06	0.05	0.06	0.06
Sat Flow, veh/h	1774	3539	1583	1774	3539	1583	3442	1863	1583	3442	1863	1583
Grp Volume(v), veh/h	45	1604	200	46	695	92	158	49	44	131	56	71
Grp Sat Flow(s), veh/h/lr		1770	1583	1774	1770	1583	1721	1863	1583	1721	1863	1583
Q Serve(g_s), s	1.2	0.0	0.0	1.2	12.1	3.1	5.9	3.3	3.5	4.9	3.8	5.7
Cycle Q Clear(q_c), s	1.2	0.0	0.0	1.2	12.1	3.1	5.9	3.3	3.5	4.9	3.8	5.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h		2203	986	271	2190	980	209	117	100	181	117	99
V/C Ratio(X)	0.10	0.73	0.20	0.17	0.32	0.09	0.76	0.42	0.44	0.72	0.48	0.72
Avail Cap(c_a), veh/h	511	2203	986	318	2190	980	225	201	171	199	201	171
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.73	0.73	0.73	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/vel		0.0	0.0	8.2	11.7	10.0	60.1	58.6	58.7	60.7	58.9	59.8
Incr Delay (d2), s/veh	0.1	1.6	0.3	0.3	0.4	0.2	12.9	2.4	3.0	11.2	3.0	9.2
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),vel		0.5	0.0	0.6	6.0	1.4	3.2	1.8	1.6	2.6	2.0	2.8
LnGrp Delay(d),s/veh	8.6	1.6	0.1	8.5	12.1	10.2	73.0	61.0	61.7	71.8	61.9	69.0
LnGrp LOS	Α	Α	Α	Α	В	В	73.0 E	61.0 E	E	71.0 E	E	67.0 E
Approach Vol, veh/h		1849			833	U		251			258	
Approach Delay, s/veh		1.6			11.7			68.7			68.9	
Approach LOS		1.0 A			11.7 B			00.7 E			08.9 E	
		А			D			E			E	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc)	, \$1.6	89.9	14.3	14.2	12.0	89.5	14.4	14.2				
Change Period (Y+Rc),		9.0	7.5	6.0	8.0	9.0	6.5	6.0				
Max Green Setting (Gm		71.0	7.5	14.0	7.0	71.0	8.5	14.0				
Max Q Clear Time (q_c		2.0	6.9	5.5	3.2	14.1	7.9	7.7				
Green Ext Time (p_c), s		37.3	0.0	0.5	0.0	33.7	0.0	0.4				
Intersection Summary												
HCM 2010 Ctrl Delay			15.0									
			15.0 B									
HCM 2010 LOS			R									

Intersection							
Int Delay, s/veh	8.7						١
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	\$			4	*	7	
Traffic Vol, veh/h	33	4	186	30	11	349	
Future Vol, veh/h	33	4	186	30	11	349	
Conflicting Peds, #/hr	0	0	0	0	0	0	
· ·	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-		-	None	
Storage Length	-	-	-	-	100	0	
Veh in Median Storage, #	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	84	84	84	84	84	84	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	39	5	221	36	13	415	
NA - ! /NA! NA -	-!1		M-!0		M:1		
	ajor1		Major2		Minor1	40	
Conflicting Flow All	0	0	44	0	520	42	
Stage 1	-	-	-	-	42	-	
Stage 2	-	-	-	-	478	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	0.0.0		
Pot Cap-1 Maneuver	-	-	1564	-	516	1029	
Stage 1	-	-	-	-	980	-	
Stage 2	-	-	-	-	624	-	
Platoon blocked, %	-	-		-			
Mov Cap-1 Maneuver	-	-	1564	-	442	1029	
Mov Cap-2 Maneuver	-	-	-	-	442	-	
Stage 1	-	-	-	-	980	-	
Stage 2	-	-	-	-	534	-	
Approach	EB		WB		NB		Ī
HCM Control Delay, s	0		6.6		10.9		
HCM LOS	U		0.0		В		
TIGIVI EOS					D		
Minor Long/Major Mumt	N	IBLn1 I	NBLn2	EBT	EBR	WBL	
Minor Lane/Major Mvmt						1 [/ /	
Capacity (veh/h)		442	1029	-	-	1564	
Capacity (veh/h) HCM Lane V/C Ratio			1029 0.404	-		0.142	
Capacity (veh/h)							
Capacity (veh/h) HCM Lane V/C Ratio		0.03	0.404	-	-	0.142	

7. Clari Darle Assass	s #2/W Park Dr & Wilkinso	ı Dl∠
7. Clari Park Access	S #2/VV Park Dr & VVIIKINSO	ın Pk

Intersection												
Int Delay, s/veh	1.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	21	354	8	4	168	15	7	0	3	26	0	38
Future Vol, veh/h	21	354	8	4	168	15	7	0	3	26	0	38
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	92	92	84	84	92	92	92	84	92	84
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	25	421	9	4	200	18	8	0	3	31	0	45
Major/Minor N	Major1		N	Major2		ا	Minor1		<u> </u>	Minor2		
Conflicting Flow All	218	0	0	430	0	0	716	702	426	694	697	209
Stage 1	-	-	-	-	-	-	476	476	-	217	217	-
Stage 2	-	-	-	-	-	-	240	226	-	477	480	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1352	-	-	1129	-	-	345	362	628	357	365	831
Stage 1	-	-	-	-	-	-	570	557	-	785	723	-
Stage 2	-	-	-	-	-	-	763	717	-	569	554	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1352	-	-	1129	-	-	319	352	628	348	355	831
Mov Cap-2 Maneuver	-	-	-	-	-	-	319	352	-	348	355	-
Stage 1	-	-	-	-	-	-	556	544	-	766	720	-
Stage 2	-	-	-	-	-	-	719	714	-	552	541	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.2			14.9			12.9		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt [NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		374	1352	-		1129	-	-	=0.4			
HCM Lane V/C Ratio			0.018	-		0.004	-	-	0.143			
HCM Control Delay (s)		14.9	7.7	0	-	8.2	0	-				
HCM Lane LOS		В	Α	A	-	A	A	-	В			
HCM 95th %tile Q(veh))	0.1	0.1	-	-	0	-	-	0.5			

Int Delay, s/veh							
=j,	7.2						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ.			4	ሻ	7	
Traffic Vol, veh/h	378	8	137	172	12	316	
Future Vol, veh/h	378	8	137	172	12	316	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	_	-	_	-	100	0	
Veh in Median Storage	, # 0	_	_	0	0	-	
Grade, %	0	_	_	0	0	_	
Peak Hour Factor	87	87	87	87	87	87	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	434	9	157	198	14	363	
IVIVIIIL I IOW	434	7	107	170	14	303	
Major/Minor N	/lajor1	N	Major2	N	Minor1		
Conflicting Flow All	0	0	443	0	951	439	
Stage 1	-	-	-	-	439	-	
Stage 2	-	-	-	-	512	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1117	-	288	618	
Stage 1	-	-	_	-	650	-	
Stage 2	_	-	_	-	602	-	
Platoon blocked, %	_	_		-	002		
Mov Cap-1 Maneuver	_	-	1117	-	242	618	
Mov Cap-2 Maneuver	_	_		_	242	-	
Stage 1	_	_	_	_	650	_	
Stage 2	_	_	_		507	_	
Staye 2	-	-	-		307	-	
Approach	EB		WB		NB		
HCM Control Delay, s	0		3.9		18.9		
HCM LOS					С		
Minor Lane/Major Mvm	+ 1	NBLn1 N	\IRI n2	EBT	EBR	WBL	
	l 1						
Capacity (veh/h)		242	618	-		1117	
		0.057		-		0.141	
HCM Lane V/C Ratio					-	QQ	
HCM Control Delay (s)		20.8	18.8	-		8.8	
		20.8 C 0.2	18.8 C	- -	-	0.6 A 0.5	

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	î,			f)		ሻ	f _a			44	
Traffic Vol, veh/h	0	259	68	21	119	7	84	2	41	0	4	0
Future Vol, veh/h	0	259	68	21	119	7	84	2	41	0	4	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	90	-	-	100	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	291	76	24	134	8	94	2	46	0	4	0
Major/Minor N	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	142	0	0	367	0	0	517	519	329	539	553	138
Stage 1	-	-	-	-	-	-	329	329	-	186	186	-
Stage 2	-	-	-	-	-	-	188	190	-	353	367	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318		4.018	
Pot Cap-1 Maneuver	1441	-	-	1192	-	-	469	461	712	453	441	910
Stage 1	-	-	-	-	-	-	684	646	-	816	746	-
Stage 2	-	-	-	-	-	-	814	743	-	664	622	-
Platoon blocked, %	1 / / /	-	-	1100	-	-	450	450	710	115	422	010
Mov Cap-1 Maneuver	1441	-	-	1192	-	-	458	452	712	415	432	910
Mov Cap-2 Maneuver	-	-	-	-	-	-	458	452	-	415	432 731	-
Stage 1	-	-	-	-	-	-	684 793	646 728	-	816 619	622	-
Stage 2	-	-	-	-	-	-	193	728	-	019	022	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.2			13.4			13.4		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt	NBLn1 I	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1		
Capacity (veh/h)		458	693	1441	-	_	1192	-		432		
HCM Lane V/C Ratio		0.206	0.07	-	-	-	0.02	-	-	0.01		
HCM Control Delay (s)		14.9	10.6	0	-	-	8.1	-	-	13.4		
HCM Lane LOS		В	В	Α	-	-	Α	-	-	В		
HCM 95th %tile Q(veh))	0.8	0.2	0	-	-	0.1	-	-	0		

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	CDD
		ERK				SBR
Lane Configurations	7	2	<u>ች</u>	4 121	\$	1
Traffic Vol, veh/h Future Vol, veh/h	3	3	4	121 121	90 90	4
		3	4			
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	3	4	132	98	4
Major/Minor	Minor2		Major1	Λ	/lajor2	
Conflicting Flow All	240	100	102	0	-	0
Stage 1	100	-	-	-	-	-
Stage 2	140	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	_	-
Critical Hdwy Stg 1	5.42	-	_	-	-	-
Critical Hdwy Stg 2	5.42	_	-	-	-	-
Follow-up Hdwy		3.318	2.218	-	-	-
Pot Cap-1 Maneuver	748	956	1490	-	-	-
Stage 1	924	-	-	-	-	-
Stage 2	887	_	-	-	-	-
Platoon blocked, %	007			_	_	_
Mov Cap-1 Maneuver	746	956	1490	-	_	_
Mov Cap-1 Maneuver	752	750	- 1770	_	_	_
Stage 1	921		-	_	_	_
Stage 2	887	-	-	-	-	-
Staye 2	007	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.3		0.2		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1	SBT	SBR
	IL					SDIX
Capacity (veh/h)		1490	-	٠.ـ	-	-
HCM Control Dolay (c)		0.003		0.008	-	-
HCM Control Delay (s) HCM Lane LOS		7.4	0	9.3	-	-
HCM 95th %tile Q(veh	١	A 0	Α	A 0	-	-
HOW FOUT WILLE CLIVELL)	U	-	U	-	-

Clari Park

Intersection												
Int Delay, s/veh	1.3											
		CDT	EDD	WDI	MDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	2	♣	1	20	4	7	<u> </u>	þ	г	ች	^	4
Traffic Vol, veh/h	3	0	3	20	0	7	4	117	5	1	131	4
Future Vol, veh/h	3	0	3	20	0	7	4	117	5	1	131	4
Conflicting Peds, #/hr	0 Ctop	O Ctop		0 Ctop	0 Ctop	0 Ctop	0	0 Free	0 Free	0	0 Free	0
Sign Control RT Channelized	Stop	Stop	Stop None	Stop -	Stop -	Stop None	Free -	riee -	None	Free -	riee -	Free None
Storage Length	-	_	None	-	_	NUHE -	50	-	None -	50	_	None
Veh in Median Storage		0		-	0	-	-	0	-	-	0	-
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	0	3	22	0	8	4	127	5	1	142	4
	_	_	_			_	•			•		•
Major/Minor I	Minor2			Minor1			Major1			Majora		
		286	144	286	286	130	<u>Major1</u> 146	0		Major2 132	0	0
Conflicting Flow All Stage 1	288 146	146	144	138	138	130	140	0	0	132	0	0
Stage 2	140	140	-	148	148	-	-	_		_	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12		-	4.12	-	
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	7.12	_	_	7.12	_	_
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218		_	2.218	_	_
Pot Cap-1 Maneuver	664	623	903	666	623	920	1436	-	-	1453	-	-
Stage 1	857	776	-	865	782	-	-	-	-	-	-	-
Stage 2	861	781	-	855	775	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	657	621	903	662	621	920	1436	-	-	1453	-	-
Mov Cap-2 Maneuver	657	621	-	662	621	-	-	-	-	-	-	-
Stage 1	854	775	-	862	780	-	-	-	-	-	-	-
Stage 2	852	779	-	851	774	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.8			10.3			0.2			0.1		
HCM LOS	Α			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	EBLn1V	VRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1436	1101	- INDIX		714	1453		JDIN.			
HCM Lane V/C Ratio		0.003	-			0.041	0.001	-	-			
HCM Control Delay (s)		7.5		-	9.8	10.3	7.5	-	-			
HCM Lane LOS		7.5 A	-	_	7.0 A	В	7.5 A	_	_			
HCM 95th %tile Q(veh))	0	-	-	0	0.1	0	-	-			
2 700 2(1011)	,											

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	1→		ሻ	ĵ.	
Traffic Vol, veh/h	3	0	10	31	0	10	13	112	4	2	147	4
Future Vol, veh/h	3	0	10	31	0	10	13	112	4	2	147	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage	-, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	0	11	34	0	11	14	122	4	2	160	4
Major/Minor N	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	324	320	162	324	320	124	164	0	0	126	0	0
Stage 1	166	166	-	152	152	-	-		_	-	-	-
Stage 2	158	154	-	172	168	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	629	597	883	629	597	927	1414	-	-	1460	-	-
Stage 1	836	761	-	850	772	-	-	-	-	-	-	-
Stage 2	844	770	-	830	759	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	616	590	883	616	590	927	1414	-	-	1460	-	-
Mov Cap-2 Maneuver	616	590	-	616	590	-	-	-	-	-	-	-
Stage 1	828	760	-	842	764	-	-	-	-	-	-	-
Stage 2	826	762	-	819	758	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.6			10.7			0.8			0.1		
HCM LOS	A			В			3.0			3.1		
	,,											
Minor Lane/Major Mvm	ıt	NBL	NBT	NDD	EBLn1V	MRI n1	SBL	SBT	SBR			
	ı	1414			803	671	1460	JDI -	JUK			
Capacity (veh/h) HCM Lane V/C Ratio			-	-	0.018			-	-			
		0.01 7.6	-		9.6	10.7	7.5	-	-			
HCM Control Delay (s) HCM Lane LOS			-	-	9.6 A			-	-			
HCM 95th %tile Q(veh)	\	A 0	-	-	0.1	0.2	A 0	-	-			
FIGIVI 7501 70016 Q(VEH)		U	-	-	0.1	0.2	U	-	-			

Intersection							
Int Delay, s/veh	1.1						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	7	1	HOR	<u> </u>	<u> </u>	
Traffic Vol, veh/h	45	7	353	54	8	182	
Future Vol, veh/h	45	7	353	54	8	182	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	- -	None	-		-	None	
Storage Length	0	0	_	-	50	-	
Veh in Median Storage		-	0	-	-	0	
Grade, %	., ,, 0	_	0	_	_	0	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	49	8	384	59	9	198	
IVIVIIIL I IOW	47	0	304	37	7	170	
	Minor1		Major1	<u> </u>	Major2		
Conflicting Flow All	630	414	0	0	443	0	
Stage 1	414	-	-	-	-	-	
Stage 2	216	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	-	2.218	-	
Pot Cap-1 Maneuver	446	638	-	-	1117	-	
Stage 1	667	-	-	-	-	-	
Stage 2	820	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	442	638	-	-	1117	-	
Mov Cap-2 Maneuver	531	-	-	-	-	-	
Stage 1	667	_	-	_	-	-	
Stage 2	813		_	-		_	
Olago Z	010						
	14/5		ND		0.0		
Approach	WB		NB		SB		
HCM Control Delay, s	12.3		0		0.3		
HCM LOS	В						
Minor Lane/Major Mvm	nt	NBT	NBRV	NBLn1V	VBLn2	SBL	
Capacity (veh/h)			_			1117	
HCM Lane V/C Ratio		_	_	0.092			
HCM Control Delay (s)		_	-	12.5	10.7	8.2	
HCM Lane LOS		_	_	В	В	A	
HCM 95th %tile Q(veh))	-	-	0.3	0	0	

Intersection													nulti-way stop.
Int Delay, s/veh	3.7								endatior stop-co			ection is	that the WB
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ሻ	f)			4			4			4		
Traffic Vol, veh/h	29	38	29	0	31	0	24	0	0	0	0	24	
Future Vol, veh/h	29	38	29	0	31	0	24	0	0	0	0	24	
Conflicting Peds, #/hr	0	0	0	0	0 \	V 0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	<u>.</u>	None	-		None	
Storage Length	0	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage	.,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	32	41	32	0	34	0	26	0	0	0	0	26	
Major/Minor N	Major1			Major2			Minor1		_N	Minor2			
Conflicting Flow All	34	0	0	73	0	0	168	155	57	155	171	34	
Stage 1	J 4	-	0	13	-	-	121	121	-	34	34	-	
Stage 2	_			_	_	_	47	34	_	121	137	_	
Critical Hdwy	4.12	_		4.12	-	_	7.12	6.52	6.22	7.12	6.52	6.22	
Critical Hdwy Stg 1	4.12	-	-	4.12	-	-	6.12	5.52	0.22	6.12	5.52	0.22	
Critical Hdwy Stg 2	-			-	-	-	6.12	5.52	-	6.12	5.52	-	
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1578	-	_	1527	-	-	796	737	1009	812	722	1039	
•	1370	-	-	1327	-	-	883	796	1009	982	867	1039	
Stage 1	-	-	-	-	-	-	967	867					
Stage 2	-	-	-	-	-	-	907	807	-	883	783	-	
Platoon blocked, %	1570	-	-	1507	-	-	7/1	722	1000	000	700	1020	
Mov Cap-1 Maneuver	1578	-	-	1527	-	-	764	722	1009	800	708	1039	
Mov Cap-2 Maneuver	-	-	-	-	-	-	764	722	-	800	708	-	
Stage 1	-	-	-	-	-	-	865	780	-	962	867	-	
Stage 2	-	-	-	-	-	-	943	867	-	865	767	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	2.2			0			9.9			8.6			
HCM LOS							Α			А			
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:					
Capacity (veh/h)		764	1578	-	-	1527	-		1039				
HCM Lane V/C Ratio		0.034	0.02	-	-	-	-	-	0.025				
HCM Control Delay (s)		9.9	7.3	-	-	0	-	-	8.6				
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α				
HCM 95th %tile Q(veh)		0.1	0.1	-	-	0	-	-	0.1				

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽	LDI	ሻ	<u>₩</u>	¥	HOR
Traffic Vol, veh/h	324	4	4	199	3	3
Future Vol, veh/h	324	4	4	199	3	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		- -	None
Storage Length	_	-	75	-	0	- INOTIC
Veh in Median Storage,		_	-	0	0	_
Grade, %	0	-	-	0	0	-
					92	92
Peak Hour Factor	92	92	92	92		
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	352	4	4	216	3	3
Major/Minor M	lajor1	N	Major2	1	Minor1	
Conflicting Flow All	0	0	356	0	578	354
Stage 1	-	-	-	-	354	-
Stage 2	_	_	_	_	224	_
Critical Hdwy	_	_	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	_	7.12	_	5.42	- 0.22
Critical Hdwy Stg 2	-	_	_		5.42	_
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver		_	1203		478	690
	-	-	1203	-	710	090
Stage 1	-	-		-		
Stage 2	-	-	-	-	813	-
Platoon blocked, %	-	-	1000	-	4	100
Mov Cap-1 Maneuver	-	-	1203	-	477	690
Mov Cap-2 Maneuver	-	-	-	-	561	-
Stage 1	-	-	-	-	710	-
Stage 2	-	-	-	-	811	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		10.9	
,	U		0.2		_	
HCM LOS					В	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		619	-	-	1203	_
HCM Lane V/C Ratio		0.011	_		0.004	_
HCM Control Delay (s)		10.9	_	-	8	_
HCM Lane LOS		В	_	_	A	_
HCM 95th %tile Q(veh)		0	-	-	0	-

Intersection						
Int Delay, s/veh	8.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7		ሻ	11.	ሻ	7
Traffic Vol, veh/h	33	4	186	30	11	349
Future Vol, veh/h	33	4	186	30	11	349
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- -	None
Storage Length	_	-	100	-	100	0
Veh in Median Storag	e,# 0	_	-	0	0	-
Grade, %	0	-	-	0	0	
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	5	221	36	13	415
Major/Minor	Major1		Major2	N	Minor1	
Conflicting Flow All	0	0	44	0	520	42
Stage 1	-	-	-	_	42	_
Stage 2	-	_	_	-	478	_
Critical Hdwy	-	_	4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	_	-	_	5.42	-
Critical Hdwy Stg 2	_	_	-	-	5.42	_
Follow-up Hdwy	_	_	2.218		3.518	
Pot Cap-1 Maneuver	_	-		-	516	1029
Stage 1	_	_	1304	_	980	1027
Stage 2	-	-	_	-	624	
Platoon blocked, %		-	-		024	-
	-	-	15/1	-	442	1000
Mov Cap-1 Maneuver		-	1564	-	443	1029
Mov Cap-2 Maneuver		-	-	-	477	-
Stage 1	-	-	-	-	980	-
Stage 2	-	-	-	-	536	-
Approach	EB		WB		NB	
HCM Control Delay, s			6.6		10.9	
HCM LOS	U		0.0		В	
HOW EOS					J	
Minor Lane/Major Mvr	nt 1	VBLn1 I		EBT	EBR	WBL
Capacity (veh/h)		477	1029	-	-	1564
HCM Lane V/C Ratio		0.027	0.404	-	-	0.142
HCM Control Delay (s)	12.8	10.8	-	-	7.7
HCM Lane LOS		В	В	-	-	Α
HCM 95th %tile Q(veh	1)	0.1	2	-	-	0.5

HCM 95th %tile Q(veh)

0.1

0.1

Intersection												
Int Delay, s/veh	1.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ሻ	f)			44			44	
Traffic Vol, veh/h	21	354	8	4	168	15	7	0	3	26	0	38
Future Vol, veh/h	21	354	8	4	168	15	7	0	3	26	0	38
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	-	-	-	-	-	-
Veh in Median Storage		0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	92	92	84	84	92	92	92	84	92	84
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	25	421	9	4	200	18	8	0	3	31	0	45
Major/Minor	Major1		ı	Major2		1	Minor1		1	Minor2		
Conflicting Flow All	218	0	0	430	0	0	716	702	426	694	697	209
Stage 1	210	-	U	430	-	-	476	476	420	217	217	209
Stage 2	-	-	-	-	-	-	240	226	-	477	480	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	4.12	-		4.12	-	-	6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	_
Follow-up Hdwy	2.218	-		2.218	-		3.518	4.018	3.318	3.518	4.018	
Pot Cap-1 Maneuver	1352	-	-	1129	-	-	345	362	628	357	365	831
Stage 1	1332	-		1127	-	-	570	557	020	785	723	001
Stage 2	-	-	-	-	-	-	763	717	-	569	554	_
Platoon blocked, %	-	-	-	-	-	-	103	/ 1 /	-	309	554	-
Mov Cap-1 Maneuver	1352	-	-	1129	-	-	321	354	628	349	357	831
Mov Cap-2 Maneuver	1332	-	-	1129	-	-	321	354	020	349	357	031
Stage 1	-	-	-	-	-		560	547		771	720	-
•	-	-	-	-	-	-	719	714	-	556	544	-
Stage 2	-	-	-	-	-	-	/ 19	/ 14	-	220	544	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.2			14.9			12.9		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		376	1352			1129		-	532			
HCM Lane V/C Ratio			0.018	_		0.004	_	_	0.143			
HCM Control Delay (s)		14.9	7.7	_	-	8.2	-	-				
HCM Lane LOS		В	Α.,	_		Α	_	_	В			
TOW LONG LOO		U	/١			, ,			ט			

0.5

Intersection						
Int Delay, s/veh	7.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				
Lane Configurations	270	0	127	172	ነ	214
Traffic Vol, veh/h	378 378	8	137	172	12	316
Future Vol, veh/h		8	137	172	12	316
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	100	None	100	None
Storage Length	-	-	100	-	100	0
Veh in Median Storage,		-	-	0	1	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	434	9	157	198	14	363
Major/Minor N	1ajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	443	0	951	439
Stage 1	-	-	-	-	439	-
Stage 2	_	_	_	_	512	_
Critical Hdwy	_		4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	_	4.12	_	5.42	0.22
Critical Hdwy Stg 2	_		-	_	5.42	-
Follow-up Hdwy	-	-	2.218			3.318
Pot Cap-1 Maneuver	-		1117	-	288	618
Stage 1	-	-	1117	-	650	-
	-	-	-		602	
Stage 2	-	-	-	-	002	-
Platoon blocked, %	-	-	1117	-	247	/10
Mov Cap-1 Maneuver	-	-	1117	-	247	618
Mov Cap-2 Maneuver	-	-	-	-	373	-
Stage 1	-	-	-	-	650	-
Stage 2	-	-	-	-	517	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		3.9		18.7	
HCM LOS	U		0.7		C	
TICIVI EOS						
NA: 1 /NA: NA 1		UDI 41	VIDI O	EDT	EDD	MDI
Minor Lane/Major Mvmt	i	VBLn1 I		EBT	EBR	WBL
Capacity (veh/h)		373	618	-		1117
HCM Lane V/C Ratio		0.037		-	-	0.141
HCM Control Delay (s)		15	18.8	-	-	8.8
HCM Lane LOS		С	С	-	-	Α
HCM 95th %tile Q(veh)		0.1	3.8	-	-	0.5
2(1011)		J	5.5			3.0

	•	_	_	_	—	•	•	<u></u>	<u> </u>	_	1	
Mayamant		EDT	▼	WDI	WDT	WDD	NDI NDI	NDT	, NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	10	7	ካካ	↑	427	\	^	7	້ ງ	^	10/
Traffic Volume (veh/h)	134 134	29 29	57 57	94 94	43 43	437 437	95 95	2055 2055	88 88	303 303	1561 1561	106 106
Future Volume (veh/h)	134 7	29 4	14	94	43 8	43 <i>1</i> 18	95 1		16	303 5	2	12
Number Initial Q (Qb), veh	0	0	0	0	0	0	0	6 0	0	0	0	0
` '	1.00	U	1.00	1.00	U	1.00	1.00	U	1.00	1.00	U	1.00
Ped-Bike Adj(A_pbT) Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	144	31	38	1003	46	170	1003	2210	59	326	1678	71
Adj No. of Lanes	2	1	1	2	1	170	102	2210	1	1	2	1
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73	0.73
Cap, veh/h	189	223	189	144	199	169	185	1988	890	257	2148	961
Arrive On Green	0.05	0.12	0.12	0.04	0.11	0.11	0.08	1.00	1.00	0.08	0.61	0.61
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	1774	3539	1583	1774	3539	1583
	144	31	38	101	46	170	102	2210	59	326		71
Grp Volume(v), veh/h	1721	1863	1583	1721	1863	1583	1774	1770	1583	320 1774	1678 1770	1583
Grp Sat Flow(s),veh/h/ln Q Serve(q_s), s	6.2	2.2	3.2	4.3	3.4	16.0	3.7	84.3	0.0	12.5	53.2	2.8
·0= /	6.2	2.2	3.2	4.3	3.4	16.0	3.7	84.3	0.0	12.5	53.2	2.8
Cycle Q Clear(g_c), s Prop In Lane	1.00	2.2	1.00	1.00	3.4	1.00	1.00	04.3	1.00	1.00	55.2	1.00
Lane Grp Cap(c), veh/h	189	223	1.00	1.00	199	1.00	1.00	1988	890	257	2148	961
V/C Ratio(X)	0.76	0.14	0.20	0.70	0.23	1.01	0.55	1,11	0.07	1.27	0.78	0.07
Avail Cap(c_a), veh/h	229	224	190	184	199	1.01	253	1988	890	257	2148	961
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.09	0.09	0.09	1.00	1.00	1.00
Uniform Delay (d), s/veh	69.9	59.1	59.6	70.9	61.4	67.0	23.0	0.09	0.09	37.2	22.0	12.1
Incr Delay (d2), s/veh	10.5	0.3	0.6	6.8	0.7	71.0	0.2	50.9	0.0	146.9	22.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.7	0.0	0.2	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.2	1.2	1.5	2.2	1.8	10.3	1.9	14.1	0.0	21.2	26.7	1.2
LnGrp Delay(d),s/veh	80.4	59.5	60.2	77.8	62.1	138.0	23.1	50.9	0.0	184.2	24.9	12.3
LnGrp LOS	F	57.5 E	60.2 E	77.0 E	02.1 E	F	23.1 C	50.7 F	Α	F	24.7 C	12.3 B
Approach Vol, veh/h		213		<u> </u>	317			2371		'	2075	
Approach Delay, s/veh		73.7			107.8			48.5			49.5	
Approach LOS		73.7 E			107.6 F			40.5 D			49.5 D	
• •											D	
Timer	1	2	3	4	5	6	/	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.2	99.5	13.3	23.9	20.0	92.8	15.2	22.0				
Change Period (Y+Rc), s	7.5	8.5	7.0	6.0	7.5	8.5	7.0	6.0				
Max Green Setting (Gmax), s	11.5	83.5	8.0	18.0	12.5	82.5	10.0	16.0				
Max Q Clear Time (g_c+l1), s	5.7	55.2	6.3	5.2	14.5	86.3	8.2	18.0				
Green Ext Time (p_c), s	0.1	28.3	0.0	1.1	0.0	0.0	0.1	0.0				
Intersection Summary			F0.0									
HCM 2010 Ctrl Delay			53.8									
HCM 2010 LOS			D									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1,4	+	7	ሻሻ		7	ሻሻ	^	7	ሻሻ	^	7
Traffic Volume (veh/h)	211	20	102	96	25	299	161	1742	88	255	1227	214
Future Volume (veh/h)	211	20	102	96	25	299	161	1742	88	255	1227	214
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	234	22	70	107	28	206	179	1936	98	283	1363	238
Adj No. of Lanes	2	1	1	2	1	1	2	2	1	2	2	1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	184	240	204	153	224	190	224	1899	850	264	1940	868
Arrive On Green	0.05	0.13	0.13	0.04	0.12	0.12	0.07	0.54	0.54	0.15	1.00	1.00
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	3442	3539	1583	3442	3539	1583
Grp Volume(v), veh/h	234	22	70	107	28	206	179	1936	98	283	1363	238
Grp Sat Flow(s),veh/h/ln	1721	1863	1583	1721	1863	1583	1721	1770	1583	1721	1770	1583
Q Serve(g_s), s	8.0	1.6	6.0	4.6	2.0	18.0	7.7	80.5	4.6	11.5	0.0	0.0
Cycle Q Clear(g_c), s	8.0	1.6	6.0	4.6	2.0	18.0	7.7	80.5	4.6	11.5	0.0	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	184	240	204	153	224	190	224	1899	850	264	1940	868
V/C Ratio(X)	1.27	0.09	0.34	0.70	0.13	1.08	0.80	1.02	0.12	1.07	0.70	0.27
Avail Cap(c_a), veh/h	184	240	204	298	224	190	264	1899	850	264	1940	868
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.43	0.43	0.43	0.52	0.52	0.52
Uniform Delay (d), s/veh	71.0	57.6	59.5	70.7	59.0	66.0	69.1	34.8	17.2	63.5	0.0	0.0
Incr Delay (d2), s/veh	159.0	0.2	1.0	5.7	0.2	89.5	6.4	18.7	0.1	60.9	1.1	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	7.9	0.8	2.7	2.3	1.0	12.6	3.9	44.0	2.0	7.7	0.3	0.1
LnGrp Delay(d),s/veh	230.0	57.7	60.5	76.4	59.2	155.5	75.5	53.5	17.3	124.4	1.1	0.4
LnGrp LOS	F	E	E	E	E	F	E	F	В	F	A	A
Approach Vol, veh/h	•	326			341			2213		•	1884	
Approach Delay, s/veh		182.0			122.8			53.7			19.6	
Approach LOS		F			F			55.7 D			19.0 B	
											ь	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	18.3	91.7	13.7	26.3	20.0	90.0	15.0	25.0				
Change Period (Y+Rc), s	8.5	9.5	7.0	7.0	8.5	9.5	7.0	7.0				
Max Green Setting (Gmax), s	11.5	80.5	13.0	13.0	11.5	80.5	8.0	18.0				
Max Q Clear Time (g_c+l1), s	9.7	2.0	6.6	8.0	13.5	82.5	10.0	20.0				
Green Ext Time (p_c), s	0.1	65.1	0.1	0.5	0.0	0.0	0.0	0.0				
Intersection Summary												
intoroccion caminary												
HCM 2010 Ctrl Delay			53.9									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1,1	†	7	75	^	7	ሻሻ	^	7	ሻሻ	^	7
Traffic Volume (veh/h)	145	6	183	43	5	75	226	1791	46	81	1239	106
Future Volume (veh/h)	145	6	183	43	5	75	226	1791	46	81	1239	106
Number	7	4	14	3	8	18	1	6	16	5	2	12
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	159	7	201	47	5	82	248	1968	51	89	1362	116
Adj No. of Lanes	2	1	1	2	1	1	2	2	1	2	2	1
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	181	250	212	98	205	174	297	2165	969	131	1995	892
Arrive On Green	0.05	0.13	0.13	0.03	0.11	0.11	0.09	0.61	0.61	0.04	0.56	0.56
Sat Flow, veh/h	3442	1863	1583	3442	1863	1583	3442	3539	1583	3442	3539	1583
Grp Volume(v), veh/h	159	7	201	47	5	82	248	1968	51	89	1362	116
Grp Sat Flow(s),veh/h/ln	1721	1863	1583	1721	1863	1583	1721	1770	1583	1721	1770	1583
Q Serve(g_s), s	7.0	0.5	19.1	2.0	0.4	7.4	10.8	73.9	2.0	3.9	41.5	5.2
Cycle Q Clear(g_c), s	7.0	0.5	19.1	2.0	0.4	7.4	10.8	73.9	2.0	3.9	41.5	5.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	181	250	213	98	205	174	297	2165	969	131	1995	892
V/C Ratio(X)	0.88	0.03	0.95	0.48	0.02	0.47	0.84	0.91	0.05	0.68	0.68	0.13
Avail Cap(c_a), veh/h	181	250	213	113	221	187	389	2165	969	192	1995	892
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.66	0.66	0.66	0.73	0.73	0.73
Uniform Delay (d), s/veh	71.5	57.2	65.3	72.7	60.4	63.5	68.4	25.8	11.8	72.2	23.5	15.6
Incr Delay (d2), s/veh	35.2	0.0	46.5	3.6	0.0	2.0	7.9	4.9	0.1	4.5	1.4	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.2	0.3	11.1	1.0	0.2	3.3	5.5	37.3	0.9	1.9	20.6	2.4
LnGrp Delay(d),s/veh	106.7	57.2	111.8	76.4	60.4	65.5	76.3	30.7	11.9	76.6	24.9	15.8
LnGrp LOS	F	E	F	E	E	E	E	С	В	E	С	B
Approach Vol, veh/h		367			134			2267			1567	
Approach Delay, s/veh		108.5			69.1			35.2			27.2	
Approach LOS		F			Е			D			С	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	21.6	91.7	11.3	27.4	14.3	99.0	15.0	23.7				
Change Period (Y+Rc), s	8.5	6.0	7.0	7.0	8.5	6.0	7.0	7.0				
Max Green Setting (Gmax), s	17.2	80.8	5.0	20.4	8.5	89.5	8.0	18.0				
Max Q Clear Time (g_c+l1), s	12.8	43.5	4.0	21.1	5.9	75.9	9.0	9.4				
Green Ext Time (p_c), s	0.3	33.7	0.0	0.0	0.0	13.0	0.0	0.7				
Intersection Summary												
HCM 2010 Ctrl Delay			39.6									
HCM 2010 LOS			D									

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	*	<u> </u>	₹
Movement EBT	EBR		WBL
Lane Configurations ††	7		ሻሻ
Traffic Volume (veh/h) 1452	21	Volume (veh/h) 1452	98
Future Volume (veh/h) 1452	21	Volume (veh/h) 1452	98
Number 2	12	r 2	1
Initial Q (Qb), veh 0	0	(Qb), veh 0	0
Ped-Bike Adj(A_pbT)	1.00	• •	1.00
Parking Bus, Adj 1.00	1.00		1.00
Adj Sat Flow, veh/h/ln 1863	1863		1863
Adj Flow Rate, veh/h 1545	22		1003
•	1		2
Peak Hour Factor 0.94	0.94		0.94
Percent Heavy Veh, % 2	2	,	2
Cap, veh/h 2515	1125		149
Arrive On Green 0.71	0.71		0.01
Sat Flow, veh/h 3632	1583	w, veh/h 3632	3442
Grp Volume(v), veh/h 1545	22	lume(v), veh/h 1545	104
Grp Sat Flow(s), veh/h/ln1770	1583	t Flow(s),veh/h/ln1770	1721
Q Serve(g_s), s 33.6	0.6		4.5
Cycle Q Clear(q_c), s 33.6	0.6		4.5
Prop In Lane	1.00	.0_ /	1.00
Lane Grp Cap(c), veh/h 2515	1125		149
V/C Ratio(X) 0.61	0.02		0.70
Avail Cap(c_a), veh/h 2515	1125		206
HCM Platoon Ratio 1.00	1.00		0.33
Upstream Filter(I) 0.74	0.74		0.33
Uniform Delay (d), s/veh 11.1	6.4	•	73.0
Incr Delay (d2), s/veh 0.8	0.0		2.0
Initial Q Delay(d3),s/veh 0.0	0.0	Delay(d3),s/veh 0.0	0.0
%ile BackOfQ(50%),veh/ln6.6	0.3	ickOfQ(50%),veh/11/6.6	2.2
LnGrp Delay(d), s/veh 12.0	6.4	Delay(d),s/veh 12.0	75.0
LnGrp LOS B	Α		Е
Approach Vol, veh/h 1567			
Approach Delay, s/veh 11.9			
Approach LOS B			
Approach LOS B		UI LOS B	
Timer 1	2	1	3
Assigned Phs 1	2	ed Phs 1	
Phs Duration (G+Y+Rc), \$4.5	112.1		
Change Period (Y+Rc), s 8.0	5.5	• •	
Max Green Setting (Gmax), &			
Max Q Clear Time (g_c+l16),5s			
Green Ext Time (p_c), s 0.1	59.8		
GIEEH EXLIHIE (P_C), 5 U.T	J 7 .0	_λι Πιπ ε (μ_c), 5 - 0.1	
Intersection Summary		ction Summary	
HCM 2010 Ctrl Delay			33.2
HCM 2010 LOS			C
HOW ZOTO LOG		010 200	

		_	_	_	←	A.	•	†	/	\	I	1
Movement	EBL	EBT	EBR	v WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	T)	^	T T	VVDL	↑ ↑	VVDIX	NDE.	<u>ND1</u>	NDK **	<u> </u>	<u> </u>	JUK T
Traffic Volume (veh/h)	133	1162	320	80	1669	174	402	76	23	173	75	46
Future Volume (veh/h)	133	1162	320	80	1669	174	402	76	23	173	75	46
Number	5	2	12	1	6	16	7	4	14	3	8	18
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	Ü	1.00	1.00		1.00	1.00	, ,	1.00	1.00	- U	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863	1863
Adj Flow Rate, veh/h	141	1236	340	85	1776	185	428	81	24	184	80	49
Adj No. of Lanes	1	2	1	1	2	103	2	1	1	2	1	1
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	161	2064	924	289	1998	894	472	217	184	232	99	84
Arrive On Green	0.10	1.00	1.00	0.03	0.56	0.56	0.14	0.12	0.12	0.07	0.05	0.05
Sat Flow, veh/h	1774	3539	1583	1774	3539	1583	3442	1863	1583	3442	1863	1583
Grp Volume(v), veh/h	141	1236	340	85	1776	185	428	81	24	184	80	49
Grp Sat Flow(s), veh/h/lr		1770	1583	1774	1770	1583	1721	1863	1583	1721	1863	1583
Q Serve(g_s), s	5.2	0.0	0.0	3.0	65.8	8.6	18.4	6.0	2.0	7.9	6.4	4.5
Cycle Q Clear(g_c), s	5.2	0.0	0.0	3.0	65.8	8.6	18.4	6.0	2.0	7.9	6.4	4.5
Prop In Lane	1.00	20/4	1.00	1.00	1000	1.00	1.00	217	1.00	1.00	00	1.00
Lane Grp Cap(c), veh/h		2064	924	289	1998	894	472	217	184	232	99	84
V/C Ratio(X)	0.87	0.60	0.37	0.29	0.89	0.21	0.91	0.37	0.13	0.79	0.81	0.58
Avail Cap(c_a), veh/h	171	2064	924	315	1998	894	493	217	184	330	99	84
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.77	0.77	0.77	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh		0.0	0.0	12.8	28.5	16.1	63.8	61.2	59.4	68.9	70.2	69.4
Incr Delay (d2), s/veh	28.6	1.0	0.9	0.6	6.4	0.5	19.8	1.1	0.3	8.3	36.7	9.6
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh		0.3	0.2	1.5	33.8	3.9	10.0	3.2	0.9	4.0	4.3	2.2
LnGrp Delay(d),s/veh	61.0	1.0	0.9	13.3	34.9	16.6	83.6	62.3	59.8		106.9	78.9
LnGrp LOS	E	A	A	В	С	В	F	E	E	E	F	E
Approach Vol, veh/h		1717			2046			533			313	
Approach Delay, s/veh		5.9			32.4			79.3			85.1	
Approach LOS		Α			С			Ε			F	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc)		96.5	17.6	23.5	15.2	93.7	27.1	14.0				
Change Period (Y+Rc),		9.0	7.5	6.0	8.0	9.0	6.5	6.0				
Max Green Setting (Gm		84.4	14.4	14.1	8.0	83.0	21.5	8.0				
Max Q Clear Time (q_c-		2.0	9.9	8.0	7.2	67.8	20.4	8.4				
Green Ext Time (p_c), s		63.5	0.2	0.5	0.0	14.4	0.2	0.0				
Intersection Summary	. 0.0	00.0	J. <u>Z</u>	5.0	5.0		J.2	3.0				
			21 5									
HCM 2010 Ctrl Delay			31.5									
HCM 2010 LOS			С									

Intersection						
Int Delay, s/veh	8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.			4	*	7
Traffic Vol, veh/h	36	15	451	39	12	213
Future Vol, veh/h	36	15	451	39	12	213
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-			None
Storage Length	-	-	_	-	100	0
Veh in Median Storage,	# 0	-	_	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	38	16	475	41	13	224
IVIVIIIL FIOW	30	10	473	41	13	224
Major/Minor M	lajor1	ľ	Major2	N	Minor1	
Conflicting Flow All	0	0	54	0	1037	46
Stage 1	-	-	-	-	46	-
Stage 2	-	-	-	-	991	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1		-	_	_	5.42	_
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	_	_	2.218		3.518	3 318
Pot Cap-1 Maneuver	_	_	1551	_	256	1023
Stage 1	_	_	1001	_	976	1023
Stage 2	-			-	359	-
Platoon blocked, %	-	-	-	-	339	-
		-	1551		176	1023
Mov Cap-1 Maneuver	-	-	1001	-		
Mov Cap-2 Maneuver	-	-	-	-	176	-
Stage 1	-	-	-	-	976	-
Stage 2	-	-	-	-	247	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		7.7		10.4	
HCM LOS	U		7.7		В	
TIOWI LOS					ט	
NA'		UDL 4	UDI C	EDT	EDD	MDI
Minor Lane/Major Mvmt		VBLn1 I		EBT	EBR	WBL
Capacity (veh/h)			1023	-		1551
HCM Lane V/C Ratio		0.072		-	-	0.306
HCM Control Delay (s)		27	9.5	-	-	8.3
HCM Lane LOS		D	Α	-	-	Α
HCM 95th %tile Q(veh)		0.2	8.0	-	-	1.3

2027 Total Traffic - PM Peak Hour

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	61	174	12	6	436	31	11	0	5	28	0	44
Future Vol, veh/h	61	174	12	6	436	31	11	0	5	28	0	44
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None		-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	65	185	13	6	464	33	12	0	5	30	0	47
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	497	0	0	198	0	0	838	831	192	817	821	481
Stage 1	-	-	-	-	-	-	322	322	- 172	493	493	-
Stage 2	_	_	_	_	_	_	516	509	_	324	328	_
Critical Hdwy	4.12	_		4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1				- 1.12	_		6.12	5.52	0.22	6.12	5.52	0.22
Critical Hdwy Stg 2	_	_			_		6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218			2.218	_		3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1067	-		1375	-	-	286	305	850	295	309	585
Stage 1	1007			1373			690	651	- 000	558	547	505
Stage 2	-				-	-	542	538	-	688	647	-
Platoon blocked, %	-					-	J42	330		000	047	
Mov Cap-1 Maneuver	1067	-		1375	-	-	248	282	850	276	286	585
Mov Cap-1 Maneuver	1007			1373		-	248	282	- 000	276	286	505
Stage 1		-			-	-	642	606	-	519	544	-
Stage 2	-						496	535	-	637	602	-
Stayt 2		-	-	-	-	_	470	555	-	037	002	-
A way was a sla	ED			WD			NID			CD		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.1			0.1			17			15.9		
HCM LOS							С			С		
N Alice and Laure (N A. 1		NDL 4	EDI	EDT	ED.	14/51	MOT	WDD	CDL 4			
Minor Lane/Major Mvm	IT	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :				
Capacity (veh/h)		318	1067	-		1375	-	-	408			
HCM Lane V/C Ratio		0.054		-	-	0.005	-		0.188			
HCM Control Delay (s)		17	8.6	0	-	7.6	0	-				
HCM Lane LOS		С	A	Α	-	A	Α	-	C			
HCM 95th %tile Q(veh))	0.2	0.2	-	-	0	-	-	0.7			

Intersection							
Int Delay, s/veh	4.3						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	ľ
Lane Configurations	₽			4	ሻ	7	
Traffic Vol, veh/h	178	29	312	459	8	206	
Future Vol, veh/h	178	29	312	459	8	206	
Conflicting Peds, #/hr	0	0	0	0	0	0	
	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-		-		-	None	
Storage Length	-	-	-	-	100	0	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	_	_	0	0	_	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	187	31	328	483	8	217	
IVIVIIIL I IOW	107	31	320	403	Ü	217	
Major/Minor M	lajor1	N	Major2	N	Minor1		
Conflicting Flow All	0	0	218	0	1342	203	
Stage 1	-	-	-	-	203	-	
Stage 2	-	-	-	-	1139	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1352	-	168	838	
Stage 1	-	-	-	-	831	-	
Stage 2	-	-	-	-	305	-	
Platoon blocked, %	_	-		-			
Mov Cap-1 Maneuver	-	_	1352	-	112	838	
Mov Cap-2 Maneuver	_	_	-	_	112	-	
Stage 1	_	_	-	_	831	_	
Stage 2	_	_	_	_	204	_	
Stage 2					201		
Approach	EB		WB		NB		
HCM Control Delay, s	0		3.4		11.9		
HCM LOS					В		
Minor Lane/Major Mvmt	ı	NBLn1 N	VIRI n2	EBT	EBR	WBL	Į
		112				1352	1
Capacity (veh/h)			838	-			
HCM Central Delay (c)		0.075		-		0.243	
HCM Long LOS		39.7	10.8	-	-	8.5	
HCM Lane LOS		Е	В	-	-	Α	
HCM 95th %tile Q(veh)		0.2	1	_	_	1	

10W 2010 1 W00	Olait i aik
9: Robert Rose Dr/Pvt Dr & Willowoak Tr	2027 Total Traffic - PM Peak Hour

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	î,		ሻ	ĵ.		ሻ	f)			4	
Traffic Vol, veh/h	0	174	124	61	266	2	109	1	37	9	5	0
Future Vol, veh/h	0	174	124	61	266	2	109	1	37	9	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	90	-	-	100	-	-	-	-	-
Veh in Median Storage	:,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	215	153	75	328	2	135	1	46	11	6	0
Major/Minor N	Major1		<u> </u>	Major2			Minor1			Minor2		
Conflicting Flow All	330	0	0	368	0	0	774	772	292	794	847	329
Stage 1	-	-	-	-	-	-	292	292	-	479	479	-
Stage 2	-	-	-	-	-	-	482	480	-	315	368	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018		3.518	4.018	3.318
Pot Cap-1 Maneuver	1229	-	-	1191	-	-	316	330	747	306	299	712
Stage 1	-	-	-	-	-	-	716	671	-	568	555	-
Stage 2	-	-	-	-	-	-	565	554	-	696	621	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1229	-	-	1191	-	-	296	309	747	273	280	712
Mov Cap-2 Maneuver	-	-	-	-	-	-	296	309	-	273	280	-
Stage 1	-	-	-	-	-	-	716	671	-	568	520	-
Stage 2	-	-	-	-	-	-	523	519	-	652	621	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			1.5			22.6			19		
HCM LOS							С			С		
Minor Lane/Major Mvm	ıt ſ	NBLn1 I	VBLn2	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1		
Capacity (veh/h)		296	720	1229	-	-	1191	-	-	275		
HCM Lane V/C Ratio		0.455		-	-	-	0.063	-	-	0.063		
HCM Control Delay (s)		26.9	10.3	0	-	-	8.2	-	-	19		
HCM Lane LOS		D	В	A	-	-	Α	-	-	С		
HCM 95th %tile Q(veh))	2.2	0.2	0	-	-	0.2	-	-	0.2		

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EBK				SBK
Lane Configurations	¥	г	<u></u>	4	102	/
Traffic Vol, veh/h	5	5	6	139	182	6
Future Vol, veh/h	5	5	6	139	182	6
Conflicting Peds, #/hr	0	0	0	_ 0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	5	7	151	198	7
D. A	N4' 0				4 ' 0	
	Minor2		Major1		/lajor2	
Conflicting Flow All	367	202	205	0	-	0
Stage 1	202	-	-	-	-	-
Stage 2	165	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	633	839	1366	-	-	-
Stage 1	832	-	-	-	-	-
Stage 2	864	-	_	_	-	_
Platoon blocked, %	00.			_	_	_
Mov Cap-1 Maneuver	630	839	1366	_	_	_
Mov Cap-1 Maneuver	673	037	1300			
	828		-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	864	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.9		0.3		0	
HCM LOS	A		0.0			
TIOM EGG	,,					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1366	-	747	-	-
HCM Lane V/C Ratio		0.005	-	0.015	-	-
HCM Control Delay (s))	7.6	0	9.9	-	-
HCM Lane LOS		Α	A	Α	-	_
HCM 95th %tile Q(veh)	0	-	0	_	_
HOW JULY JULIE Q(VEH	/	U		U		_

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ች	ĵ.		ሻ	(î	
Traffic Vol, veh/h	5	0	5	14	0	1	6	196	22	5	177	6
Future Vol, veh/h	5	0	5	14	0	1	6	196	22	5	177	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	5	0	5	15	0	1	7	213	24	5	192	7
Major/Minor N	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	446	457	196	447	448	225	199	0	0	237	0	0
Stage 1	206	206	-	239	239	-	-	-	-		-	-
Stage 2	240	251	_	208	209	_	_	_	-	_	_	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	_	_	-	_	
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218	_	_
Pot Cap-1 Maneuver	523	500	845	522	506	814	1373	-	-	1330	-	-
Stage 1	796	731	-	764	708	-	-	_	_	-	_	_
Stage 2	763	699	-	794	729	-	-	-	-	_	-	-
Platoon blocked, %								-	-		_	
Mov Cap-1 Maneuver	519	496	845	515	501	814	1373	-	-	1330	-	-
Mov Cap-2 Maneuver	519	496	-	515	501	-	-	_	_	-	_	_
Stage 1	792	728	-	760	704	-	-	-	-	-	-	-
Stage 2	758	696	_	786	726	_	_	-	_	_	_	_
5 -	. 55	3,3		. 55	. 23							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.7			12			0.2			0.2		
HCM LOS	В			В			0.2			0,2		
1.5W E00	J			, ,								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	EBLn1V	WRI n1	SBL	SBT	SBR			
Capacity (veh/h)	IL	1373	-	NDIX		528	1330	-	- JUIX			
HCM Lane V/C Ratio		0.005	_		0.017		0.004	-	-			
HCM Control Delay (s)		7.6	-	-		12	7.7	-	-			
HCM Lane LOS		7.0 A	-	-	В	12 B	Α.	-	-			
HCM 95th %tile Q(veh))	0	-	-	0.1	0.1	0	-	-			
HOW 75th 70the Q(VeH)		U	-	-	0.1	0.1	U		-			

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	(Î	
Traffic Vol, veh/h	5	0	16	22	0	0	17	218	34	7	183	6
Future Vol, veh/h	5	0	16	22	0	0	17	218	34	7	183	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	-
Veh in Median Storage	:,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	5	0	17	24	0	0	18	237	37	8	199	7
Major/Minor I	Minor2			Minor1			Major1		ľ	Major2		
Conflicting Flow All	511	529	203	519	514	256	206	0	0	274	0	0
Stage 1	219	219	-	292	292	-	-	-	-	-	-	-
Stage 2	292	310	-	227	222	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	473	455	838	467	464	783	1365	-	-	1289	-	-
Stage 1	783	722	-	716	671	-	-	-	-	-	-	-
Stage 2	716	659	-	776	720	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	466	446	838	451	455	783	1365	-	-	1289	-	-
Mov Cap-2 Maneuver	466	446	-	451	455	-	-	-	-	-	-	-
Stage 1	773	718	-	707	662	-	-	-	-	-	-	-
Stage 2	707	650	-	755	716	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.3			13.4			0.5			0.3		
HCM LOS	В			В			3.0			3.0		
Minor Lane/Major Mvm	nt	NBL	NBT	NRD	EBLn1V	MRI n1	SBL	SBT	SBR			
Capacity (veh/h)	It	1365	IND I	NDK -	704	451	1289	JDI -	JUK			
HCM Lane V/C Ratio		0.014			0.032			-	-			
HCM Control Delay (s)		7.7	-	-	10.3	13.4	7.8	-	-			
HCM Lane LOS		Λ./	-	-	10.3 B	13.4 B	7.0 A	-	_			
HCM 95th %tile Q(veh))	0	-	-	0.1	0.2	0	-	-			
HOW 75th 70the Q(VeH)		U		_	0.1	0.2	U	-	-			

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	WDR	IND I	NDK	3DL 1	<u>361</u>
Traffic Vol, veh/h	1 70	ր 11	215	75	1 2	T 455
Future Vol, veh/h	70	11	215	75	12	455
Conflicting Peds, #/hr	0	0	0	0	0	455
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	310p	None	-	None	-	None
Storage Length	0	0	-	-	50	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	_	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	76	12	234	82	13	495
IVIVIIIL FIOW	70	12	234	02	13	493
Major/Minor N	Vinor1		Major1	N	Major2	
Conflicting Flow All	796	275	0	0	316	0
Stage 1	275	-	-	-	-	-
Stage 2	521	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	-	-	2.218	-
Pot Cap-1 Maneuver	356	764	-	-	1244	-
Stage 1	771	-	-	-	-	-
Stage 2	596	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	352	764	-	-	1244	-
Mov Cap-2 Maneuver	460	-	-	-	-	-
Stage 1	771	-	-	-	-	-
Stage 2	590	-	-	-	-	-
·						
Approach	WB		NB		SB	
			0		0.2	
HCM Control Dolay c	100		U		0.2	
HCM LOS	13.8					
HCM Control Delay, s HCM LOS	13.8 B					
HCM LOS	В					
HCM LOS Minor Lane/Major Mvm	В	NBT	NBRV	VBLn1V		SBL
HCM LOS Minor Lane/Major Mvm Capacity (veh/h)	В	NBT -	-	460	764	1244
HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	B ut	NBT - -	-	460 0.165	764 0.016	1244 0.01
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	B ut	-	-	460 0.165 14.4	764 0.016 9.8	1244 0.01 7.9
HCM LOS Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	B ut	- -	-	460 0.165	764 0.016	1244 0.01

14: Clari Park Access #4/Clari Park Access #3 & Willowoak Trail/Willow@ak ThiaTraffic - PM Peak Hour

Intersection												
Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ነ		7	7	₽			ની	7		4	
Traffic Vol, veh/h	6	164	133	133	230	12	124	0	124	11	0	5
Future Vol, veh/h	6	164	133	133	230	12	124	0	124	11	0	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	0	100	-	-	-	-	100	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	7	178	145	145	250	13	135	0	135	12	0	5
Major/Minor I	Major1		ا	Major2		ا	Minor1		N	Minor2		
Conflicting Flow All	263	0	0	323	0	0	741	745	178	879	884	257
Stage 1	-	-	-	-	-	-	192	192	-	547	547	-
Stage 2	-	-	-	-	-	-	549	553	-	332	337	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1301	-	-	1237	-	-	332	342	865	268	284	782
Stage 1	-	-	-	-	-	-	810	742	-	521	517	-
Stage 2	-	-	-	-	-	-	520	514	-	681	641	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1301	-	-	1237	-	-	299	301	865	205	250	782
Mov Cap-2 Maneuver	-	-	-	-	-	-	299	301	-	205	250	-
Stage 1	-	-	-	-	-	-	806	738	-	518	457	-
Stage 2	-	-	-	-	-	-	456	454	-	572	638	-
ŭ												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			2.9			18.3			19.5		
HCM LOS							С			С		
Minor Lane/Major Mvm	nt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1		
Capacity (veh/h)		299	865	1301	_		1237	_	_	266		
HCM Lane V/C Ratio		0.451	0.156		_		0.117	_	_	0.065		
HCM Control Delay (s)		26.6	9.9	7.8	-	-	8.3	-	_			
HCM Lane LOS		20.0 D	Α.,	Α.	_	_	Α	_	_	C		
HCM 95th %tile Q(veh))	2.2	0.6	0	_	_	0.4	_	_	0.2		
		۷.۲	0.0				0. 1			0.2		

Intersection													ulti-way sto	
Int Delay, s/veh	4							ecomme also be				ction is t	hat the WB	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	Ť	(î			4			4			4			
Traffic Vol, veh/h	40	52	40	0	48	0	38	0	0	0	0	38		
Future Vol, veh/h	40	52	40	0	48	0	38	0	0	0	0	38		
Conflicting Peds, #/hr	0	0	0	0	0\	/ 0	0	0	0	0	0	0		
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop		
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None		
Storage Length	0	-	-	-	-	-	-	-	-	-	-	-		
Veh in Median Storage	.,# -	0	-	-	0	-	-	0	-	-	0	-		
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-		
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92		
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2		
Mvmt Flow	43	57	43	0	52	0	41	0	0	0	0	41		
Major/Minor N	Major1			Major2		N	/linor1		_	Minor2				
Conflicting Flow All	52	0	0	100	0	0	238	217	79	217	238	52		
Stage 1	-	-	-	-	-	-	165	165	-	52	52	-		
Stage 2	_		_	_		_	73	52	_	165	186			
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22		
Critical Hdwy Stg 1	-	_	_	-	_	_	6.12	5.52	0.22	6.12	5.52	0.22		
Critical Hdwy Stg 2	_		_	_		_	6.12	5.52	-	6.12	5.52	-		
Follow-up Hdwy	2.218	_	_	2.218	_	_	3.518		3.318	3.518	4.018	3.318		
Pot Cap-1 Maneuver	1554	_	_	1493	_	_	716	681	981	739	663	1016		
Stage 1	-		_	1475	_	_	837	762	-	961	852	-		
Stage 2			_		_	_	937	852	-	837	746	_		
Platoon blocked, %			_		_	_	701	002		007	740			
Mov Cap-1 Maneuver	1554		_	1493		_	672	662	981	723	644	1016		
Mov Cap-1 Maneuver	1334	_	_	1475	_	_	672	662	701	723	644	1010		
Stage 1	_			_		_	814	741	-	934	852	_		
Stage 2				_			899	852	-	814	725	-		
Jiage Z	-			-			077	002		014	123			
Annroach	ED			MD			ND			CD				
Approach	EB			WB			NB			SB				
HCM Control Delay, s	2.2			0			10.7			8.7				
HCM LOS							В			А				
Minor Lane/Major Mvm	ıt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SRI n1					
Capacity (veh/h)	it I		1554	EDI -	EDK -	1493	VVDI		1016					
HCM Lane V/C Ratio		0.061		-	-	1473	-		0.041					
HCM Control Delay (s)		10.7	7.4	-	-	0		-	8.7					
HCM Lane LOS		10.7 B		-			-							
			A	-	-	A	-	-	A 0.1					
HCM 95th %tile Q(veh)		0.2	0.1	-	-	0	-	-	U. I					

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDI	WBL		NDL W	אטוז
Traffic Vol, veh/h	292	4		↑ 369		5
Future Vol, veh/h	292	6	6	369	5	5
		6	6		5	
Conflicting Peds, #/hr	0	0	0	0	O Cton	O Cton
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	75	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	317	7	7	401	5	5
Major/Minor M	laior1	N	Majora	N	/linor1	
	lajor1		Major2			221
Conflicting Flow All	0	0	324	0	736	321
Stage 1	-	-	-	-	321	-
Stage 2	-	-	-	-	415	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1236	-	386	720
Stage 1	-	-	-	-	735	-
Stage 2	-	-	-	-	666	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1236	-	384	720
Mov Cap-2 Maneuver	-	-	-	-	492	-
Stage 1	_	_	-	_	735	_
Stage 2	_	_	_	_	662	_
Olugo 2					002	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		11.3	
HCM LOS					В	
		IDI. 1		E55	11151	14/5-
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		585	-		1236	-
HCM Lane V/C Ratio		0.019	-	-	0.005	-
HCM Control Delay (s)		11.3	-	-	7.9	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-
,						

Intersection						
Int Delay, s/veh	7.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		ሻ	<u>₩</u>	ሻ	7
Traffic Vol, veh/h	36	15	451	39	12	213
Future Vol, veh/h	36	15	451	39	12	213
Conflicting Peds, #/hr	0	0	0	0	0	0
· ·	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	100	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	38	16	475	41	13	224
Major/Minor M	ajor1	ľ	Major2	1	Vinor1	
Conflicting Flow All	0	0	54	0	1037	46
Stage 1	-	-	-	-	46	-
Stage 2	-	-	-	-	991	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1551	-	256	1023
Stage 1	-	-	-	-	976	-
Stage 2	-	-	-	-	359	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1551	-	178	1023
Mov Cap-2 Maneuver	-	-	-	-	222	-
Stage 1	-	-	-	-	976	-
Stage 2	-	-	-	-	249	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		7.7		10.2	
HCM LOS					В	
Minor Lane/Major Mvmt	ľ	NBLn1 ľ	\IRI n2	EBT	EBR	WBL
Capacity (veh/h)	<u> </u>		1023	LDI		1551
HCM Lane V/C Ratio		0.057		-		0.306
HCM Control Delay (s)		22.2	9.5	-	-	8.3
HCM Lane LOS		C	7.5 A	_	_	Α
HCM 95th %tile Q(veh)		0.2	0.8	_		1.3
110111 70111 701110 Q(VCII)		0.2	0.0			1.0

: Clari Park Access #2/W Park Dr & Wilkinson Pk	2027 Total Traffic Improved - PM Peak Hour
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Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR	Intersection												
Lane Configurations	Int Delay, s/veh	2.4											
Traffic Vol, veh/h Friture Vol, veh/h 61 174 12 6 436 31 11 0 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 0 5 28 0 40 62 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 44 Future Vol, veh/h 61 174 12 6 436 31 11 0 5 5 28 0 40 40 Sign Control Free Free Free Free Free Free Free Free	Lane Configurations	ሻ	₽		ħ	(4			4	
Conflicting Peds, #/hr	Traffic Vol, veh/h	61	174	12	6		31	11		5	28		44
Sign Control Free Future Vol, veh/h	61	174	12	6	436	31	11	0	5	28	0	44	
RT Channelized	Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Storage Length 100	Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Veh in Median Storage, # - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Grade, %	Storage Length	100	-	-	100	-	-	-	-	-	-	-	-
Peak Hour Factor 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94	Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2	Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Mymt Flow 65 185 13 6 464 33 12 0 5 30 0 47 Major/Minor Major1 Major2 Minor1 Minor2 Conflicting Flow All 497 0 0 198 0 0 838 831 192 817 821 481 Stage 1 - - - - - 322 322 - 493 493 - Stage 2 - - - - 516 509 - 324 328 - Critical Hdwy Stg 1 - - - - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - <td>Peak Hour Factor</td> <td>94</td>	Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Major/Minor Major1	Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Stage 1	Mvmt Flow	65	185	13	6	464	33	12	0	5	30	0	47
Stage 1													
Stage 1	Maior/Minor	Maior1		N	Maior2			Minor1		ľ	Minor2		
Stage 1			0			0			831			821	481
Stage 2		-		-	-								
Critical Hdwy 4.12 - 4.12 - - 7.12 6.52 6.22 7.12 6.52 6.22 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - 6.12 5.52 - - 6.12 5.52 - 6.12 5.52 - - 6.12 5.52 - - 6.12 5.52 - - 6.12 5.52 - <		-	-	_	-	_	_			-			_
Critical Hdwy Stg 1 - - - - - 6.12 5.52 - 6.12 5.52 - Critical Hdwy Stg 2 - - - - - 6.12 5.52 - 6.12 5.52 - Follow-up Hdwy 2.218 - - 2.218 - - 3.518 4.018 3.318 3.518 4.018 3.318 Pot Cap-1 Maneuver 1067 - - 1375 - - 286 305 850 295 309 585 Stage 1 - - - - - 640 651 - 558 547 - Stage 2 - - - - - 542 538 - 688 647 - Mov Cap-1 Maneuver 1067 - 1375 - - 250 285 850 278 289 585 Mov Cap-2 Maneuver - - - - 648 611 - 524 545 - - <td>ū</td> <td>4.12</td> <td>-</td> <td>-</td> <td>4.12</td> <td>-</td> <td>-</td> <td></td> <td></td> <td>6.22</td> <td></td> <td></td> <td>6.22</td>	ū	4.12	-	-	4.12	-	-			6.22			6.22
Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.12 5.52 - Follow-up Hdwy 2.218 - - 2.218 - - 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.318 3.518 4.018 3.518 4.018 4.018 3.518 4.018 4.018 4.018 4.018 4.018 4.018 4.018 4.018 4.018 4.018 4	,	-	-	-	-	-	_						_
Follow-up Hdwy 2.218 - 2.218 - 3.518 4.018 3.318 3.518 4.018 3.318 Pot Cap-1 Maneuver 1067 - 1375 - 286 305 850 295 309 585 Stage 1 690 651 - 558 547 - Stage 2 542 538 - 688 647 - Platoon blocked, % 542 538 - 688 647 - Platoon blocked, % 542 538 - 688 647 - Platoon blocked, % 542 538 - 688 647 - Mov Cap-1 Maneuver 1067 - 1375 - 250 285 850 278 289 585 Mov Cap-2 Maneuver 648 611 - 524 545 - Stage 1 648 611 - 524 545 - Stage 2 648 611 - 524 545 - Stage 2 496 536 - 642 608 - Approach EB WB NB SB		-	-	-	-	-	-			-			-
Pot Cap-1 Maneuver 1067	3 0	2.218	_	_	2.218	_	_			3.318			3.318
Stage 1 - - - - 690 651 - 558 547 - Stage 2 - - - - 542 538 - 688 647 - Platoon blocked, % - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -<			-	-		-	-						
Stage 2 - - - - 542 538 - 688 647 - Platoon blocked, % - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - <	•	-	-	-	-	-	_						
Platoon blocked, % - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -		-	-	-	-	-	-			-			-
Mov Cap-1 Maneuver 1067 - - 1375 - - 250 285 850 278 289 585 Mov Cap-2 Maneuver - - - - - 250 285 - 278 289 - Stage 1 - - - - - 648 611 - 524 545 - Stage 2 - - - - - 496 536 - 642 608 - Approach EB WB WB NB SB HCM Control Delay, s 2.1 0.1 16.8 15.8 HCM Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 <td>Platoon blocked, %</td> <td></td> <td>-</td> <td>-</td> <td></td> <td>-</td> <td>_</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	Platoon blocked, %		-	-		-	_						
Mov Cap-2 Maneuver - - - - 250 285 - 278 289 - Stage 1 - - - - - 648 611 - 524 545 - Stage 2 - - - - - 496 536 - 642 608 - Approach EB WB NB NB SB HCM Control Delay, s 2.1 0.1 16.8 15.8 HCM Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A <td< td=""><td></td><td>1067</td><td>-</td><td>-</td><td>1375</td><td>-</td><td>-</td><td>250</td><td>285</td><td>850</td><td>278</td><td>289</td><td>585</td></td<>		1067	-	-	1375	-	-	250	285	850	278	289	585
Stage 1 - - - - 648 611 - 524 545 - Stage 2 - - - - - 496 536 - 642 608 - Approach EB WB NB SB HCM Control Delay, s 2.1 0.1 16.8 15.8 HCM LOS C C C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - - A - - C	Mov Cap-2 Maneuver	-	-	-	-	-	_						
Stage 2 - - - - 496 536 - 642 608 - Approach EB WB NB SB HCM Control Delay, s 2.1 0.1 16.8 15.8 HCM LOS C C C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - A - C		-	-	-	-	-	-			-			-
Approach EB WB NB SB HCM Control Delay, s 2.1 0.1 16.8 15.8 HCM LOS C C C Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - 15.8 HCM Lane LOS C A - - A - - C	•	-	-	-	-	-	_			-			_
HCM Control Delay, s 2.1 0.1 16.8 15.8	J.												
HCM Control Delay, s 2.1 0.1 16.8 15.8	Approach	FB			WB			NR			SB		
Minor Lane/Major Mvmt NBLn1 EBL EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - - A - C													
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - - A - - C		۷.۱			0.1								
Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - A - C	TOW LOS							U			U		
Capacity (veh/h) 321 1067 - - 1375 - - 409 HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - A - C	Minor Lane/Maior Mym	nt N	VRI n1	FRI	FRT	FBR	WRI	WRT	WRR	SBI n1			
HCM Lane V/C Ratio 0.053 0.061 - - 0.005 - - 0.187 HCM Control Delay (s) 16.8 8.6 - - 7.6 - - 15.8 HCM Lane LOS C A - - A - C		1											
HCM Control Delay (s) 16.8 8.6 7.6 15.8 HCM Lane LOS C A A C													
HCM Lane LOS C A A C													
									_				
110W 70W 70W Q (VOI))							-				
	110W 75W 70W Q(VCH)		0.2	0.2			U			0.7			

Intersection						
Int Delay, s/veh	4.2					
	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				
Lane Configurations	170	20	212	↑	\	204
Traffic Vol, veh/h	178 178	29 29	312 312	459	8	206
Future Vol, veh/h				459		206
Conflicting Peds, #/hr	0	0	0	0	O Cton	O Cton
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	100	None	100	None
Storage Length	-	-	100	-	100	0
Veh in Median Storage,		-	-	0	1	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	187	31	328	483	8	217
Major/Minor Ma	ajor1	N	Major2	ı	Minor1	
Conflicting Flow All	0	0	218	0	1342	203
			210		203	
Stage 1	-	-	-	-		-
Stage 2	-	-	4 1 2	-	1139	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218			3.318
Pot Cap-1 Maneuver	-	-	1352	-	168	838
Stage 1	-	-	-	-	831	-
Stage 2	-	-	-	-	305	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1352	-	127	838
Mov Cap-2 Maneuver	-	-	-	-	199	-
Stage 1	-	-	-	-	831	-
Stage 2	-	-	-	-	231	-
Annraach	ΓD		WD		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		3.4		11.3	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn11	VRI n2	EBT	EBR	WBL
Capacity (veh/h)	<u> </u>	199	838	-		1352
HCM Lane V/C Ratio		0.042				0.243
				-		8.5
HCM Long LOS		23.9	10.8	-	-	
HCM Lane LOS		C	В	-	-	A
HCM 95th %tile Q(veh)		0.1	1	-	-	1

MURFREESBORO PLANNING COMMISSION STAFF COMMENTS, PAGE 1 MAY 5, 2021 PROJECT PLANNER: MATTHEW BLOMELEY

3.c. Street renaming [2021-902] to rename an approximately two-mile long segment of Mercury Boulevard (west of South Rutherford Boulevard) to Dr Martin Luther King Jr Boulevard, City of Murfreesboro Planning Department applicant.

Section 5.2 of the Subdivision Regulations states that "The Planning Commission shall have final authority over street names." At its March 10, 2021 meeting, the City Council directed the Planning Staff to bring to the Planning Commission for its consideration the renaming of an approximately two-mile long segment of Mercury Boulevard from Southeast Broad Street to South Rutherford Boulevard. This segment is proposed to be renamed to "Dr Martin Luther King Jr Boulevard." A detailed report regarding this proposed street renaming is attached to these staff comments for the Planning Commission's review.

Action Needed:

The Planning Commission will need to conduct a public hearing, after which it should discuss this matter and then take action on the request.

MURFREESBORO PLANNING COMMISSION STAFF COMMENTS, PAGE 1 MAY 5, 2021 PROJECT PLANNER: MATTHEW BLOMELEY

3.d. Street renaming [2021-903] to rename an approximately 600'-long segment of Mercury Boulevard (east of South Rutherford Boulevard) to John Bragg Highway, City of Murfreesboro Planning Department applicant.

Section 5.2 of the Subdivision Regulations states that "The Planning Commission shall have final authority over street names." At its March 10, 2021 meeting, the City Council directed the Planning Staff to bring to the Planning Commission for its consideration the renaming of an approximately 600'-long segment of Mercury Boulevard east of South Rutherford Boulevard. This segment is proposed to be renamed to "John Bragg Highway." A detailed report regarding this proposed street renaming is attached to these staff comments for the Planning Commission's review.

Action Needed:

The Planning Commission will need to conduct a public hearing, after which it should discuss this matter and then take action on the request.

Report on Potential Renaming of Mercury Boulevard to Dr Martin Luther King Jr Boulevard April 2021

City of Murfreesboro Planning Department

Impetus for Report

During its regular meeting on February 4, 2021, Council directed Staff to study the potential renaming of Mercury Boulevard to Dr Martin Luther King Jr Boulevard. This report has been drafted to provide Council additional background on the street renaming process, the history of Mercury Boulevard, and the impacts of a potential renaming. At its March 10, 2021 workshop, Council directed Staff to bring the renaming request to the Planning Commission for its consideration.

Street Naming Authority

Section 5.2 of the City's Subdivision Regulations states that "The Planning Commission shall have final authority over street names." Having consistent and logical protocols for street naming and property addressing aids in predictability for the general public as well as for service providers, including emergency service providers. However, the Subdivision Regulations offers little additional guidance regarding street naming, except that proposed street names may not duplicate existing subdivision names or street names and that extensions of existing streets shall continue the same street name. In an effort to fill in the gaps, Staff, in consultation with various emergency service providers, attempts to provide professional guidance to the Planning Commission in carrying out its street naming authority.

In an effort to clarify when renaming a street is appropriate, the Planning Commission adopted the following policy in January 2019: "It shall be the policy of the City of Murfreesboro Planning Commission to rename existing streets only when a legitimate public interest is served and only when it promotes the public health, safety, and welfare of the community as a whole (e.g., in conjunction with a road construction project or to eliminate confusion for emergency service providers). The Planning Commission shall not endorse street renaming requests that do not meet this standard." This policy was adopted to provide guidance for the Planning Commission in determining when to consider renaming streets.

Before the Planning Commission votes on whether to rename a street, it conducts a public hearing, so that all affected and interested parties have an opportunity to express their opinion on the renaming. To Staff's knowledge, there is no legal requirement to hold a public hearing for a street renaming. However, this has been

the City's customary practice for at least several decades.

Additional Background on Mercury Boulevard

Mercury Boulevard's western terminus is located at its intersection with Southeast Broad Street (US HWY 41/SR 2). The Mercury Boulevard street name extends approximately 600 linear feet beyond its intersection with South Rutherford Boulevard. At that point, its street name changes to John Bragg Highway. Mercury Boulevard is also identified as US HWY 70s and SR 1. Existing land uses along Mercury Boulevard include an assortment of single-family residential, multi-family residential, commercial, and institutional.

Mercury Boulevard was constructed in phases. The first phase was the segment from Southeast Broad Street eastward to what is now known as Lasseter Drive. It was constructed in the early-mid 1950s. In the 1956 City Directory edition (BEK Directories, Inc), three addressed properties, including Bradley Academy, are noted on Mercury Boulevard. The earliest subdivision plat located by Staff adjacent to this roadway calling out the street name as "Mercury Boulevard" was in 1955. A 1945 plat of the lots near the intersection of Mercury and Carver Avenue shows a "Washington Street" in the vicinity of where the current Mercury Boulevard right-of-way is located. Staff is not certain of the reason the highway was named Mercury Boulevard when it was constructed. However, there are other streets located south of Mercury Boulevard with names related to Roman and Greek mythology, including Mars, Diana, Minerva, Atlas, Venus, Olympia, and Apollo. Most of these streets were constructed around the same time as Phase 1 of Mercury Boulevard, so naming the highway "Mercury" Boulevard was consistent with the street naming theme of that area.

Proposed Renaming

Upon reviewing the request by Council, Staff reviewed the existing street naming and address numbering conditions. Staff has always been concerned about the confusion created by the 600' segment of highway named Mercury Boulevard east of South Rutherford Boulevard. Having a roadway transition to a different street name at a major intersection is almost always preferable to a transition at a much less conspicuous location. South Rutherford Boulevard is likely where most people already think that the street name changes to John Bragg Highway. This renaming project, if it moves forward, will allow Staff to rectify this existing, confusing situation, creating a more recognizable location where the street name changes to John Bragg Highway. Numerically, most of the addresses are in range. However, there are approximately ten address numbers west of South Rutherford Boulevard that Staff believes need to change. In addition, all of the address numbers east of Rutherford will need to be adjusted. Changing the numerical addresses can be accomplished simultaneously with the street name changes.

The total number of address changes associated with this street name change will be approximately 403, including approximately 39 commercial and government addresses and approximately 364 residential addresses (including both single-family residential homes and dwelling units in multi-family residential developments). An

overall map, as well as maps showing individual segments of the proposed renaming, can be found in Appendix A.

Impact to Property Owners and Tenants

It is unavoidable that there will be confusion caused by any street name change. Street name changes take time for everyone to get accustomed to, especially for a street like Mercury Boulevard, which has been known by its current name for over 60 years. It is predictable that some people will continue to call a street by its old name even after its name has officially been changed. In addition, some entities may commit data entry errors or omissions during the course of updating the affected addresses. These are known risks associated with any street name or address change. However, with the proper steps taken by the City and by the impacted property owners and tenants, these issues can be minimized during and after the transition.

When streets are renamed, there are certain tasks that must be accomplished in order to implement the name change. City Staff determines an effective date for the name change that will provide sufficient time for service providers and property owners/tenants to implement the change. Staff will then notify various service providers of the name change and the effective date, including emergency service providers, utilities, the USPS, and applicable City Departments, to name a few. On a side note, various private entities, including various websites and GPS providers, separately obtain updated street and address information from non-City entities. Some of these websites and GPS providers may have a lag time in updating their data. This is outside of the City's ability to control.

Property owners and tenants will be responsible for notifying entities that they do business with, such as personal creditors (e.g., credit card companies and banks), subscriptions to magazines, customers, and family members and friends. Also, for any addresses where the number is also changing, the address numbers on the mailbox and on the building will need to be changed. The numbers on the structures will need to meet MFRD requirements. In addition, the State of Tennessee Department of Safety requires any affected residents to change their address in the State driver's license records, which can be done online. Finally, property owners may need to amend the deeds to their property, so that there are no address-related inconsistencies, potentially causing issues with future sales of their property.

Impact to Service Providers

All service providers will need adequate time to implement the proposed street name change and the associated address changes. Staff contacted the City Transportation Department, MPD, MFRD, and the Rutherford County Emergency Communications District (RCECD) for additional information on the impact of the renaming. (Copies of the correspondence from these entities can be found in Appendix B.) According to the Transportation Department, the estimated cost of fabricating and installing new street name signs for the length of this roadway is approximately \$10,000.

MPD and MFRD have not indicated that this street name change will be a hindrance in continuing to provide its current level of services. In addition, both departments

are in favor of seizing the opportunity change the street name to John Bragg Highway at the roadway's intersection with South Rutherford Boulevard. Correspondence from RCECD indicates that the Martin Luther King Jr Boulevard street name is not in conflict with any existing street names within Rutherford County and is available for use. The "Mercury" street name is duplicated elsewhere in Rutherford County, as there is a Mercury Drive in LaVergne. It appears that Mercury Drive in LaVergne has been in use as a street name since at least the 1950s. While it is generally not recommended practice to duplicate street names, in this instance the address ranges for the two streets are not coincident and RCECD has indicated that it is not aware of any incidents where the existing duplication of the Mercury street name has caused issue in determining a correct service address in a time of emergency. Further, it is unlikely that Mercury Boulevard and Mercury Drive address ranges will overlap as it is unlikely that either street will be extended in the future into coincident address ranges.

RCECD reached out to the State of Tennessee's GIS vendor regarding the use of "Dr" in the proposed street name. ("Dr" is also the abbreviation for the suffix "Drive.") The GIS vendor indicated that the use of "Dr" should work as long as it is input correctly during times of emergency. However, if it is used incorrectly, then it could create discrepancies in matching addresses in the system during times of emergency. Additional information on how street names and addresses are changed for emergency service providers can be found in the attached letter and e-mail from RCECD in Appendix B.

Consistency with Subdivision Regulations and Planning Commission Street Renaming Policy

As previously stated, the Subdivision Regulations states that proposed street names may not duplicate existing street names. Both the Mercury Boulevard and Mercury Drive street names appear to have originated in the 1950s, long before the adoption of the current Subdivision Regulations. Although the existing street name duplication appears to pose little risk in the provision of emergency services, eliminating the duplication will help eliminate future risks of these two streets being confused with one another. In addition, City Staff is of the opinion that creating the street name break at South Rutherford Boulevard complies with the adopted 2019 street naming policy promoting public health, safety and welfare, as it is beneficial for the community and for emergency service providers and it will help to reduce confusion regarding where the John Bragg Highway street name begins. The name change location for John Bragg Highway at South Rutherford Boulevard is consistent with recent Planning Commission initiatives (e.g., South Church Street/Shelbyville Pike, Memorial Boulevard/Lebanon Pike) to align major street name change locations to facilitate NextGen E-911 services and timely emergency response.

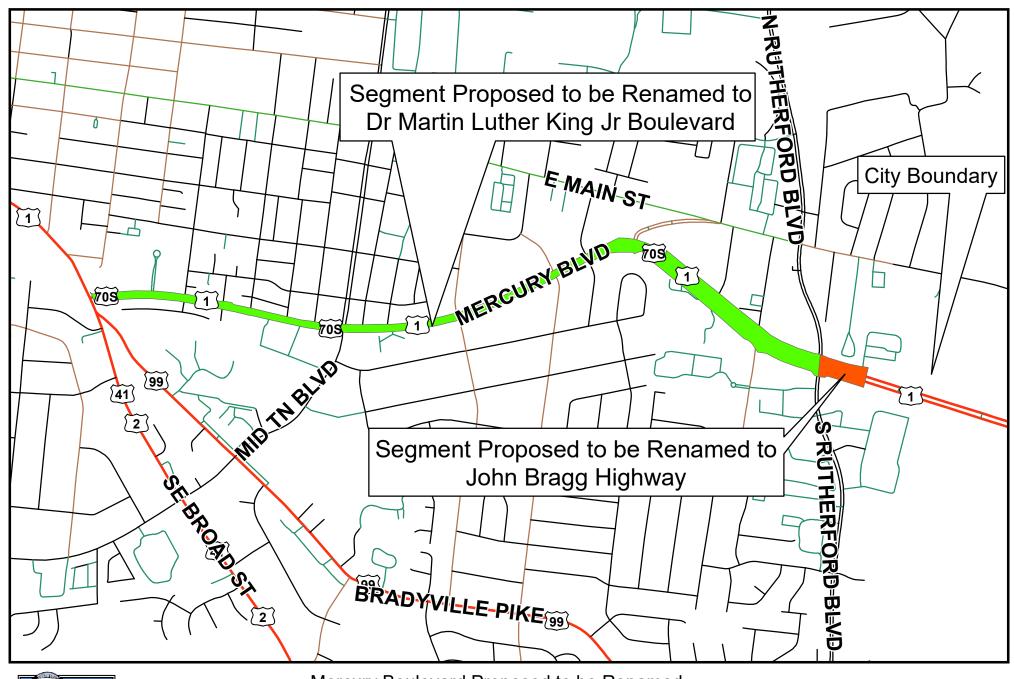
Beyond the items noted above, however, is the reason this proposed street name change was brought up in February -- which is to honor the substantial contributions made by Dr. Martin Luther King, Jr. in the advancement of civil rights in our nation and, as a result, in our community as well. In addition, it is intentional that Mercury Boulevard was chosen to be studied for this renaming, as it is located in an area of significant cultural and historical importance Renaming a major street after Dr. Martin Luther King, Jr. embraces the diversity of our community and honors the

legacy of Dr. King. The street renaming policy states that renamings must "serve a legitimate public interest" and "promote the public health, safety, and welfare of the community as a whole." Staff believes that this renaming achieves these goals.

Next Steps

The Planning Commission should conduct public hearings on these matters, after which it should discuss them and then take action. The public hearings have been noticed via the City website, the Murfreesboro Post, signs along Mercury Boulevard, and notices mailed directly to affected property owners, commercial tenants, and residential tenants. If approved by the Planning Commission, Staff will determine an effective date and begin implementing the street name change and associated address changes.

Appendix A Maps of Mercury Boulevard

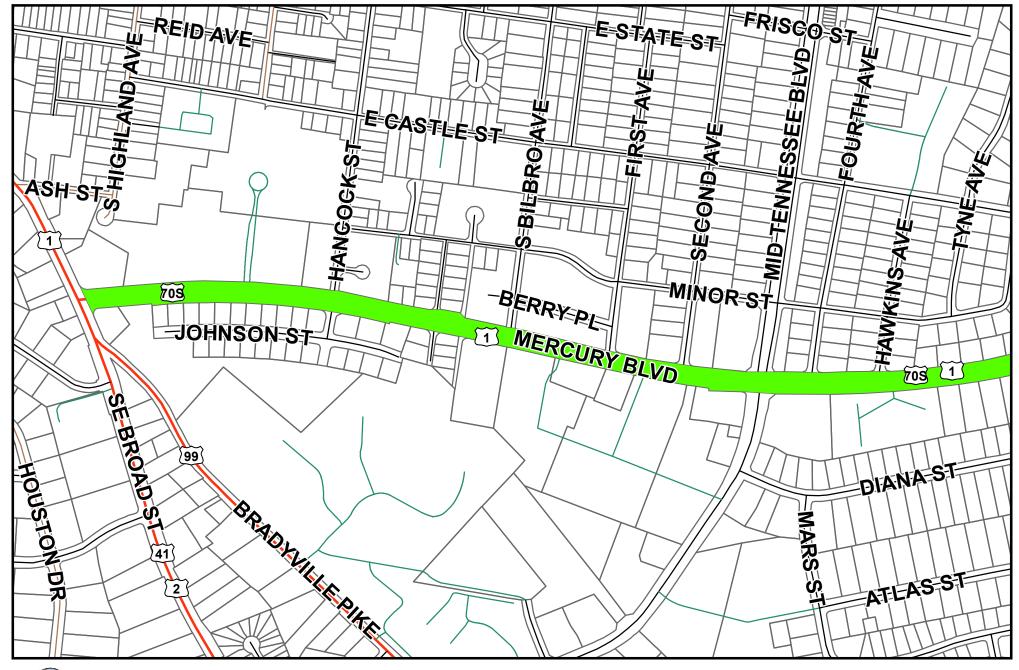




Mercury Boulevard Proposed to be Renamed
Dr Martin Luther King Jr Boulevard and John Bragg Highway

Teet 0 1,000 2,000 4,000 6,000



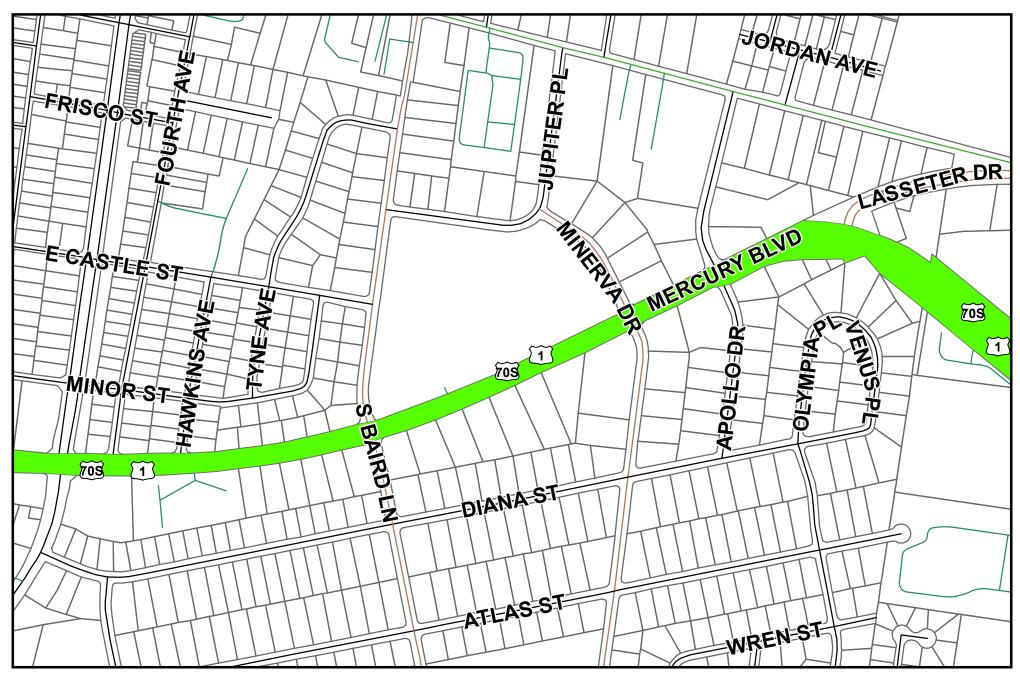




Segment of Mercury Boulevard Proposed to be Renamed Dr Martin Luther King Jr Boulevard





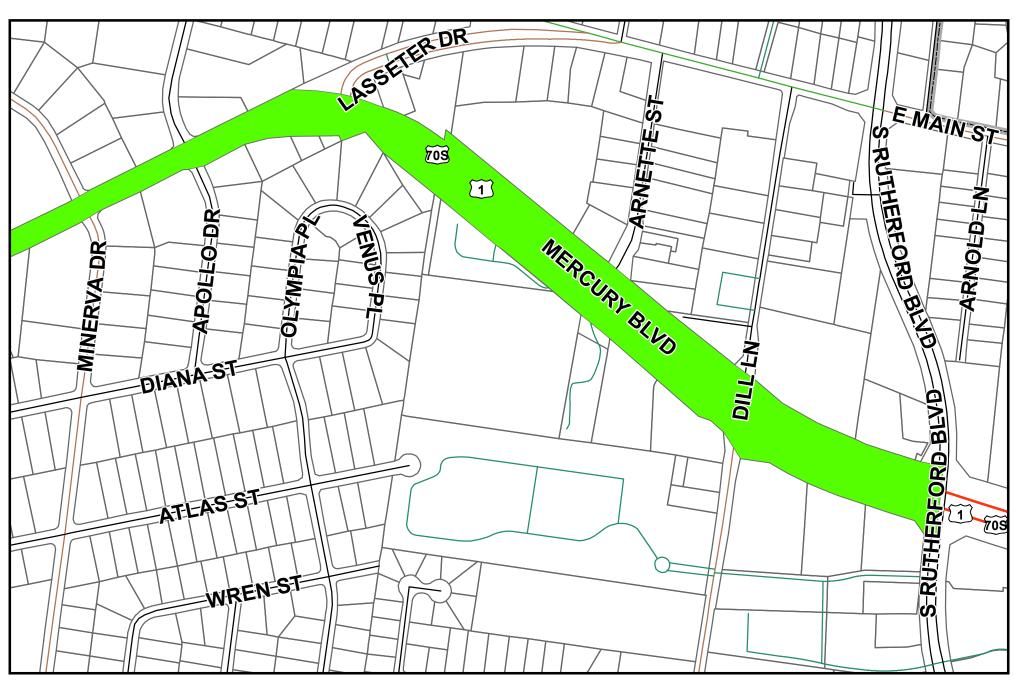




Segment of Mercury Boulevard Proposed to be Renamed Dr Martin Luther King Jr Boulevard





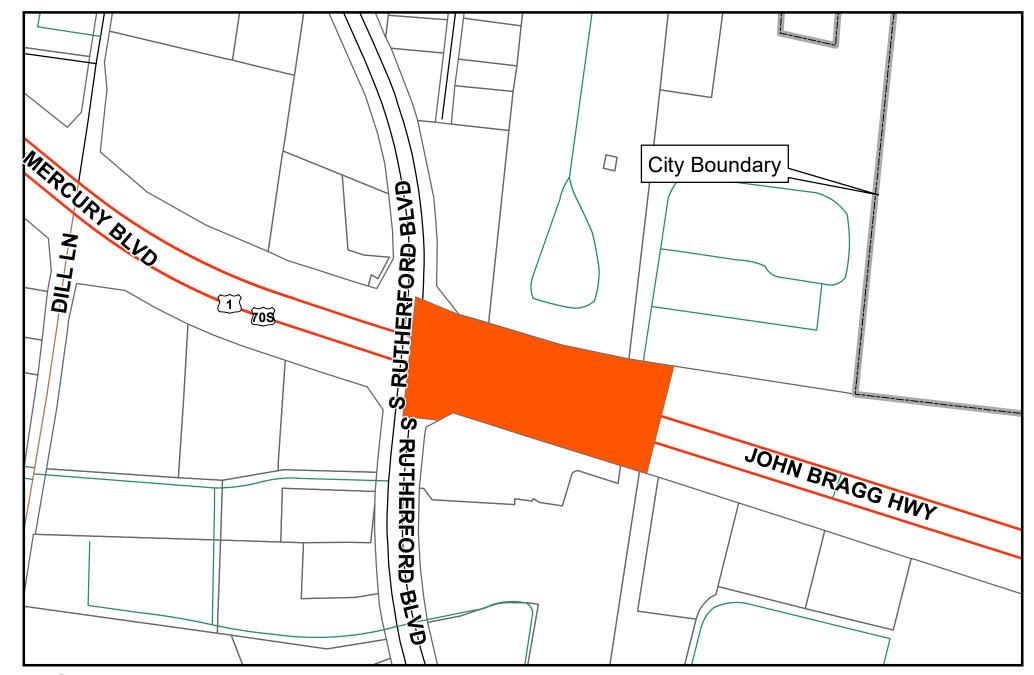




Segment of Mercury Boulevard Proposed to be Renamed Dr Martin Luther King Jr Boulevard









Segment of Mercury Boulevard Proposed to be Renamed John Bragg Highway





Appendix B Responses from Service Providers

From: <u>Jim Kerr</u>

To: Matthew Blomeley; Ram Balachandran

Cc: Sam Huddleston; Greg McKnight; Matt Fasig

Subject: RE: Potential Mercury Boulevard renaming

Date: Wednesday, March 3, 2021 10:00:25 AM

Matthew,

Knowing this was coming, we have already pulled together an estimated cost for ground and overhead street name sign replacements. Not sure the exact name MLK Blvd. or Martin Luther King Jr. Blvd. we estimate the fabrication and installation at about \$10,000. This includes contracting S&W for the overhead signs at the signalized intersection locations. Let us know if you need further information.

Thanks

Jim Kerr

Transportation Director City of Murfreesboro P.O. Box 1139 Murfreesboro, TN 37133-1139

Office: (615) 893-6441 Fax: (615) 849-2606

Email:jkerr@murfreesborotn.gov

From: <u>Carl Peas</u>
To: <u>Matthew Blomeley</u>

Subject:RE: Mercury Boulevard renamingDate:Wednesday, March 3, 2021 1:41:55 PM

Matthew,

After consulting with The Community Risk Reduction Staff and the responders from Station #3 (Mercury Blvd.). We agree that changing the name of Mercury Blvd. to Martin Luther King Blvd. would not cause any disruption in the services that we provide. As you pointed out in our phone conversation earlier, we all like the idea of having Martin Luther King Blvd. having a hard stop at Rutherford Blvd. then picking up the name John Bragg Hwy. This would help eliminate any confusion to the current Mercury Blvd. that proceeds past Rutherford Blvd. If you have any questions, please feel free to contact me. Thanks.

Carl Peas
Assistant Chief/Fire Marshal
Murfreesboro Fire Rescue Department
1311 Jones Blvd.
Murfreesboro TN. 37130

Office: 615-893-1422 Mobile: 615-642-3224 cpeas@murfreesborotn.gov From: <u>Clayton Williams</u>

To: Matthew Blomeley; Carl Peas
Cc: Sam Huddleston; Greg McKnight
Subject: RE: Mercury Boulevard renaming
Date: Wednesday, March 3, 2021 2:20:53 PM

Mr. Blomeley,

The Police Department has no issue with changing the roadway name and we agree that using Rutherford Blvd as the transition point for MLK/John Bragg is fine. However, we will need some time and a heads up before the official change takes place to address some back end issues.

These issues include updating our Records Management System and Computer Aided Dispatch to marry the new address names and numbers to the current ones in order to properly maintain historical call history of any affected properties. We will also need to update the information with any business or residence with an alarm permit to maintain accurate records and billing.

Does that make sense?



Clayton Williams Lieutenant Administration

Murfreesboro Police Department 1004 North Highland Avenue Murfreesboro, TN 37130

Phone: 629-201-5572 Cell: 615-971-6370

email: 0417@murfreesborotn.gov



RUTHERFORD COUNTY EMERGENCY COMMUNICATIONS DISTRICT

591 FORTRESS BOULEVARD • MURFREESBORO, TN 37128-4129 TELEPHONE (615) 890-7550

March 4, 2021

Matthew Blomeley Murfreesboro Planning Dept. 111 West Vine Street Murfreesboro, TN 37130

Dear Matthew,

Per the City of Murfreesboro Planning Department's request, the Rutherford County Emergency Communications District completed research to determine the various factors that should be taken into consideration when deliberating whether to rename Mercury Boulevard. The key factors are as follows:

1. Proposed Road Name Variations –

MLK, $Martin\ Luther\ King$, $Martin\ Luther\ King\ Jr$ – none of the proposed spelling variations are in conflict with other road names either currently in use within Rutherford County or listed as a reserved street name on the Rutherford County ECD Reserved Street Name List. Furthermore, the Rutherford County ECD office is not aware of any issues with existing abbreviated road names such as $MTSU\ Boulevard$ or $MTCS\ Road$.

2. Road Name Duplication -

The road name *Mercury* is currently duplicated in Rutherford County. *Mercury Boulevard* is located within the Murfreesboro city limits and *Mercury Drive* is located within the La Vergne city limits.

Mercury Dr 100-299 LaVergne 262 ESN Mercury Blvd 500-2399 Murfreesboro 261 ESN

3. Previously known issues regarding the current road name –

The Rutherford County ECD office is not aware of any prior issues in determining the location of a caller requesting emergency services on either Mercury Boulevard or Mercury Drive.

BOARD OF DIRECTORS

KEVIN ARNOLD, CHAIRMAN • MIKE FITZHUGH, VICE-CHAIRMAN • MIKE WALKER, SECRETARY • JOHN HOOD, TREASURER MICHAEL BOWEN • DAVID BRENISER • CHRIS CLARK • MARK FOULKS • BILL KETRON

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4. Process involved in changing a road name in the Master Street Address Guide (MSAG) -

Various contractual and software changes with both AT&T and Intrado during the last few years have changed the number of guaranteed changes and how the number of requested changes is calculated. When working on a project, Intrado is currently contracted, by AT&T, to guarantee 30 changes per day.

If an MSAG entry for a road is broken up into multiple segments, each segment/translates to an additional MSAG entry. Each segment/MSAG entry that is submitted for a change will count individually. Roads that transition between two or more jurisdictions or roads that include a large gap in address ranges are broken up in the appropriate number of MSAG entries to reflect the transition. Many of the main roads transition from one jurisdiction to another, multiple times, as one jurisdiction annexes portions of land on either side of the road.

If a road name change includes combining two or more of the individual segments/MSAG entries, the act of combining the segments will decrease the number of changes requested.

In the past, each customer record that required a change, based upon a road name change, counted as an additional change. This however has changed to our favor in some cases. The current policy now states that if the customer address numeric is not changed in addition to the road name, the customer record change will not count as a separate change. However, if the address numeric is changed, each customer record submitted for an address numeric change will increase the number of requested changes, which will add to the count of changes requested for the separate MSAG entry/entries for the road.

If any approved road name and/or address changes result in 31 or more changes to the Master Street Address Guide database, the approved changes can be completed in segments, with different effective dates.

As always, the Rutherford County ECD office is available to answer any additional questions or address any concerns regarding the information provided today.

Respectfully,

Cassie Lowery, ENP Assistant Director From: <u>Cassie Lowery</u>
To: <u>Matthew Blomeley</u>

Cc: Steve Smith; Sam Huddleston

Subject: RE: [EXTERNAL]- Fwd: Street Name Question Date: Thursday, April 15, 2021 10:35:05 AM

Attachments: <u>image001.png</u>

Matthew,

I believe spelling out Doctor rather than using an abbreviation would work but you should keep in mind that someone, down the road, may end up changing or referring to it "unofficially" with the abbreviation. We could also run into an issue if anyone searching for the street name, via electronic means, did so by using the common abbreviation. I know this seems like a lot of "but" and "if" scenarios for a simple street name request but I feel compelled to point the possibilities out so that everyone can make an informed decision.

Thank You, Cassie



Cassie Lowery, ENP Rutherford County E.C.D./9-1-1 Assistant Director (615) 890-7550 591 Fortress Blvd Murfreesboro, TN 37128-4129 http://www.rcecd911.org

From: Matthew Blomeley <mblomeley@murfreesborotn.gov>

Sent: Thursday, April 15, 2021 10:08 AM **To:** Cassie Lowery <clowery@rcecd911.org>

Cc: Steve Smith <ssmith@rcecd911.org>; Sam Huddleston <shuddleston@murfreesborotn.gov>

Subject: RE: [EXTERNAL]- Fwd: Street Name Question

Good morning, Cassie.

Based on TrueNorth's e-mail below, it seems that it would work but there is an increased chance for human or computer error with the "Dr" at the beginning of the streetname. If the "Dr" was changed to "Doctor", would that address these concerns?

Thanks,

Matthew

Matthew T. Blomeley, AICP Assistant Planning Director

City of Murfreesboro Planning Department Office Phone: (615)-893-6441 Ext. 1605 E-mail: mblomeley@murfreesborotn.gov Website: www.murfreesborotn.gov

From: Cassie Lowery < <u>clowery@rcecd911.org</u>>

Sent: Friday, March 5, 2021 11:57 PM

To: Matthew Blomeley < mblomeley@murfreesborotn.gov >

Cc: Steve Smith < ssmith@rcecd911.org>

Subject: [EXTERNAL]- Fwd: Street Name Question

Matthew,

Per my previous email, I reached out to the GIS vendor that is contracted by the State of Tennessee to consume and integrate local GIS data into a single, consolidated and standardized statewide dataset for NG911 call routing. Please refer to the email included below for TrueNorth Geographic Technologies' response to your inquiry.

Thank You, Cassie Lowery

Sent from my iPhone

Begin forwarded message:

From: James Wood <jwood@tngeo.com>
Date: March 5, 2021 at 7:15:16 PM CST
To: Cassie Lowery <<u>clowery@rcecd911.org></u>
Cc: David Speight <<u>dspeight@tngeo.com></u>
Subject: RE: Street Name Question

As long as it's parsed correctly, it should be OK, but I prefer to stay away from things that could potentially create complications for processes outside of human control. For instance if I look at "DR LOWERY DR", I can probably tell what is "Doctor" and what is "Drive", but with geocoding engines expecting prefix street types ("AVE A" is a good example), then there could not entirely be some confusion as to how to parse, and

example), then there could potentially be some confusion as to how to parse, and therefore how to correctly match, a particular address. You don't want that happening in the PSAP or for other public safety applications relying on non-human interaction.

From: Cassie Lowery < <u>clowery@rcecd911.org</u>>

Sent: Friday, March 5, 2021 11:49

To: James Wood < <u>iwood@tngeo.com</u>>; David Speight < <u>dspeight@tngeo.com</u>>

Subject: Street Name Question

I have another street name question and did attempt to locate an answer in the GIS Standards for NG911 document but was unable to do so. What is your opinion on the use of an abbreviation for doctor (Dr) as part of a street name?

Thank You, Cassie



Cassie Lowery, ENP
Rutherford County E.C.D./9-1-1
Assistant Director
(615) 890-7550
591 Fortress Blvd
Murfreesboro, TN 37128-4129
http://www.rcecd911.org

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MURFREESBORO PLANNING COMMISSION STAFF COMMENTS, PAGE 1 MAY 5, 2021

PROJECT PLANNER: MATTHEW BLOMELEY

- 3.e. Proposed amendments to the Zoning Ordinance [2020-807] regarding townhouses, the RS-A zone, and other miscellaneous topics and pertaining to the following sections:
 - Section 2: Interpretation and Definitions:
 - Section 19: Residential Districts;
 - Section 26: Off-Street Parking, Queuing, and Loading;
 - Chart 1: Uses Permitted by Zoning District (including Chart 1 Endnotes);
 - Chart 2: Minimum Lot Requirements, Minimum Yard Requirements, and Land Use Intensity Ratios (including Chart 2 Endnotes); and
 - Chart 4: Required Off-Street Parking and Queuing Spaces by Use.

City of Murfreesboro Planning Department applicant.

In 2017, an amendment to the Zoning Ordinance was adopted creating the RS-6 and RS-A zones. The RS-A zone includes the following three types: Type 1 (Zero-Lot Line), Type 2 (Suburban Townhouse), and Type 3 (Urban Townhouse). Before the adoption of this ordinance amendment, the Zoning Ordinance included very little detail on single-family attached uses, except for zero-lot line attached and detached dwellings. Staff believes that the adoption of the 2017 ordinance amendment has proven to be effective in providing additional opportunities and a more defined regulatory framework for single-family attached residential uses, including townhomes. However, in administering the regulations over the course of the last several years, Staff has noticed areas that it believes need improvement. In this request, Staff brings to the Planning Commission for its consideration proposed amendments to the Zoning Ordinance pertaining to the RS-A zone and, generally, to single-family residential and townhouse uses. It is Staff's goal in proposing these amendments to provide clarification on certain issues for City Staff and decision makers as well as the public and the development community. See below for additional detail.

Definitions (Section 2):

The current definition of "townhouse dwelling" in the Zoning Ordinance can be read to apply to both the "multi-family dwelling" definition and the "single-family attached dwelling" definitions. This has led to confusion about how to classify townhouse uses. This amendment seeks to include language that, for zoning purposes, townhouses are a type of single-family attached dwelling and not multi-family. This

would seem to be consistent with the 2017 creation of the RS-A zoning district -- which stands for "Residential Single-Family Attached" -- for uses such as townhomes.

Chart 1 (Uses Permitted) and Chart 1 Endnotes:

When the RS-A zone was adopted, Chart 1 was not revised to differentiate between single-family attached zero-lot line residential uses and single-family attached townhouse uses. While both are types of single-family attached residential uses, they are distinct, and in Staff's opinion, warrant separate entries in Chart 1 in order to clarify where they are permitted. In addition, Staff proposes to clarify which types of residential uses are permitted in each of the three RS-A zones. In order to achieve this, endnotes are proposed to be added to create more certainty. Endnotes are included in the proposed amendment that specify that zero-lot line dwellings are only permitted in Type 1, "suburban" style townhouses are only permitted in Type 2, and "urban" style townhouses are only permitted in Type 3. This is intended to create more predictability and continuity in how property in the various RS-A zones can be developed. Clarification has also been included that single-family detached dwellings (not of the zero-lot line variety) shall be permitted in all three types of RS-A zoning.

Chart 2 (Bulk Regulations) and Chart 2 Endnotes:

In the 2017 ordinance amendment, when the RS-6 zone was created, exterior material requirements were included for this district. The ordinance specifies several different types of exterior materials with limited combustibility that must be used as the primary exterior material on each building façade. However, there are other zoning districts with similar minimum lot sizes and side setbacks that do not currently have this requirement. Some of these other districts actually have smaller minimum lot size requirements that the RS-6 zone. With that mind, in order to be consistent, Staff proposes identical exterior material requirements for certain other zones that allow single-family uses and that have similar lot sizes and setbacks. Language is also proposed to specify that the reason for the exterior material requirements is to minimize the extent of fire damage on adjacent structures. Additional language is proposed to be added to give the Planning Director authority to review requests for alternative combinations of building materials after a review of their combustibility with the Building and Codes Director.

In addition, the RS-6 zone requires a minimum front garage setback of 35' in order to allow for driveways of adequate length to accommodate four (4) vehicles; however, the balance of the structure is allowed a reduced minimum front setback of 25'. There are a number of zoning districts -- RS-8, for example -- that currently have a 30' minimum front building setback. This does not allow for driveways of adequate depth to accommodate four (4) vehicles. The 30' front setback creates a situation where vehicles block the public sidewalk due to a lack

of adequate driveway depth. Staff proposes to modify the front setback requirements of those zoning districts, so that they mirror those of the RS-6 zone.

Modifications are also proposed to the Chart 2 endnotes which contain the standards for the RS-A zones.

Miscellaneous:

In addition to the aforementioned revisions, Staff also proposes some other miscellaneous revisions to ensure that the language in the Zoning Ordinance is consistent throughout.

Action Needed:

A draft of the proposed ordinance amendment is included in the agenda packet for the Planning Commission's review. The Planning Commission will need to conduct a public hearing, after which it will need to discuss and then formulate a recommendation to City Council. **ORDINANCE 21-O-XX** amending Murfreesboro City Code Appendix A—Zoning, Sections 2, 19, 26, Chart 1, Chart 1 Endnotes, Chart 2, Chart 2 Endnotes, and Chart 4, City of Murfreesboro Planning Staff, applicant. [2020-807]

BE IT ORDAINED BY THE CITY COUNCIL OF THE CITY OF MURFREESBORO, TENNESSEE, AS FOLLOWS:

<u>SECTION 1</u>. Appendix A, Section 2, Interpretation and Definitions, of the Murfreesboro City Code is hereby amended by deleting the definitions for "Dwelling, Townhouse" and "Dwelling, Zero-Lost Line" and replacing with the following definitions:

<u>"Dwelling, Townhouse:</u> A row of three or more adjoining dwelling units, each of which is separated from the others by one or more unpierced common walls extending from the ground to the roof and have at least two exterior walls. For the purposes of this article, "Dwelling, Townhouse" shall be a type of Single-Family Attached Dwelling and not a type of Multiple-Family Dwelling.

<u>Dwelling</u>, <u>Zero-Lot Line</u>: A dwelling located on a lot in such a manner that one or more sides rests directly on a lot line. For the purposes of this article, "Dwelling, Zero-Lot Line" shall either be a type of Single-Family Attached Dwelling or Single-Family Detached Dwelling."

<u>SECTION 2</u>. Appendix A, Section 19, Residential Districts, of the Murfreesboro City Code is hereby amended by deleting the section titled "RS-A, SINGLE-FAMILY ATTACHED" and replacing with the following:

"RS-A, SINGLE-FAMILY ATTACHED

This district is intended to permit the development and maintenance of residential areas characterized by three specific development types:

Type 1: Zero-lot line. Type 1 includes two-unit structures with lots of at least three thousand square feet of lot area per dwelling unit.

Type 2: Suburban Townhouse. Type 2 includes single-family attached developments characterized by multi-unit townhouse structures with lots of least two thousand square feet per dwelling unit. Because Type 2 developments require broad building setbacks and dedicated open space, these developments are appropriate for suburban areas.

Type 3: Urban Townhouse. Type 3 includes single-family attached developments characterized by multi-unit townhouse structures with lots of at least two thousand square feet per dwelling unit. Because Type 3 developments have shallow setback requirements and do not necessitate dedicated open space, these developments are appropriate in urban areas, particularly as infill redevelopment.

Other uses such as single-family detached dwellings, schools, churches, and specified services associated with or compatible with the residential uses allowed in this district are also permitted, some of which are subject to site plan review and approval or the issuance of a special use permit therefore. The uses permitted in this district, the special permit uses that may be allowed in this district, and the uses for which administrative site plan

review and approval are required are listed in Chart 1 and its endnotes, unless otherwise regulated in this article. The minimum lot and yard requirements, maximum height, maximum gross dwelling unit density and the land use intensity ratios which govern any use in this district are listed on Chart 2 and its endnotes, unless otherwise regulated in this article. From and after the effective date of this amendment, all references in the Zoning Ordinance and Zoning Map to "RZ" shall be deemed to refer to "RS-A Type 1"."

SECTION 3. Appendix A, Section 26, Off-Street Parking, Queuing, and Loading, of the Murfreesboro City Code is hereby amended by deleting the last sentence of Section 26(C)(2)(d). (Ed. Note: The sentence deleted is as follows: Parking spaces within garages for multifamily residential structures that are also classified as single-family attached residential structures (e.g., townhomes) shall be regulated by Section 26(C)(1) (a)[5] of this article.)

<u>SECTION 4</u>. Appendix A, Chart 1, Uses Permitted by Zoning District, of the Murfreesboro City Code is hereby by deleting it in its entirety and substituting in lieu thereof the attached Chart 1.

<u>SECTION 5</u>. Appendix A, Chart 1 Endnotes, Uses Permitted by Zoning District, of the Murfreesboro City Code is hereby amended as follows:

- Delete Chart 1 Endnote 2 and replace it with "Reserved."
- Add Chart 1 Endnotes 23-28 as follows:
 - 23. Single-Family attached or detached, zero-lot line developments shall be subject to the use and development regulations listed in Section 33 of this article.
 - 24. The RS-A, Type 1 zone shall not permit single-family attached structures consisting of more than two dwelling units. While single-family attached or detached zero-lot line structures (max. 2 units attached) shall be permitted in the RS-A, Type 1 zone, they shall not be permitted in the RS-A, Type 2 or Type 3 zones.
 - 25. Suburban Type townhouses shall be permitted in the RS-A, Type 2 zone but not in the RS-A, Type 1 or Type 3 zones. Suburban Type townhouses may be on one lot of record as a horizontal property regime or on zero-lot line individual lots of record.
 - 26. Urban Type townhouses shall be permitted in the RS-A, Type 3 zone but not in the RS-A, Type 1 or Type 2 zones. Urban Type townhouses may be on one lot of record as a horizontal property regime or on zero-lot line individual lots of record.
 - 27. Single-family detached dwellings shall be permitted by right in all RS-A zones.
 - 28. In the RS-A, Type 2 and RS-A, Type 3 districts, single-family attached townhouse dwellings may be located one lot of record as part of a horizonal property regime or on individual lots of record. In all other districts where townhouses are permitted (with the exception of duly-approved PUD or PRD zones specifically allowing townhouses on individual lots of record), they shall be located on one lot of record as part of a horizontal property regime.

SECTION 6. Appendix A, Chart 2, Minimum Lot Requirements, Minimum Yard Requirements and Land Use Intensity Ratios, of the Murfreesboro City Code is hereby amended by deleting it in its entirety and substituting in lieu thereof the attached Chart 2.

<u>SECTION 7</u>. Appendix A, Chart 2 Endnotes, Minimum Lot Requirements, Minimum Yard Requirements and Land Use Intensity Ratios, of the Murfreesboro City Code is hereby amended as follows:

- Delete Chart 2 Endnote 10 and replace it with "Reserved."
- In both Chart 2 Endnotes 26 and 27, add "and single-family residential attached townhouse developments" after "multi-family residential developments."
- Delete in their entirety Chart 2 Endnotes 28-36 and substitute in lieu thereof Chart 2 Endnotes 28-38 as follows:
 - 28.In all RS-A districts as well as the RS-4, RS-6, RS-8, RD, RM-12, RM-16, and CL districts, in order to minimize the extent of fire damage on adjacent structures, the facades of single-family detached dwellings shall consist primarily of one or more of the following materials: brick, stone, or cementitious siding. Other building materials such as EIFS, vinyl siding, and wood siding may be used for decorative or accent purposes and may constitute no more than 25 percent of any façade. Alternative combinations of exterior materials may be permitted only with the approval of the Planning Director, in consultation with the Building and Codes Director, after a review of the combustibility of the materials.
 - 29. In the RS-4, RS-6, RS-8, RD, CM-R, CM-RS-8, OG-R, and CL districts, a garage attached to a single-family detached dwelling shall have a minimum front setback of 35 feet. The remaining portion of the structure shall have a minimum front setback of 25 feet. The driveway of an attached or detached garage for a single-family detached dwelling in the above districts shall have sufficient width and depth to accommodate four vehicles. A single-family detached dwelling unit in the above zoning districts that has no garage shall have a minimum front setback of 35 feet.
 - 30.In the RM-12, RM-16, CU, CM-R, and RS-A, Type 2 and Type 3 districts, in order to minimize the extent of fire damage on adjacent structures, the facades of townhouse units shall consist primarily of one or more of the following materials: brick, stone, or cementitious siding. Other building materials such as EIFS, vinyl siding, and wood siding may be used for decorative or accent purposes and may constitute no more than 25 percent of any façade. Alternative combinations of exterior materials may be permitted only with the approval of the Planning Director, in consultation with the Building and Codes Director, after a review of the combustibility of the materials.
 - 31. In the RD, RM-12, RM-16, CM-R, OG-R, CL, and RS-A, Type 1 districts, in order to minimize the extent of fire damage on adjacent structures, the facades of single-family attached and detached zero-lot line structures (max. 2 units attached) shall consist primarily of one or more of the following materials: brick, stone, or cementitious siding. Other building materials such as EIFS, vinyl siding, and wood siding may be used for decorative or accent purposes and may constitute no more than 25 percent of any façade. Alternative combinations of exterior materials may be permitted only with the approval of the Planning Director, in consultation

- with the Building and Codes Director, after a review of the combustibility of the materials.
- 32.In the RS-A district, a row of Type 2 (Suburban Townhouse) or Type 3 (Urban Townhouse) townhouses shall consist of a minimum of three townhouse units and no more than eight townhouse units or 240 feet of building length, whichever is less.
- 33.In the RS-A district, Type 2 (Suburban Townhouse), developments shall set aside a minimum of twenty percent of the gross development area as open space. A minimum of five percent of the gross development area shall be designated as formal open space and shall be maintained in perpetuity by the developer and/or Homeowners Association (HOA). A formal open space shall consist of a minimum of 5,000 square feet and may include hardscape improvements, street furnishings, recreational facilities, and amenity structures (i.e. gazebos, arbors, band shells, etc.). The above requirements shall apply to single-family residential attached developments in the RS-A, Type 2 zone but not to single-family residential detached developments in the RS-A, Type 2 zone shall be subject to any applicable open space requirements in the Design Guidelines.
- 34. The following standards shall apply to developments in the RS-A district for Type 3 (Urban Townhouse) developments: (a) When the front setback is less than 30 feet, townhouses shall have a minimum finished floor elevation of eighteen inches above the finished grade located adjacent to the front of the structure. Usable porches/stoops, landscaping, and nonopaque decorative fencing may be used to distinguish between public and private space. (b) Buildings shall be no less than two stories and the maximum building height shall be 45 feet or three stories, whichever is less. However, projections for rooftop patios, such as stairwells and the like, as well as other common rooftop projections such as chimneys, may be allowed up to a maximum height of 55' for three-story buildings. (c) In areas where sidewalk width is equal to or greater than eight feet, and where on-street parking is available in front of the proposed development, townhouses may be constructed to the rear edge of the sidewalk. (d) Offstreet parking shall be located to the rear or side of the building and shall be accessed via alleyway or shared driveway. Individual driveways off of a public street shall not be allowed. Front-facing garages or carports shall not be allowed.
- 35.An application for RS-A zoning shall clearly indicate the development type sought (i.e. Type 1 Zero-Lot Line, Type 2 Suburban Townhouse, or Type 3 Urban Townhouse). If multiple development types are sought for a property, the application shall include a description of the property designated for each development type. 36.Minimum lot area and minimum lot width shall apply to townhouses recorded on individual lots of record. For a townhouse development recorded as a horizontal property regime, the minimum lot area and width requirements shall not apply.
- 37. Single-family detached dwellings and zero-lot line single-family attached and detached dwellings in the RM-12, RM-16, and RS-A (Type 1, Type 2, and Type 3) zones shall have a minimum front setback of 35'. However, in the event that the requirements in Section 26 of this article are met in order to allow garages to count toward the minimum parking requirements, the minimum front setback may be reduced to 25', as long as the minimum number of parking spaces for each lot are being provided on-site. The reduction to 25' may not be made for individual lots on a "case-by-case basis"; rather, a developer shall request the reduction for an

entire subdivision or for an entire section of a subdivision, so that the structures in the development will be constructed in a uniform manner.

38. If there is any conflict between Section 24, Article VI (City Core Overlay District) and the front setback requirements denoted in Chart 1 and its endnotes, Section 24, Article VI (City Core Overlay District) shall prevail.

<u>SECTION 8</u>. Appendix A, Chart 4, Required Off-Street Parking and Queuing Spaces by Use, of the Murfreesboro City Code is hereby amended as follows:

- Change the first entry under "Dwellings" to read "Single-family detached dwellings; single-family detached or attached dwellings, zero-lot line; and two-family dwellings"
- Change the fourth entry under "Dwellings" to read "Single-family attached townhouse dwellings, Urban or Suburban."

<u>SECTION 9</u>. That this Ordinance shall take effect fifteen (15) days after its passage upon second and final reading, the public welfare and the welfare of the City requiring it.

Passed:	Shane McFarland, Mayor
1 st reading	, •
2 nd reading	
ATTEST:	APPROVED AS TO FORM:
Melissa B. Wright City Recorder	Adam F. Tucker City Attorney

SEAL

Chart 1	
Page 1 of 8	

USES PERMITTED ³						ZC	NIN	G DI	STR	ICTS	3														
	15	12	10	8	9	4		RM 12	RM 16	RS-A	R MO	0G R	۵,		14			Q				CM-RS-8	CM-R		
	RS	RS	RS	RS	RS	RS	RD	N N	Ζ	RS	2	90	90	CL	CF ¹⁴	CH	M	CBD	豆 こ	<u>ত</u>	\exists	Ċ	$\frac{2}{\circ}$	CM	DO P
DWELLINGS																									
Single-Family detached	Х	Х	Х	Х	Х	Х	Х	Х	Х	X ²⁷		Х		Х								Х	Х		Х
Single-Family attached or detached, zero-lot																									
line (max. 2 units attached) ²³							Х	Х	Х	X^{24}		Х		Х									Х		Χ
Single-Family attached, townhouse ^{25, 26, 28}								Х	Х	Х													Х		Χ
Two-Family							Х	Χ	Х			Х		Х									Х		Х
Three-Family								Х	Х			Х		Χ									Х		X
Four-Family								Х	Х			Χ		Х									Х		Х
Multiple-Family								X^{21}	X ²¹								X^{21}	X^{21}							Χ
OTHER HOUSING																									
Accessory Apartment	S ⁸	S ⁸	S ⁸	S ⁸	S ⁸	S ⁸				S ⁸															
Accessory Dwelling Unit												X^1	X^1	X^1	X^1	X^1	Х	X ¹	X ¹	X ¹	X^1				
Assisted-Care Living Facility ¹⁵							S	Х	Х	Х		Х		Х	Χ	Χ	Х	Х				Х	Х	Х	S
Bed-and-Breakfast Homestay	S	S	S	S	S		S	S	Х	S		S		Х	Х	Χ		Х				S	S	S	Х
Bed-and-Breakfast Inn	S	S	S	S	S		S	S	S	S		S		S	Х	Χ		Х				S	S	S	S
Boarding House ¹⁵							S	S	Х	Х		S		Х	Х	Х		Х					S	S	Х
Emergency Shelter	Χ	Х	Χ	Х	Х	Χ	Х	Х	Х	Х	Х	Х	Х	Χ	Χ	Χ	Х	Х	Х	Χ	Х	Х	Х	Χ	ХХ
Extended Stay Hotel/Motel																Χ	Х								
Family Crisis Shelter												S		S	S	S			S	S	S		S		
Family Violence Shelter								S	S			S	S	S	Χ	Χ			Χ	Χ	Х		Х	S	S
Fraternity/Sorority												S		S	S	S							S	S	S
Group Shelter								S	S			S	S	S	S	S			S	S					
Class I Home for the Aged ¹⁵	S	S	S	S	S	S	S	Χ	Χ	Х		Χ		Χ	Χ	Χ		Х				S	S	S	S
Class II Home for the Aged 15	S	S	S	S	S		S	S	S	S		S		Х	Χ	Χ		Х				S	S	S	S
Class III Home for the Aged 15								S	S			S		S	Х	Х	Х	Х				S	S	S	S
Hotel																Χ	Х	Х	Χ	Х	Х				
Mission ¹⁰																			S	S	S				
Mobile Homes											Χ														
Motel																Χ	Χ		Χ	Χ	Χ				
Rooming House							S	S	S									Χ					S	S	Х
Student Dormitory									S																Х
Transitional Home							S	S	S			S	S									<u> </u>	S	S	
INSTITUTIONS																									
Adult Day Care Center	S	S	S	S	S	S	S	S	S	S		Х	Х	Χ	Χ	Χ	Χ		Χ	Χ	Χ	S	Х	Χ	
Adult Day Care Home	S	S	S	S	S	S	S	S	S	S	S	Χ	S	Х	Χ	Χ		Х	Χ	Χ	Χ	Х	Χ	Х	

X = Use permitted by right.
S = Use requiring site plan review and approval subject to the issuance of a special use permit in accordance with the provisions of Sections 8 and 9 of this article.

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USES PERMITTED ³						ZC	NIN	G DI	STR	ICTS	3															
	RS 15	RS 12	RS 10	RS 8	RS 6	RS 4	RD	RM 12	RM 16	RS-A	R MO	OG R	90	CL	CF ¹⁴	СН	MU	CBD	豆	GI		CM-RS-8	CM-R	CM	CU	
Airport, Heliport	S	S	S	S	S	S	S	S	<u>~</u>	S	<u>~</u>	0	0	O	0	S	2	O	S	S	S	S	S	S	S	S
Cemetery, Mausoleum	S	S	S	S	S	S	S	S	S	S	S	S	S			S			S	S	S	3	3	3	٦	0
Church ¹³	S	S	S	S	S	S	S	X	X	S	S	S	X	Х	Х	X	Χ	Х	X	X	X	S	S	Х	Х	
College, University	3	3	3	3	3	3	3	^	^	3	3	X	X	^	^	^	X	^	^	^	^	3	X		^ X	
Day-Care Center							S	S	S		S	S	S	Х	Х	Χ	X	Х	Х	Х	Х	S	S	S	^-	
Family Day-Care Home	S	S	S	S	S	S	S	S	S	S	S	S		X	X	X	^	X	X	X	X	S	S	S	Х	
Group Day-Care Home	S	S	S	S	S	S	S	S	S	S	S	S		X	X	X		X	X	X	X	S	S	S	X	
Hospital	<u> </u>	<u> </u>	<u> </u>									X	Х			-	Χ		X	X	X	X	X	X	H	
Lodge, Club, Country Club ¹³	S	S	S	S	S	S	S	S	S	S	S	S	S	S	Х	Χ	Х	Х	Х	Х	Х	S	S	S	П	
Mental Health Facility	۳	۲	۳	۳	-	۳	-	۳	۳	۳	۳	X	X	X	<u> </u>	X	X		X	X	X		X	X	\vdash	
Morgue												<u> </u>	<u> </u>			Х	X		X	X	X		X	X	\vdash	
Museum							S	S	S			S	S	S	Х	Х	Х	Х	X	X	X	S	S	S	Х	S
Nursing Home							_	_				X	X	S	S	S	Χ		Х	Х	Х	Х	X	Х	m	
Nursery School							S	S	S		S	S	S	S	S	S	Χ		S	S	S	S	S	S	Х	
Park	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Χ	Χ	Х	Х	Χ	Х	Х	Х	Х	Х	Χ
Philanthropic Institution							S	S	S			Х	Х	Х	Х	Χ	Χ	Х	Χ	Χ	Х	Х	Х	Х	Х	
Pet Cemetery	S	S	S												S	S			S	S	S					
Public Building ¹³	S	S	S	S	S	S	S	S	S	S	S	S	S	Х	Х	Х	Χ	Х	Х	Х	Х	S	S	S	Х	
Recreation Field ¹³	S	S	S	S	S	S	S	S	S	S	S	S	S	Χ	Х	Χ	Χ		Х	Х	Х	S	S	S	Х	Χ
Senior Citizens Center	S	S	S	S	S	S	S	X	X	S	<u> </u>	X	X	Х	X	X	Х		X	X	X	S	X	X		Ť
School, Public or Private, Grades K - 12 ¹³	S	s	S	S	S	S	S	S	S	s	S	S	S	Χ	Х	Χ	Χ	Х	Х	Х	Х	S	S	S	Χ	
Student Center	<u> </u>	<u> </u>	<u> </u>					S	S			S	S	S	S	S	X			,,			S	S	H	
AGRICULTURAL USES																										
Customary General Farming	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶	X ⁶				X ⁶	Χ			Х	Х	Х				Х	Х
Crop, Soil Preparation Agricultural Services	S	S	S	S	S	S	S	S	S	S	S				X	Х			X	X	X			 	Х	Х
Farm Labor and Management Services	<u> </u>	<u> </u>	<u> </u>	_	_	_	_	_	_	_	_	Х	Х	Х	X	X		Х	X	X	X			 	X	Ť
Fish Hatcheries and Preserves																Х			X	X	X				m	
Grain, Fruit, Field Crop and Vegetable																										
Cultivation and Storage	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х	Х								Х	Х	Х				Х	l
Livestock, Horse, Dairy, Poultry, and Egg																										
Products	S	S	S	S	S	S	S	S	s	s									Х	Χ	Х				Х	l
Timber Tracts, Forest Nursery, Gathering of																										
Forest Products	S	S	S	S	S	S	S	S	S	S	S								Х	Χ	Χ					
COMMERCIAL																										
Adult Cabaret																			X ⁹							
Adult Entertainment Center																			X ⁹							
Adult Motel		†																	X ⁹							
7 Idail Hotol	<u> </u>	<u> </u>	<u> </u>			l	l	l	l	l	l	l	l		l			l	_ ^		1	l	l			

X = Use permitted by right.
S = Use requiring site plan review and approval subject to the issuance of a special use permit in accordance with the provisions of Sections 8 and 9 of this article.

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USES PERMITTED ³						ZC	NIN	G DI	STR	ICTS	3															
	RS 15	RS 12	RS 10	RS 8	RS 6	RS 4	RD	RM 12	RM 16	RS-A	R MO	OG R	90	CL	CF ¹⁴	СН	MU	CBD	王	В		CM-RS-8	CM-R	CM	cn	Д
Adults-Only Bookstore																			X ⁹							
Adults-Only Motion Picture Theater																			X ⁹							
Amusements, Commercial Indoor															Χ	Χ	Χ	Χ	Χ	Х	Х				S	
Amusements, Commercial Outdoor excluding																.,	.,			.,						
Motorized								<u> </u>								Χ	Χ		Х	Х	Х				S	S
Amusements, Commercial Outdoor Motorized																				_						
except Carnivals																			S	S	S				<u> </u>	Ш
Animal Grooming Facility															Х	Х	Χ		Х	Х	Х				<u> </u>	Ш
Antique Mall															Χ	Χ	Χ	Χ	Χ	Х	Х				<u> </u>	Ш
Antique Shop <3,000 sq. ft.												Χ	Х	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Х		Х		<u> </u>	Ш
Apothecaries (pharmaceuticals only)												Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Х		Ш
Art or Photo Studio or Gallery												Х	Χ	Χ	Х	Х	Χ	Х	Χ	Χ	Χ		Х		Χ	
Automotive Repair 12																Х	Χ		Χ	Х	Χ					
Bakery, Retail														Χ	Χ	Χ	Х	Χ	Χ	Χ	Χ					
Bank, Branch Office												Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ	Χ					
Bank, Drive-Up Electronic Teller												Χ	Χ	Χ	Χ	Χ	Х	Χ	Χ	Χ	Χ					
Bank, Main Office																Χ	Х	Χ	Χ	Χ	Χ					
Barber or Beauty Shop												Х	Х	Χ	Χ	Χ	Х	Х	Χ	Х	Х		Х			
Beer, Packaged														Χ	Χ	Χ		Χ	Χ	Х	Х					
Boat Rental, Sales, or Repair																Χ			Χ	Х	Х					
Book or Card Shop												Χ	Χ	Χ	Χ	Χ	Х	Χ	Χ	Χ	Х		Х			
Business School												Χ	Χ		Χ	Χ	Х	Χ	Χ	Χ	Х					
Business and Communication Service												Х	Х	Χ	Χ	Χ	Х	Х	Χ	Х	Х					
Campground, Travel-Trailer Park																Χ			Χ	Х	Х					
Carnivals																S			S	S	S					S
Catering Establishment				Ì				Ì				Χ	Х	Х	Χ	Χ	Χ	Х	Χ	Х	Х		Х			П
Clothing Store														Χ	Χ	Х	Χ	Х	Χ	Х	Х					П
Coffee, Food, or Beverage Kiosk														Х	Χ	Х	Χ		Χ	Х	Χ					
Commercial Center														Χ	Χ	Χ	Χ		Х	Х	Х					П
Convenience Sales and Service, maximum				1																						\Box
5,000 sq. ft. floor area														Х	Х	Х	Χ	Х	Х	Х	Х					
Crematory																			S	S	S					
Delicatessen							1			l				Χ	Χ	Χ	Χ	Х	Χ	Х	Х	1				П

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Fireworks Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Sa S S S S S S S S S S S S S S S S S S	USES PERMITTED ³						ZC	NIN	G DI	STR	ICTS	3															\neg
Department or Discount Store		S 15	S 12	S 10			S 4	Q	M 12	M 16	S-A	МО	GR	و و	Ļ	F ¹⁴	Į	⊇	BD	_	=		M-RS-8	M-R	Σ	D.	
Drive-In Theater	Department or Discount Store	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> 62</u>	<u>~</u>	<u>«</u>	<u> </u>	2	<u> </u>	<u>~</u>	0	0	O								Ö	O	0	0	₽.
Dry Cleaning Pick-Up Station	'															^		^	^						₩	\vdash	
Dry Cleaning Pick-Up Station															V	V		V	V						\vdash	\vdash	-
Financial Service																									$\vdash \vdash$	$\overline{}$	_
Fireworks Public Display													Y	Y											$\vdash \vdash \vdash$		
Fireworks Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Seasonal Retailer Fireworks Sa S S S S S S S S S S S S S S S S S S													^	^	^	^		^	^	^	^	^			$\vdash \vdash$	$\overline{}$	Χ
Fireworks Seasonal Retailer																	9			9	9	9			\vdash	\vdash	_
Flower or Plant Store															9										\vdash	\vdash	
Funeral Home Garage, Parking Garden and Lawn Supplies Gas-Liquified Petroleum, Bottled and Bulk Gas-Liquified Petroleum, Bottled and Bulk Gas Station Gas-Liquified Petroleum, Bottled and Bulk Gas Station Gas Station Gas-Liquified Petroleum, Bottled and Bulk Gas Station Gas-Liquified Petroleum, Bottled and Bulk Gas Station Gas-Liquified Petroleum, Bottled and Bulk Gas Station Cas Station		1	1	-	-		1	-	1	-		1	V	V		У		У	У					V	$\vdash\vdash$	$\overline{}$	
Garage, Parking		 	1	-		-	-	 	-	 		 		 ^					^					_^	₩	-	
Garden and Lawn Supplies		-		-	 	-		-	 						0	-			_					-	$\vdash \vdash$	-	
Gas-Liquified Petroleum, Bottled and Bulk Gas Station Gas Station Gas General Service and Repair Shop GlassAuto, Plate, and Window GlassStained and Leaded Greenhouse or Nursery Group Assembly, <250 persons Group Assembly, >250 persons Group Ass		-		-	 	-		-	 							6								-	$\vdash \vdash$		
Gas Station																0		^	^						 	\vdash	
General Service and Repair Shop									<u> </u>						V	V		· ·	V						 	⊢⊦	
GlassStained and Leaded GlassStained and Leaded Greenhouse or Nursery Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons Group Assembly, <250 persons					-										Χ	Χ									<u> </u>	\vdash	
ClassStained and Leaded					-		ļ					-							Х							\vdash	
Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Signature Sign					-		ļ					-														\vdash	
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S S S S S S S S S S S S S S S S S S S													_	_											ليبا	$\vdash \vdash$	
Health Club																										$\vdash \vdash$	
Color Retail																							S		S	\longrightarrow	
Interior Decorator													Х	Х	Х			Х						Х		ш	
Iron Work																										ш	
Janitorial Service													Х	Х	Х	Х		Х	Х					Х		ш	
Karate, Instruction XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX																											
Kennels X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>																											
Keys, Locksmith XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	Karate, Instruction															Х		Х	Χ								
Laboratories, Medical X X X X X X X X X X X X X X X X X X X	Kennels																			Χ							
Laboratories, Testing XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	Keys, Locksmith															Х			Χ		Χ	Χ				لـــــا	
Laundries, Self-Service X X X X X X X X X X X X X X X X X X X	Laboratories, Medical												Χ	Х									Χ	Χ	Χ		
Lawn, Tree, and Garden Service X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X <td< td=""><td>Laboratories, Testing</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>X</td><td>X</td><td>X</td><td></td><td>X</td><td>X</td><td>X</td><td></td><td></td><td></td><td></td><td></td></td<>	Laboratories, Testing															X	X	X		X	X	X					
Liquor Store X X X X X X X X X Livestock, Auction X X X X X X X Lumber, Building Material X X X X X Manufactured Home Sales X X X X	Laundries, Self-Service														Х	Х	Х			Χ	Χ	Х					
Livestock, Auction X X X Lumber, Building Material X X X X Manufactured Home Sales X X X	Lawn, Tree, and Garden Service																Х			Χ	Χ	Х					
Lumber, Building Material X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X <td>Liquor Store</td> <td></td> <td>Х</td> <td>Х</td> <td>Х</td> <td>Х</td> <td>Χ</td> <td></td> <td>Х</td> <td></td> <td></td> <td></td> <td></td> <td></td>	Liquor Store															Х	Х	Х	Х	Χ		Х					
Lumber, Building Material X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X X <td>Livestock, Auction</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>l</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>l</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>Χ</td> <td>Χ</td> <td>Х</td> <td></td> <td></td> <td></td> <td></td> <td></td>	Livestock, Auction							l						l						Χ	Χ	Х					
Manufactured Home Sales X X	Lumber, Building Material							l						l			Х			Χ	Χ	Х					
	Manufactured Home Sales																			Χ	Χ						
Massage Parlor	Massage Parlor	Ì	İ		Ì				Ì											X ⁹							

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USES PERMITTED ³						ZC	NINC	IG DI	ISTR	ICTS	3															
	RS 15	RS 12	RS 10	RS 8	RS 6	RS 4	RD	RM 12	RM 16	RS-A	R MO	0G R	90	CL	CF ¹⁴	СН	МО	CBD	王	GI	_	CM-RS-8	CM-R	CM	cn	<u>م</u>
Motor Vehicle Sales (Automobiles)																S	S		X3	X ³	X ³					=
Motor Vehicle Sales (Other Than																										=
Automobiles)																S	S		Х	Х	Х					
Motor Vehicle Service 12																Χ	Χ		Х	Х	Х					ヿ
Movie Theater															Х	Х	Χ	Х	Х	X	X			\Box		\dashv
Music or Dancing Academy															Х	Х	Χ		Х	Х	Х					\dashv
Offices												Х	Х	Χ	Χ	Χ	Χ	Х	Х	Х	Х	X^5	X ⁵	X ⁵		
Optical Dispensaries												Х	Х		Х	Х	Х	Х	Х	Х	Х	Х	Х	Х		$\overline{}$
Pawn Shop		1						1	1	1		1	<u> </u>			Х		X	X	X	X		<u> </u>			\dashv
Personal Service Establishment									1					Χ	Х	Х	Х	X	X	X	X			\Box		\exists
Pet Crematory							l	1	1									1	S	S	S			\Box		\dashv
Pet Funeral Home															Х	Х			Х	Х	Х					ヿ
Pet Shops															Χ	Χ	Χ	Х	Х	Χ	Х					\exists
Pharmacies												Х	Х	Χ	Χ	Χ	Х	Х	Х	Χ	Х	Х	Х	Χ		\exists
Photo Finishing														Χ	Χ	Χ	Χ	Х	Х	Χ	Х					
Photo Finishing Pick-Up Station														Χ	Χ	Χ	Х		Х	Χ	Х					\Box
Radio, TV, or Recording Studio																Χ	Χ	Х	Х	Χ	Х					\Box
Radio and Television Transmission Towers															S	S		S	S	S	S				S	
Rap Parlor																			X ⁹							
Reducing and Weight Control Service												Х	Х	Χ	Χ	Χ	Х	Х	Х	Χ	Х	Х	Х	Χ		\exists
Restaurant and Carry-Out Restaurant														Χ	Χ	Χ	Χ	Х	Х	Χ	Х					\exists
Restaurant, Drive-In																Χ			Х	Χ	Х					
Restaurant, Specialty														Χ	Χ	Χ	Χ	Х	Х	Χ	Х					\Box
Restaurant, Specialty -Limited												S	S	Χ	Χ	Х	Χ	Х	Х	Χ	Х	S	S	S		
Retail Shop, other than enumerated elsewhere															Χ	Χ	Χ	Χ	Χ	Χ	Х				\sqcup	
Salvage and Surplus Merchandise																Χ			Χ	Χ	Χ				$ldsymbol{\sqcup}$	
Sauna					<u></u>	<u></u>												<u></u>	X ⁹						<u>L</u>	
Sheet Metal Shop																Χ			Χ	Χ	Χ					
Shopping Center, Community																Χ	Χ		Х	Χ	Χ					
Shopping Center, Neighborhood															Χ	Χ	Χ		Х	Χ	Χ					
Shopping Center, Regional																Х	Χ		Χ	Х	Χ					
Specialty Shop												Х	Х	Χ	Х	Х	Χ	Х	Х	Χ	Χ		Χ			
Tavern																Χ		Х	Χ	Χ	Χ				igsqcut	\square
Taxidermy Studio																S			S	S	S				Ш	
Towing 12																Χ			Х	X	Χ				ш	\square

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USES PERMITTED ³						ZC	NIN	G DI	STR	ICTS	3															
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	15	12	10	œ	9	4		RM 12	RM 16	ℴ	0	œ			l_							CM-RS-8	22			
	RS,	RS.	RS.	RS 8	RS (RS 4	R)	Σ	Σ	RS-A	Σ	OG R	90	CF	CF ¹⁴	НЭ	ΩМ	СВD	Ξ	19	_	Ā-	CM-R	CM	C	11
Veterinary Office	<u> </u>	<u>~</u>	2	<u> </u>	<u> </u>	<u>~</u>	<u> </u>	<u> </u>	2	2	22	X	X	X	X		≥	O	X	X	⊒ X	Ö	X	10	0	Д
Veterinary Office Veterinary Clinic												^		^	X	X	X		X	X	X		^	₩		
Veterinary Hospital															^	X	X		X	X	X			+-	\vdash	_
Vehicle Sales (Non-Motorized)																X	X		X	X	X			+	\vdash	
Vehicle Wash														Х		X	X		X	X	X			+	\vdash	
Video Rental														X	Х	X	X	Χ	X	X	X			+	\vdash	
Wholesaling		-	-													X		X	X	X	X			+	\vdash	
Wireless Telecommunications Towers,																								_		
Antennas ¹⁷	s	s	s	s	s	S	s	s	s	s	s	s	s	S	s	S	s	s	S	S	S	s	s	S	s	S
Wrecker Service, Wrecker Storage Yard ¹²	-	3	3	-	3	3	3	-	-	-	-	-	-	3	3	X	3	3	X	X	X	-	3	- 3	3	
INDUSTRIAL																Λ.								 		
Manufacture, Storage, Distribution of:																							1	-	H	
Abrasive Products																			Х	Х				1		
Alcoholic Beverage Manufacture																			X ²⁰	X ²⁰				\vdash		
Asbestos Products																			S	^				+	\vdash	
Automobile Dismantlers and Recyclers ⁷		-	-																S ⁷					+	\vdash	
																				· · ·				 	\vdash	
Automobile Manufacture																			Χ	Х				<u> </u>		
Automobile Parts and Components Manufacture																			V	V						
Automobile Seats Manufacture																			X	X				 	\vdash	
Bakery Goods, Candy																			X	X	Х			 	\vdash	
Boat Manufacture				-				-												X	^				\vdash	
Bottling Works				-				-											X	X	Х				\vdash	
Brewery																			X	X	^			₩		
Canned Goods				-	-			-	-										X	X			-	 	₩	
Chemicals																			X	^				\vdash	₩	
Composting Facility																			S					+-	S	
Contractor's Storage, Indoor																Χ		Х	X	Х	Х			+	3	
Contractor's Yard or Storage, Outdoor																X		X	X	X	X			+	\vdash	
Cosmetics		-	-																X	X	X			+	\vdash	
Custom Wood Products																		Χ	X	X	X			_		
Electrical or Electronic Equipment,	1			1	1	1		1	†						1						 ^`		†	\vdash		
Appliances, and Instruments																			Х	Х	Х					
Fabricated Metal Products and Machinery				<u> </u>				<u> </u>											X	X	X			\vdash	\vdash	
Fertilizer				1	1			1	1				1						X					T		
				1	1			1	1				1											T		
Food and Beverage Products except animal																										
slaughter, stockyards, rendering, and brewery																			Х	Χ	Χ					

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Revised: ?/?/21

USES PERMITTED ³						70	NINC	G DI	STR	ICTS	<u> </u>														$\overline{}$	
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	15	12	10			١		2	9	_	0	~										CM-RS-8	~			
	RS 1	RS 1	RS 1	RS 8	RS 6	RS 4	RD.	RM 12	RM 16	RS-A	R MO	0G R	90	딩	CF ¹⁴	СН	MU	CBD	_	_		A-R	CM-R	CM	\Box	
	2	2	2	2	2	2	2	2	2	2	2	0	0	ပ	O	ပ	Σ	ပ	王	ō		บิ	O	O	긍	Д
Furniture and Fixtures													-						X	X	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		-	<u> </u>	<u> </u>	Щ
Jewelry						ļ													Х	Х	Х			₽	₩	Ш
Leather and Leather Products except tanning																			.,	.,	.,					
and finishing													-		-				Х	Х	Х		<u> </u>	—		Щ
Leather and Leather Products, Tanning and																			.,							
Finishing						ļ													Х	.,				₽	₩	Ш
Lumber and Wood Products						ļ													Х	Х				ــــــ	<u> </u>	Ш
Mobile Home Construction																			Х		L			ــــــ	ــــــ	Ш
Musical Instruments																			Х	Χ	Х			ــــــ	Ь	Ш
Office/Art Supplies	<u> </u>					<u> </u>	<u> </u>	<u> </u>					1	<u> </u>					Χ	Х	Χ			<u> </u>	<u> </u>	Ш
Paints																			Χ	Х						
Paper Mills																			S							
Paper Products excluding paper and pulp mills	3																		Χ	Χ						
Petroleum, Liquified Petroleum Gas and Coal																										
Products except refining																			S							
Petroleum and Coal Products Refining																			S							
Pharmaceuticals																			Χ	Χ	Х					
Photographic Film Manufacture																			Χ	Χ						
Pottery, Figurines, and Ceramic Products																			Χ	Х	Х					
Primary Metal Distribution and Storage																			Χ	Х						
Primary Metal Manufacturing																			Χ	Х						
Printing and Publishing																Х	Х	Х	Χ	Х	Х					
Rubber and Plastic Products except rubber or																										
plastic manufacture																			Χ	Х						1
Rubber and Plastic Products, Rubber and																										
Plastic Manufacture																			Χ	Х						
Saw Mills																			Χ							
Scrap Processing Yard																			S							
Scrap Metal Processors																			S							
Scrap Metal Distribution and Storage							l –	l –					1	l –					S					1		
Secondary Material Dealers																			S							
Silverware and Cutlery					1														Χ	Х	Х			1		
Small Moulded Metal Products					1														Χ	Х				1		
Sporting Goods					1														Χ	Х	Х			1		
Stone, Clay, Glass, and Concrete Products													1						Х	X			1		T	
Textile, Apparel Products, CottonFactoring,																								†		
Grading																			Х	Х	Х					
Textile, Apparel Products, Cotton Gin																			X	X				†		

X = Use permitted by right.

S = Use requiring site plan review and approval subject to the issuance of a special use permit in accordance with the provisions of Sections 8 and 9 of this article.

Revised: ?/?/21

USES PERMITTED3 ZONING DISTRICTS CM-RS-8 RM 12 RM 16 RS 10 RS 15 R MO CM-R RS-A OG R RS 4 MU ∞ CF¹⁴ CH RS 3S RS (B 90 S C M Tire Manufacture Χ Χ Tobacco Products Χ Х Toiletries Χ Χ Χ Transportation Equipment Χ Χ Χ Warehousing, Transporting/Distributing ¹⁸
TRANSPORTATION AND PUBLIC Χ Χ Χ UTILITIES Bus Terminal or Service Facility Х X X X Garbage or Refuse Collection Service Χ Χ Refuse Processing, Treatment, and Storage S Gas, Electric, Water, Sewerage Production and/or Treatment Facility Χ Χ S Landfill¹⁹ S Post Office or Postal Facility X X X X X Х Х Χ X X X X Telephone or Communication Services Х Х Χ Electric Transmission, Gas Piping, Water S Pumping Station S S S S S S s s S S S S Х Х XX Χ Χ Χ Χ Taxicab Dispatch Station Χ Χ Χ Χ Freight Terminal, Service Facility Χ Χ Χ Χ OTHER Advertising Sign Χ Х X Х S¹¹ S¹¹ S¹¹ | S¹¹ | S¹¹ | S¹¹ | S¹¹ | S¹¹ | S¹¹ | S¹¹ | S¹¹ | S¹¹ S¹¹ S¹¹ S¹¹ Χ Χ Home Occupations Junkyard S Recycling center S Χ Χ Х Self-Service Storage Facility 16 s s x s Χ Χ Χ Wholesale Establishments Χ Χ Χ Χ Χ Temporary Mobile Recycling Center S S S S S S

X = Use permitted by right.

S = Use requiring site plan review and approval subject to the issuance of a special use permit in accordance with the provisions of Sections 8 and 9 of this article.

Chart 2 Page 1 of 4

	Minimu	ım Lot		nimum Ya					Land Use		
	Require	ements	Requ	irements ^{[5}][17][25]			Int	ensity Ratio	os	
						Maximum	Gross		•		Maximum Lot
	Area	Width	Front ^[38]	Side	Rear	Height ^[16]	Density ^[2]	Maximum		Minimum	Coverage
DISTRICT AND USE	(Sq. Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(D.U./Acre)	F.A.R.	L.S.R.	O.S.R.	(percent)
RS-15 DISTRICT											
Dwellings and other uses permitted	15,000	75 ^[12]	40	12.5	30	35	2.9	none	none	none	25
RS-12 DISTRICT											
Dwellings and other uses permitted	12,000	70 ^[12]	35	10	25	35	3.63	none	none	none	25
RS-10 DISTRICT											
Dwellings and other uses permitted	10,000	65 ^[12]	35	10	25	35	4.4	none	none	none	25
RS-8 DISTRICT											
1. Dwellings and other uses permitted ^[28]	8,000	55 ^[12]	35 ^{[1][29]}	5 ^[10]	20	35	5.4	none	none	none	30
RS-6 DISTRICT											
1. Dwellings and other uses permitted ^[28]	6,000	50 ^[12]	35 ^{[1][29]}	5	20	35	7.2	none	none	none	50
RS-4 DISTRICT											
1. Dwellings and other uses permitted ^[28]	4,000	40 ^[12]	35 ^{[1][29]}	5	20	35	10.8	none	none	none	40
R-D DISTRICT											
Single-family detached dwellings and											
other uses permitted except ^[28]	8,000	55 ^[12]	35 ^{[1][29]}	5	25	35	5.4	none	none	none	30
2. Two-family dwellings	8,000	55 ^[12]	30 ^[1]	5	25	35	10.9	none	none	none	30
3. Single-family attached or detached with	,										
zero lot line (max. 2 units attached)[7][31]	4,000	27 ^[12]	35 ^[1]	10 ^[7]	25	35	10.9	0.3	0.48	0.7	none

Chart 2 Page 2 of 4

	Minimu	ım Lot		nimum Ya					Land Use			
	Require	ements	Requ	irements ^[5]][17][25]			Int	ensity Ratio	os		
						Maximum	Waximum Gross				Maximum	
	Area	Width	Front ^[38]	Side	Rear	Height ^[16]	Density ^[2]	Maximum	Minimum	Minimum	Lot Coverage	
DISTRICT AND USE	(Sq. Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(D.U./Acre)	F.A.R.	L.S.R.	O.S.R.	(percent)	
RM-12 DISTRICT	(= 4)	()	()	()	()	()	(2.0			0.0	(10.00)	
Single-family detached dwellings and												
other uses permitted except ^[28]	7,500	50 ^[12]	35 ^{[1][37]}	5	25	35	5.8	none	none	none	30	
2. Two-family dwellings	7,500	50 ^[12]	30 ^[1]	5	25	35	11.6	none	none	none	30	
3. Three-family dwellings	11,250	50 ^[12]	30 ^[1]	5	25	35	11.6	none	none	none	30	
4. Four-family dwellings	15,000	50 ^[12]	30 ^[1]	5	25	35	11.6	none	none	none	30	
5. Single-family attached or detached with	10,000	00	00	O	20	00	11.0	Hone	Hone	Hone	00	
zero lot line (max. 2 units attached) ^{[7][31]}	3,750	18 ^[12]	35 ^{[1][37]}	10 ^[7]	25	35	11.6	none	none	none	none	
6. Multiple-family dwellings and Single-	0,. 00			. •								
family attached townhouse dwellings ^[30]	FN ^[14]	50 ^[12]	30 ^[1]	FN ^[3]	25	45 ^[11]	FN ^[14]	none	none	FN	none	
RM-16 DISTRICT						-						
1. Single-family detached dwellings and												
other uses permitted except ^[28]	6,000	50 ^[12]	35 ^{[1][37]}	5	25	35	7.3	none	none	none	35	
2. Two-family dwellings	6,000	50 ^[12]	30 ^[1]	5	25	35	14.5	none	none	none	35	
3. Three-family dwellings	9,000	50 ^[12]	30 ^[1]	5	25	35	14.5	none	none	none	30	
4. Four-family dwellings	12,000	50 ^[12]	30 ^[1]	5	25	35	14.5	none	none	none	30	
5. Single-family attached or detached with	·											
zero lot line (max. 2 units attached)[7][31]	3,000	18 ^[12]	35 ^{[1][37]}	10 ^[7]	25	35	14.5	none	none	none	none	
6. Multiple-family dwellings and Single-												
family attached townhouse dwellings ^[30]	FN ^[9]	50 ^[12]	30 ^[1]	FN ^[3]	25	45 ^[11]	FN ^[9]	none	none	FN	none	
RS-A DISTRICT ^[35]												
1. Single-family detached and single-family												
attached or detached with zero-lot line												
(max. 2 units attached) ^{[7][28][31]}	3,000	30 ^[12]	35 ^{[1][37]}	5	20	35	14.5	0.3	0.48	0.7	none	
2. Single-family attached townhouse on one												
lot or individual lots (Suburban	[36]	[36]	[1]									
Type) ^{[30][32][33]}	2,000 ^[36]	20 ^[36]	35 ^[1]	5	20	35	12	1	0.5	0.25	none	
3. Single-family attached townhouse on one												
lot or individual lots (Urban Type)[30][32][33][34]	2,000 ^[36]	20 ^[36]	20 ^{[1][34]}	_	20	45 ^[34]	40					
4. Other uses permitted		30 ^[12]	30 ^[1]	5	20	_	12	1	none	none	none	
R-MO DISTRICT	6,000	30. 1	30. ,	10	20	35	none	none	none	none	35	
1. Mobile homes	4,000	40 ^[12]	25 ^[1]	10	15	12	10.9	nono	nono	nono	nono	
1. WOSHO HOHIOO	4,000	40	25"	10	15	ΙZ	10.9	none	none	none	none	

Chart 2 Page 3 of 4

	Minimu	um Lot		inimum Ya					Land Use		
	Require	ements	Requ	irements ^{[5}][17][25]			Int	ensity Ratio	os	
						Maximum	Gross				Maximum
	Area	Width	Front ^[38]	Side	Rear	Height ^[16]	Density ^[2]	Maximum	Minimum	Minimum	Lot Coverage
DISTRICT AND USE	(Sq. Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(D.U./Acre)	F.A.R.	L.S.R.	O.S.R.	(percent)
CM-R DISTRICT				,			/				/ /
1. Single-family detached	5,000	50 ^[12]	35 ^{[1][29]}	10	20	35	8.7	none	none	none	none
2. Two-family dwellings	5,000	50 ^[12]	30 ^[1]	10	20	35	16	none	none	none	none
3. Single-family attached or detached with	5,555										
zero lot line (max. 2 units attached)[7][31]	2,500	30	35 ^[1]	10	20	35	16	none	none	none	none
4. Single-family attached townhouse											
dwellings ^[30]	2,500	50 ^[12]	30 ^[1]	10	20	35	16 ^[9]	0.3	0.48	0.7	none
5. Four-family dwellings	15,000	50 ^[12]	30 ^[1]	5	25 ^[4]	35	11.6	none	none	none	30
6. Medical offices, clinics, and other related		[40]	[41								
uses	none	50 ^[12]	30 ^[1]	10	20	60	none	none	none	none	none
CM DISTRICT											
Medical offices, clinics, and other related uses	none	50 ^[12]	30 ^[1]	10	20	60	none	none	none	none	none
CM-RS-8 DISTRICT	HOHE	30	30	10	20	00	Hone	HOHE	Hone	Hone	Hone
Single-family detached	8,000	50 ^[12]	35 ^{[1][29]}	10	20	35	5.4	none	none	none	none
2. Medical offices, clinics, and other related	0,000	00		10	20	00	0.1	110110	110110	110110	110110
uses	none	50 ^[12]	30 ^[1]	10	20	60	none	none	none	none	none
OG-R DISTRICT											
Offices and other uses except	5,000	50 ^[12]	30 ^[1]	10	20	35	none	0.3	0.28	0.6	none
Single-family detached	5,000	50 ^[12]	35 ^{[1][29]}	10	20	35	8.7	none	none	none	none
3. Two-family dwellings	5,000	50 ^[12]	30 ^[1]	10	20	35	17.4	none	none	none	none
4. Three-family dwellings	7,500	50 ^[12]	30 ^[1]	10	20	35	17.4	none	none	none	30
5. Four-family dwellings	12,000	50 ^[12]	30 ^[1]	10	20	35	14.5	none	none	none	30
6. Single-family attached or detached with											
zero lot line (max. 2 units attached)[7][31]	2,500	25 ^[12]	35 ^[1]	10	20	35	17.4	none	none	none	none
OG DISTRICT				_	_						
Offices and other uses	5,000	50 ^[12]	30 ^[1]	10	20	35	none	0.3	0.28	0.6	none

Chart 2 Page 4 of 4

	Minim	um Lot		nimum Ya					Land Use		
	Require	ements	Requ	irements ^{[5}][17][25]			Int	ensity Ratio	os	
							Maximum				Maximum
			[38]	-	_	Maximum	Gross			l	Lot
DIOTRIOT AND LIGH	Area	Width	Front ^[38]	Side	Rear	Height ^[16]	Density ^[2]	Maximum	Minimum	Minimum	Coverage
DISTRICT AND USE	(Sq. Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(Ft.)	(D.U./Acre)	F.A.R.	L.S.R.	O.S.R.	(percent)
CL DISTRICT		[13]		[6]							
All commercial uses except	none	none ^[13]	42	10 ^[6]	20	35	none	none	none	none	none
2. Single-family detached dwellings ^[28]	7,500	50 ^[12]	35 ^{[1][29]}	5	25	35	5.8	none	none	none	30
Two-family dwellings	7,500	50 ^[12]	30 ^[1]	5	25	35	11.6	none	none	none	30
4. Three-family dwellings	11,250	50 ^[12]	30 ^[1]	5	25	35	11.6	none	none	none	30
5. Four-family dwellings	15,000	50 ^[12]	30 ^[1]	5	25	35	11.6	none	none	none	30
6. Single-family attached or detached with											
zero lot line (max. 2 units attached)[7][31]	3,750	18 ^[12]	35 ^[1]	10 ^[7]	25	35	11.6	0.3	0.48	0.7	none
CF DISTRICT											
1. All uses	none	none ^[13]	42	10 ^[15]	20 ^[15]	45	none	none	none	none	none
CH DISTRICT											
1. All uses	none	none ^[13]	42	10 ^[6]	20	75	none	none	none	none	none
MU DISTRICT											
1. Multiple family dwellings	5 acres	100 ^[20]	15 ^[21]	10 ^[22]	20 ^[23]	75	25 ^[24]	none	none	none	none
2. All commercial uses except mixed use	none	100 ^[20]	15 ^[21]	10 ^[22]	20 ^[23]	150	none	none	none	none	none
3. Mixed uses (vertical mix)	none	100 ^[20]	15 ^[21]	10 ^[22]	20 ^[23]	150	25 ^[24]	none	none	none	none
CBD DISTRICT											
1. All uses except	none	none ^[13]	none	none	none	75	none	none	none	none	none
2. Multiple-family dwellings	none	none ^[13]	none	none	none	75	FN ^[8]	none	none	none	none
H-I DISTRICT											
1. All uses	none	50 ^[13]	42	10	20	75	none	none	none	none	none
G-I DISTRICT											
1. All uses	none	50 ^[13]	42	10	20	75	none	none	none	none	none
L-I DISTRICT											
1. All uses	none	50 ^[13]	42	10	20	75	none	none	none	none	none
CU DISTRICT											
Single-family detached	10,000	65 ^[12]	35	10	20	35	4.4	none	none	none	25
2. Two-family dwellings	10,000	65 ^[12]	35	10	20	35	8.7	none	none	none	25
3. Three-family dwellings	15,000	65 ^[12]	35	10	20	35	8.7	none	none	none	25
4. Four-family dwellings	20,000	65 ^[12]	35	10	20	35	8.7	none	none	none	25
5. Multiple-family dwellings and Single-							0.7	110110	1.5110		_~
family attached townhouse dwellings ^[30]	25,000	65 ^[12]	35	10 ^[3]	20 ^[4]	35	FN ^[9]	0.35	0.45	0.65	none
6. Educational institutions and other uses	25,000	65 ^[12]	35	10	20	35	none	0.3	0.48	0.6	none
P DISTRICT	,			-				-			
1. All uses permitted	none	none ^[13]	none	none	none	none	none	none	none	none	none